

# **TRAFFIC & PARKING IMPACT ASSESSMENT**

# PROPOSED RESIDENTIAL APARTMENT DEVELOPMENT 36 – 38 RODLEY AVENUE **PENRITH**

PREPARED FOR INGLOW INVESTMENT TWO PTY. LTD. **OUR REF: 18-045-2** 



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# 1. INTRODUCTION

# 1.1 Background

A Development Application (DA18/0890) was lodged with Penrith Council in September 2018 seeking consent for the demolition of two existing detached dwellings at 36 – 38 Rodley Avenue, Penrith and the construction of a six storey residential flat building incorporating 20 apartments.

The original application was proposed to be serviced by two basement levels containing 36 passenger vehicle parking spaces. The development was proposed to be serviced by a single access driveway connecting with Rodley Avenue northeastern corner of the site. Connectivity between the access driveway and the upper basement parking level was proposed by a single lane access ramp, whereby two-way traffic flow was proposed to be governed by internal traffic signals. A similar single lane ramp, governed by traffic signals, was proposed to facilitate connectivity between the upper and lower basement parking levels.

Further to the above internal site works, the proposal involved the construction of an indented bay within the southern Rodley Avenue footway. This bay, providing approximate dimensions of 20.5m x 2.5m, was proposed to partially encompass the passenger vehicle driveway but be primarily located to the west, to accommodate waste collection activities associated with the proposed development.

This Practice prepared a Traffic & Parking Impact Assessment in support of the original Development Application, dated August 2018, on behalf of Inglow Investment Two Pty. Ltd.

The Local Planning Panel refused the application on 22 May 2019 stating a number of reasons, including, but not limited to the following:

- The application has not satisfied Council with respect to the requirements under Section C5 'Waste Management', specifically:
  - The proposal provides for street collection and waste bin storage rooms on the ground floor.
- The application has not satisfied Council with respect to the requirements under Section C10 'Transport Access and Parking', specifically:
  - The proposal provides for single width ramps to the basement levels to cater for two way traffic.
  - The indented waste collection is not supported.

Various amendments were made to the development scheme, including but not being limited to the accommodation of refuse collection vehicles and activities on-site. The amended development scheme was subject to a Section 8.2 Review which upheld the original refusal.

An entirely new development application has now been formulated. The new application involves the demolition of the existing two dwellings and the construction of a new five storey residential flat building containing 16 apartments.

The current application is proposed to be serviced by a single basement level containing 22 passenger vehicle parking spaces. The development is proposed to be serviced by a single access driveway connecting with Rodley Avenue northeastern corner of the site. Connectivity between the access driveway and the basement parking level is proposed by a single lane access ramp, whereby twoway traffic flow was proposed to be governed by internal traffic signals. These signals are however proposed to provide altered operational management arrangements to that originally proposed.

Further to the above internal site works, the current application involves the construction of an indented waste collection bay within the southern Rodley Avenue footway, providing increased width and length dimensions compared with the original application.

The current development scheme has been subject to pre-lodgement assessment and advice by Council officers and the Urban Design Review Panel.

This report forms an updated Traffic & Parking Impact Assessment, reflecting the current application scheme which is now subject to a new Development Application, and specifically addresses the originally cited reasons for refusal which relate to traffic and parking considerations.

## 1.2 Scope of Assessment

Stanbury Traffic Planning has been retained by Inglow Investment Two Pty. Ltd. to prepare an updated Traffic & Parking Impact Assessment to accompany the new Development Application to be lodged with Penrith City Council. The Development Application seeks consent for the demolition of two detached residential dwellings and the construction of a residential apartment development containing 16 dwellings at 36 – 38 Rodley Avenue, Penrith (hereafter referred to as the 'subject site').

This aim of this assessment is to investigate and report upon the potential traffic and parking consequences of the development application and to recommend appropriate ameliorative measures where required. This report provides the following scope of assessment:

- Section 1 provides a summary of the site location, details, existing and surrounding land-uses;
- Section 2 describes the proposed development;
- Section 3 assesses the adequacy of the proposed site access arrangements, parking provision, internal circulation and servicing arrangements with reference to relevant Council, Roads & Maritime Services and Australian Standard specifications;

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- Section 4 assesses the existing traffic, parking and transport conditions surrounding and servicing the subject development site including a description of the surrounding road network, traffic demands, operational performance and available public transport infrastructure; and
- Section 5 estimates the projected traffic generating ability of the proposed development and assesses the ability or otherwise of the surrounding road network to be capable of accommodating the altered demand in a safe and efficient manner.

The report has been prepared pursuant to State Environmental Planning Policy (Infrastructure) 2007. The application is not of sufficient scale to be referred to the Roads & Maritime Services under this Instrument.

#### 1.3 Reference Documents

Reference is made to the following documents throughout this report:

- The Roads & Maritime Services' *Guide to Traffic Generating Developments* and the more recently released *Technical Direction TDT 203/04a*;
- Penrith City Council's Penrith Development Control Plan 2014 (DCP 2014);
- Australian Standard for Parking Facilities Part 1: Off-Street Car Parking (AS2890.1:2004);
- Australian Standard for Parking Facilities Part 3: Bicycle Parking Facilities (AS2890.3:2015); and
- Australian Standard for *Parking Facilities Part 6: Off-Street Parking for People with Disabilities* (AS2890.6:2009).

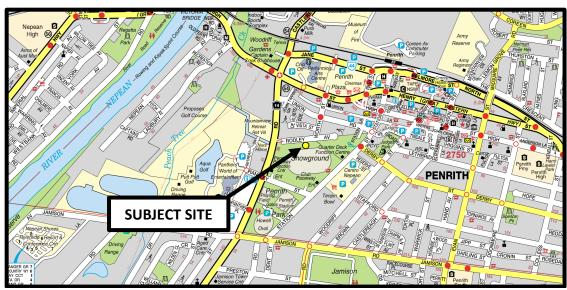
Architectural plans have been prepared by Morson Group and should be read in conjunction with this report, reduced copies of a selection of which (ground and basement plans only) are included as **Appendix 1** for reference.

#### 1.4 Site Details

#### 1.4.1 Site Location

The subject site is situated on the southern side of Rodley Avenue, approximately 60m to the west of Worth Street, Penrith. The site location is illustrated overleaf within a local and aerial context by **Figure 1** and **Figure 2**, respectively.

# FIGURE 1 SITE LOCATION WITHIN A LOCAL CONTEXT



Source: UBD's Australian City Streets - Version 4

FIGURE 2
SITE LOCATION WITHIN AN AERIAL CONTEXT



Source: Google Earth (accessed 11/03/20)

#### 1.4.2 Site Description

The subject site provides a real property description of Lot 58 and 59 DP 33490 and a street address of 36 and 38 Rodley Avenue, Penrith. Collectively, the allotments form a rectangular shaped parcel of land, providing an approximate frontage of 30.5m to Rodley Avenue. The site extends to the south away from Rodley Avenue some 36.5m, providing a total area of approximately 1,113m<sup>2</sup>.

#### 1.4.3 Existing Site Use

The subject site currently accommodates two detached residential dwellings and associated outbuildings. Each dwelling is serviced by vehicular access driveway connecting separately with Rodley Avenue, situated in the north-western corner of each lot.

## 1.4.4 Surrounding Uses

The site is adjoined to the north, east and west by similar detached residential dwellings, fronting and serviced by Rodley Avenue.

A series of medium density residential developments are situated to the northwest and north-east of the site, fronting Rodley Avenue, Worth Street and Vista Street.

Further, a recently constructed residential apartment building is situated some 80m to the west of the site, providing a street address of 50 – 54 Rodley Avenue. This development, comprising 42 apartments, contained within a six storey building is serviced by an indented waste collection bay along the southern side of Rodley Avenue in a similar manner to that proposed.

Penrith Paceway adjoins the site to the south.

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# 2. PROPOSED DEVELOPMENT

#### 2.1 Built Form

The subject application seeks Council's approval to demolish the existing dwellings and the construction of residential apartment development comprising a total of 16 dwellings, made up of the following:

- 13 two bedroom dwellings; and
- 3 three bedroom dwellings

The dwellings are proposed to be contained within a five storey building located approximately central to the site.

The development is to be serviced a single level of basement parking accommodating 22 passenger vehicle spaces. Access between this parking area and Rodley Avenue is proposed via combined ingress / egress driveway located within the north-eastern corner of the site.

Pedestrian connectivity is proposed between the development and the southern Rodley Avenue footway to the west and separate from the abovementioned vehicular access driveway.

Further to the above internal site works, the proposal involves the construction of an indented bay within the southern Rodley Avenue footway. This bay, providing approximate dimensions of 22.5m x 2.8m, is proposed to partially encompass the passenger vehicle driveway but be primarily located to the west, to accommodate waste collection activities associated with the proposed development.

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# 3. SITE ACCESS & INTERNAL CIRCULATION

### 3.1 Passenger Vehicular Access

#### 3.1.1 Driveway Design

Vehicular access between the development site and Rodley Avenue is proposed to be provided via a single combined ingress / egress driveway provided within the north-eastern corner of the site. The access driveway is proposed to provide a width of 7.4m.

This driveway is proposed to provide direct connectivity to an internal roadway which provides a 3m wide ingress lane separated from a 3.1m wide egress lane by a 0.7m wide median.

AS2890.1:2004 provides driveway design specifications based on the proposed primary land use, the functional order of the access road and the number of spaces the driveway is to serve. Tables 3.1 and 3.2 of AS2890.1:2004 specify that a Category 1 type driveway is required, providing a combined ingress / egress driveway width of between 3m and 5.5m based on the local (non-arterial) nature of Rodley Avenue, the residential land-use and the on-site passenger vehicle parking provision of less than 25 spaces. The proposed 7.4m wide combined ingress / egress driveway therefore exceeds the minimum AS2890.1-2004 specifications.

Swept path plans have been prepared in order to demonstrate the ability of passenger vehicles to enter and exit the site, copies of which are included as **Appendix 2**. These swept paths also indicate that an entering vehicle is capable of wholly being accommodated within the site prior to being required to stop.

The safety and efficiency of access / egress movements are also proposed to be assisted by the provision of a relatively level (maximum of 1:20) grade within the first 6m inside the property boundary.

The consistency of the horizontal and vertical alignment of Rodley Avenue in the vicinity of the subject site results in a good level of sight distance prevailing between the driveway and the frontage road. Sight distance between vehicles exiting the site and Rodley Avenue to the west of the site is proposed to be assisted by the provision of a triangle adjoining the southern side of the driveway measuring 2.5m into the site and 2.0m along the boundary being free of obstructions to visibility in accordance with the relevant AS2890.1:2004 requirements. In consideration of this and the abovementioned discussion, the proposed site access arrangement is considered to be satisfactory.

#### 3.1.2 Basement Access Ramp

Connectivity between the site access driveway and the basement car parking area is proposed via a roadway / ramp running along the eastern site boundary. This ramp is to provide a width of between 6m and 6.8m for the first 6m inside

the property prior to reducing in width to a minimum of 3m for a length of approximately 15m prior to linking with the basement parking area.

It is acknowledged that the narrow section of the access ramp is not capable of accommodating two-way traffic movements simultaneously. This roadway however suitably accords with Clause 3.2.2 of AS2890.1:2004, which allows for a two-way driveway and connecting roadway width of 3m, where the two directional traffic volume is less than 30 movements per hour.

Section 5.1 of this report presents that the access ramp / driveway could be expected to accommodate up to 9 peak hour vehicle movements based on the development yield, being significantly less than the abovementioned maximum of 30 movements. Accordingly, the width of the access driveway and connecting roadway is only required to accord with the one-way traffic requirements as specified within Clause 2.5.2 (a) (i) of AS2890.1:2004, which requires a minimum roadway width of 3m. Compliance with this Clause is achieved.

Notwithstanding the above, it is noted that the access driveway and immediately connecting internal roadway has been designed to be capable of accommodating an entering and exiting vehicle simultaneously. Entering vehicles are therefore able to wait wholly within the property in the event of a vehicle exiting the development at the same time.

#### 3.1.3 Basement Access Ramp Management

Notwithstanding the findings of Section 3.1.2 of this report, it is proposed to provide for an internal traffic signal system to limit the direction of traffic flow within the basement access ramp to one-way at any given time. The traffic signal system is to utilise red / green traffic lanterns located at the access driveway and within the basement level. The lantern within the access driveway are to be supplemented with 'Stop Here on Red' signage and a stop line located clear of the access ramp and roadway thereby allowing vehicles to safely manoeuvre to / from the roadway / ramp clear of queued vehicles.

The default position will display a green to the movement for entering vehicles from Rodley Avenue and a red display for vehicles exiting the parking area. Under this arrangement, when vehicles approach the site, they will be provided with a green display and move towards the parking areas in an unimpeded fashion. An input is to be received by the operating system from radar direction units as the vehicle travels towards the parking areas.

Motorists wishing to exit the basement parking levels will activate the internal traffic signals via in-vehicle remotes (similar to a roller door remote) or a push button situated within the within the basement lift lobby. Visitors exiting the designed parking spaces within the upper basement parking area (whom will not hold an in-vehicle remote) will be required to utilise this push button system to activate the traffic signals and thence exit the site. Signage will be provided within the basement specifying that vehicles are not to exit car spaces until a green lantern is displayed within the basement to ensure there is no undesirable conflict within the basement circulation aisles.

Upon activation of the remote / push button, the operating system will then display a red to the site entrance lantern while the vehicle/s wishing to exit the parking area will remain red. The operating system will have recorded any vehicles already in internal access roadway / ramp via radar detection units and commenced a timer to allow a vehicle to complete its journey into the parking area. The display for the activated traffic signal within the specific parking area will then change from red to green thereby allowing vehicles to safely exit the parking space within the basement parking level and travel unimpeded towards the site access driveway. When the directional sensitive radar unit located at the driveway is activated by the exiting vehicle/s, the system returns to the default position.

The indicative location of the entrance stop line, vehicle detector, lanterns and push button are illustrated on the amended architectural plans. Notwithstanding this, the specific details of the internal traffic signal system are typically specified by traffic signal contractors at construction certificate stage, complete with a management plan, including measures to be implemented during malfunctions or blackouts.

In regard to the above ,traffic signal systems such as that described above are typically fitted with a battery powered back up system to ensure that they continue to operate during power black outs. It is further understood that traffic signal systems such as that proposed tend to be very reliable and rarely malfunction. However, in the event of a malfunction occurring, the manufacturer provides a maintenance crew which is on call 24 hours per day, which will be dispatched to the site immediately. Further, the system incorporates a computer which in most cases can self-diagnose a problem and inform service personnel who can often fix the issue remotely via the internet.

The requirement for detailed design of the traffic signal system, including operational management measures to be implemented, could reasonably be imposed by Council as a condition of development consent.

This Practice notes that internal development traffic signal systems have been successfully implemented within numerous similarly sized residential developments throughout the Sydney metropolitan area, the closest of known example of which is situated a 8 John Tipping Grove, Penrith.

Incorporating such an internal traffic signal system, the proposed single lane vehicular ramp servicing the development is envisaged to be satisfactory.

#### **3.1.4** Likelihood of Delay

In order to further assess the suitability of the proposed traffic signal system governing the basement access ramps, an assessment is made with respect to the likelihood that an entering vehicle will be delayed. It has previously been presented that the traffic signals are provide a green signal for entering traffic in default mode such that entering vehicles are generally not required to be delayed. Notwithstanding this, when a vehicle wishes to exit the basement level, the signal governing vehicle entry will change to a red until such time as the

vehicle exiting the site has completed its travel from the basement to the egress driveway.

The likelihood of delay in this case is a function of opposing traffic demands and service time. The following discussion provides an assessment of delay on the basis of these development parameters.

Section 5.1 of this report estimates that the development will generate up to nine vehicle movements to and from the site during weekday commuter peak hours. For the purposes of this assessment, these vehicle trips are expected to comprise 2 inbound and 7 outbound trips during the morning peak period and 7 inbound and 2 outbound trips during the evening peak hour.

The service time is the time it takes for a vehicle to travel between the basement parking area and the egress driveway. The exiting vehicle is expected to take up to 30 seconds to travel between the basement parking level and the egress driveway, however in order to further generate an absolute worst case scenario, the service time is assumed to be 60 seconds.

On the basis of the above, **Table 1** provides an assessment of the probability of vehicles being delayed when entering the site, by a vehicle exiting the site.

TABLE 1 ANALYSIS OF PROBILITY OF DELAY BETWEEN INBOUND / OUTBOUND VEHICLES DURING WEEKDAY COMMUTER PEAK HOURS			
MORNING PEAK HOUR	Outbound	Inbound	
Arrivals at Conflict Area	7 vehicles / hour	2 vehicles / hour	
Travel Time Through Conflict Area	60 seconds	60 seconds	
Probability of Vehicle in Conflict Area	11.7%	3.3%	
Probability of Opposing Vehicles in Conflict Area	0.4%		
EVENING PEAK HOUR			
Arrivals at Conflict Area	2 vehicles / hour	7 vehicles / hour	
Travel Time Through Conflict Area	60 seconds	60 seconds	
Probability of Vehicle in Conflict Area	3.3%	11.7%	
Probability of Opposing Vehicles in Conflict Area	g Vehicles in Conflict Area 0.4%		

**Table 1** indicates that there is a 0.4% probability (approximately 1 in 250 chance) of an entering vehicle being delayed by an exiting vehicle during weekday commuter peak periods. It is accordingly most unlikely that there will be any unreasonable delay for entering vehicles, thereby ensuring that entering vehicles are most unlikely to queue from the site into the southern Rodley Avenue footpath or indeed, the adjoining public roadway pavement.

#### 3.2 Pedestrian Access Arrangements

Pedestrian connectivity between the building and the southern Rodley Avenue footway is proposed via pathway separate and to the west of the abovementioned access driveway. Further to this access, pedestrian connectivity between all parking spaces within the basement parking levels and the building is proposed via lift and stairs located approximately central to the subject site.

# 3.3 Parking Provision

#### 3.3.1 Vehicular Parking Provision

The development is serviced by a total of 22 off-street passenger vehicle parking spaces provided as follows:

Visitor spaces 2 Visitor / wash / service space 1

Resident spaces 19 (including 2 adaptable)

**Total** 22 spaces

#### 3.3.2 Council's Vehicular Parking Requirements

Penrith City Council provides the following locally sensitive parking requirements for residential flat building with DCP 2014:

1 space per 1 or 2 bedrooms

2 spaces per 3 or more bedrooms

1 space per 40 units for service vehicles

1 space per every 5 dwellings, or part thereof for visitors

1 space for car washing for every 50 units, up to a maximum of 4 spaces per building

**Table 2** below provides the off-street parking requirements based on the above Penrith City Council's car parking rate.

TABLE 2 OFF-STREET PARKING REQUIREMENTS PENRITH DCP 2014				
Item	Rate	No.	Spaces Required	
1 or 2 bedroom dwellings	1 space per dwelling	13	13	
3 bedroom dwellings	2 space per dwelling	3	6	
Service vehicles	1 space per 40 dwellings	16	0.4 (adopt 1)	
Visitor Parking	1 space per 5 dwellings	16	3.2 (adopt 3)	
Car Washing	1 space per 50 units	16	0.3 (adopt 1)	
		Total	24	

**Table 2** indicates that DCP 2014 requires the development provide a total of 24 passenger vehicle parking spaces, comprising 19 resident, three visitor, one service and one car wash space.

The proposed total passenger vehicle parking provision of 22 spaces therefore represents a numerical parking shortfall of two spaces in accordance with DCP 2014.

The following is however acknowledged:

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- The service vehicle, washing bay and third visitor parking space have been combined in a single parking space; and
- The visitor parking requirement of 3.2 spaces has been rounded down to three spaces.

The combining of the service bay and wash bay is considered to be satisfactory given the sporadic nature of the use of such bays. Further the rounding down of the visitor parking calculations is supported as the abovementioned service / wash bay could reasonably be utilised as by visitors if so required. In consideration of this, the proposed passenger vehicle parking provision and allocation is considered to be satisfactory.

### 3.3.2 Motorcycle Parking

The subject development is to provide one motorcycle parking space within the upper basement level.

Penrith City Council's DCP 2014 does not prescribe motorcycle parking rates for residential developments. Notwithstanding this, the proposed provision of one motorcycle space is considered to be appropriate and satisfactory.

# 3.3.3 Bicycle Parking

The subject development is to provide two bicycle storage racks capable of accommodating up to four bicycles within Basement Level 2.

Penrith Council refers to NSW Government's *Planning Guidelines for Walking and Cycling* 2004 with respect to the provision of bicycle parking. This publication provides the following recommendations relevant to the subject proposal:

#### Resident

20% of units should provide a space

#### **Visitors**

5% of units should provide a space

Based on 16 dwellings, the NSW Government's *Planning Guidelines for Walking and Cycling* recommends resident and visitor bicycle parking provision of 3.2 (adopt three) resident and 0.8 (adopt one) visitor parking spaces or a total of four spaces.

The proposed provision of four on-site bicycle parking spaces therefore complies with the requirement specified in the NSW Government's guidelines and accordingly, is considered to be satisfactory.

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#### 3.4 Internal Circulation and Manoeuvrability

#### 3.4.1 Site and Basement Access Ramps

Connectivity between the site access driveway and the basement car parking area is proposed via a roadway / ramp running along the northern site boundary. This ramp is to provide a minimum width of 6m for the first 6m inside the property prior to reducing in width to a minimum of 3m for an approximate length of 15m prior to linking with the upper basement parking area.

It is acknowledged that the narrow section of the access ramp is not capable of accommodating two-way traffic movements simultaneously. The access ramps and their connections to the access driveway and the basement parking levels are therefore proposed to be governed by traffic signal control to ensure that only one direction of traffic is accommodated at any one time, the suitability of which has been discussed in previous sections of this report.

The site and basement access ramp grades have been designed to accord with the relevant requirements of AS2890.1:2004, providing the following characteristics:

- Maximum ramp grade = 1 in 4;
- Maximum change in grade = 1 in 8; and
- Maximum ramp grade for the first 6m inside the site = 1 in 20.

In consideration of this and the above discussions, the site and basement access ramp is considered to be satisfactory.

#### 3.4.2 Basement Car Park

Passenger vehicles, upon entry to the site, will travel in a forward direction via a ramp running along the eastern site boundary to access Basement Level 1. Basement Level 1 is proposed to contain a series of standard 90 degree angled parking spaces serviced by an adjoining parking / circulation aisle, with spaces being allocated to residents and visitors.

The basement parking areas have been designed with the following minimum dimensions in accordance with the requirements of AS2890.1:2004 and AS2890.6:2009:

- Standard vehicular parking space width = 2.4m;
- Disabled vehicular parking space width = 2.4m (with adjoining 2.4m wide shared area);
- Additional vehicular space width where parking spaces adjoins an obstruction = 0.3m;
- Standard and disabled vehicular parking space length = 5.4m;

- Vehicular parking aisle width adjoining parking spaces = 5.8m;
- Motorcycle parking space width = 1.2m;
- Motorcycle parking space length = 2.5m;
- Headroom = 2.2m; and
- Headroom above disabled parking spaces and adjoining shared areas = 2.5m.

Safe and efficient internal manoeuvring and parking space accessibility is anticipated to result, taking into consideration the above compliance with the relevant AS2890.1:2004, AS2890.3:2015 and AS2890.6:2009 specifications.

In order to demonstrate the internal passenger vehicle manoeuvrability within the vicinity of these areas and generally throughout the overall parking areas, this Practice has prepared a number of swept path plans which are included as **Appendix 2**. The turning paths provided on the plans have been generated using Autoturn software and derived from B85 and B99 vehicle specifications provided within AS2890.1-2004.

Section B4.4 of AS2890.1-2004 states the following with regard to the use of templates to assess vehicle manoeuvring:

'Constant radius swept turning paths, based on the design vehicle's minimum turning circle are not suitable for determining the aisle width needed for manoeuvring into and out of parking spaces. Drivers can manoeuvre vehicles within smaller spaces than swept turning paths would suggest.'

It would therefore appear that whilst the turning paths provided within AS 2890.1 - 2004 can be utilised to provide a 'general indication' of the suitability or otherwise of internal parking and manoeuvring areas, vehicles can generally manoeuvre more efficiently than the paths indicate. Notwithstanding this, the swept path plans illustrate that passenger vehicles can manoeuvre throughout and enter and exit the most difficult passenger vehicle parking spaces within the parking areas.

It is further noted that the basement parking area forms a dead end aisle. The alignment of the internal circulation aisle is however such that vehicles are able to turnaround in order to ensure that vehicles can exit the site in a forward direction in the event that all parking spaces are occupied. The proposed basement parking area layout as it relates to passenger vehicle manoeuvrability is therefore considered to be satisfactory.

#### 3.5 Site Servicing

The subject site is anticipated to generate the requirement for regular waste collection vehicle servicing. Waste collection vehicles are proposed to service the site via an indented bay provided within the southern Rodley Avenue footway, situated to the immediate west of the development vehicular access driveway.

Garbage bins are proposed to be contained within a storage room located within Basement Level 2. These bins are to be transferred to a holding room located approximately central to the site frontage to Rodley Avenue, which can efficiently be wheeled to the proposed indented collection bay within the southern public road footway.

The proposed indented bay provides the following design criteria:

- A width of 2.8m, measured from the existing kerb line to the new kerb line, for a length of 15m;
- A 7m ingress splay / manoeuvring space at the rear of the bay, measured along the existing kerb line (and partially encompassing the development access driveway);
- A 4m egress splay / manoeuvring space at the front of the bay, measured along the existing kerb line;
- A 1.2m wide footpath within the southern Rodley Avenue footway between and providing a minimum separation of 0.8m to the northern site boundary and the indented bay; and
- A 1.8m wide kerb ramp to facilitate wheeled bin access to the rear of the indented bay.

It is acknowledged that detailed design of the above arrangements, and any supplementary signage and line marking arrangements, will need to be submitted to and approved by Council through a Section 138 Application, including the payment of application and inspection fees. The requirement for this can reasonably be imposed by Council as a condition of consent.

It is further acknowledged that Council's PDCP 2014 specifies that it is desirable that development design facilitate the provision of on-site collection of waste by Council's 10.5m long collection vehicle and facilitate forward entry and exit movements between the site and the adjoining public road network. The DCP however also specify that on-site collection may not be practicable for all development sites due to site dimensions, topography or the resultant associated undesirable planning outcome achieved.

The above proposed waste collection arrangements have accordingly been formulated following a detailed assessment of potential on-site collection arrangements in accordance with Council's DCP. These alternate collection arrangements have also being the subject of various discussions with Council staff through pre-lodgement meetings / assessment and reviews by Council's

Urban Design Panel both during the current and the previous development application.

In regard to the above, alternate development schemes have been prepared for discussions with Council officers which incorporated the provision of an at-grade heavy vehicle turntable within the south-eastern corner of the site and a dedicated waste collection area within the basement. These alternate on-site collection areas were to be accessed via driveways both separate to and combined with the passenger vehicle access driveway.

Swept path assessment of Council's collection vehicle with respect to the above alternate design schemes illustrated undesirable interaction with public road traffic flow and parking areas as well as vehicular access / egress movements associated with the development basement parking areas.

In comparison, swept path plans which have been prepared by this Practice, copies of which are attached as **Appendix 2**, demonstrate that Council's 10.5m long collection vehicle is capable of manoeuvring to and from the proposed indented bay without unreasonable encroachment on public road traffic flow and parking areas, also being clear of the development access driveway servicing the basement parking areas.

Whilst it is acknowledged that the proposed development access driveway and indented bay will result in the loss of approximately four on-street parallel parking spaces, a greater loss of approximately six on-street parking spaces would eventuate from the excessively wide driveways and manoeuvring which would be required in the event that collection vehicles were required to enter and exit the site. Graphical representations of the abovementioned existing and proposed modified on-street parking arrangements within Rodley Avenue are attached as **Appendix 3**.

It should further be acknowledged that any on-street reduction in parking would be limited to that immediately adjacent to the subject site. All adjoining properties provide formal off-street parking infrastructure and additional adjacent on-street kerb-side if so required. Indeed, recent observations have indicated that whilst demand for on-street parking within Rodley Avenue is notable, capacity exists to accommodate the minor reduction in supply. It is accordingly not expected that the minor loss of on-street parking supply will unreasonably impact adjoining residential amenity. In fact, it could be argued that the provision of the indented bay will provide for additional roadway capacity by providing additional pavement width to allow oncoming vehicles to pass.

Importantly, it should be noted that the proposed off-site collection arrangements and design are consistent with that recently approved and constructed residential apartment development at 50 – 54 Rodley Avenue via DA16/0262. Council's Major Assessment Report for this development dated 16 March 2016 specified the following:

Despite this department not generally accepting the provision of an on-street loading bay, in the circumstances of this case it is the most suitable solution

for the development given manoeuvring through the basement with a heavy vehicle is difficult to achieve. The amended proposal provides proximity to the collection area and to the basement driveway, and allows for the waste vehicle to enter and exit the property (bay) in a forward direction as required.

Whilst it is acknowledged that the proposed indented bay provides for as reduction in footway width, the 6m width of footway facilitates the retention of 3.1m of usable footway in conjunction with the bay, which is considered to be readily capable of accommodating a formalised pedestrian footpath in conjunction with street trees as necessary.

It is further acknowledged that proposed indented bay will necessitate the alterations to existing unrestricted on-street parking, which requires approval by Transport for NSW given the proximity of the site to a railway station. Such a requirement could reasonably be imposed by Council through the implementation of a condition of consent in a similar manner to that applied to other recently approved and constructed developments providing indented waste collection bays.

In consideration of the above and the previous discussion, the proposed waste collection arrangements are therefore considered to be satisfactory and consistent with that previously approved within Rodley Avenue.

# 4. EXISTING TRAFFIC CONDITIONS

## 4.1 Surrounding Road Network

The following provides a description of the road network surrounding the subject site:

 Rodley Avenue performs a local access road function under the care and control of Penrith City Council. It provides an east-west alignment connecting the southbound Mulgoa Road carriageway in the west with Worth Street approximately 60m to the east of the site. Rodley Avenue extends a further 160m to the east of Worth Street, at which point it forms a terminating culde-sac.

Rodley Avenue provides an approximate pavement width of 7m providing one through lane of traffic in each direction in conjunction with parallel parking along both kerb alignments. Traffic flow is governed by a sign posted speed limit of 50km/h.

Rodley Avenue provides a half road closure to the west on immediate approach to the Mulgoa Road southbound carriageway, from which left turn entry movements only are facilitated.

Rodley Avenue forms a T-junction with Worth Street to the east, operating under major / minor priority control with Rodley Avenue performing the through route.

Worth Street provides a north-south collector function, also under the care
and control of Penrith City Council. It provides a connection between High
Street and Penrith Plaza in the north, intersecting with which under traffic
signal control, and Rodley Avenue in the south. Worth Street also intersects
with Union Road under traffic signal control, some 150m to the north of
Rodley Avenue.

Worth Street, between Rodley Avenue and Union Road, provides a 7m wide pavement providing one through lane of traffic in each direction, in conjunction with indented parallel passenger vehicle parking bays. To the north of Union Road, Worth Street forms a 13m wide pavement providing two though lanes of traffic in each direction.

Union Road provides an east-west collector function, linking Station Street in
the east with Mulgoa Road in the west. Union Road provides a 13m wide
pavement, primarily providing one through lane of traffic in each direction in
conjunction with parallel parking along both kerb alignments. Parking
restrictions apply on immediate approach to and departure from Worth
Street, facilitating two through lanes on approach and departure from the
signalised intersection.

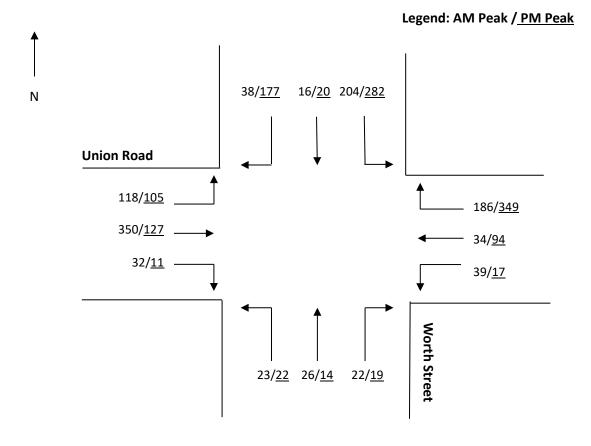
Union Road intersects with Station Street (and The Broadway) under single lane circulating roundabout control to the east. To the west, Union Road intersects with Mulgoa Road under major / minor priority control with Mulgoa Road performing the priority route. Whilst a dedicated right turn lane and break is provided within the Mulgoa Road central median, facilitating right turn movements to Union Road, right turn egress movements from Union Road are prohibited.

## 4.2 Existing Traffic Volumes

Staff of Stanbury Traffic Planning have undertaken surveys of the intersection of Union Road and Worth Street in order to accurately ascertain the traffic demands. Surveys were undertaken between 7:00am – 9:00am and 4:00pm – 6:00pm on the 28<sup>th</sup> of May 2018. Whilst it is acknowledged that these surveys are now almost two years now approach two years old, recent check observations indicate that the survey results remain valid.

**Figure 3** provides a summary of the surveyed commuter peak hour (8:00am – 9:00am and 4:00pm – 5:00pm) traffic flows at the intersections whilst full details are contained within **Appendix 4** for reference.

FIGURE 3
EXISTING WEEKDAY COMMUTER PEAK HOUR TRAFFIC VOLUMES
INTERSECTION OF UNION ROAD & WORTH STREET



#### Figure 3 illustrates the following:

- Worth Street, to the south of Union Road, accommodates directional traffic demands of less than 100 vehicles per hour;
- Worth Street, to the north of Union Road, accommodates directional traffic demands of 300 – 500 vehicles per hour; and
- Union Road accommodates directional traffic demands of 300 500 vehicles per hour.

### 4.3 Existing Road Network Operation

#### 4.3.1 Intersection Operation

The surveyed intersection of Union Road and Worth Street has been analysed utilising the SIDRA computer intersection analysis program in order to objectively assess the operation of the surveyed intersections. SIDRA is a computerised traffic arrangement program which, when volume and geometrical configurations of an intersection are imputed, provides an objective assessment of the operation efficiency under varying types of control (i.e. signs, signal and roundabouts). Key indicators of SIDRA include level of service where results are placed on a continuum from A to F, with A providing the greatest intersection efficiency and therefore being the most desirable by the Roads and Maritime Services.

SIDRA uses detailed analytical traffic models coupled with an iterative approximation method to provide estimates of the abovementioned key indicators of capacity and performance statistics. Other key indicators provided by SIDRA are average vehicle delay, the number of stops per hour and the degree of saturation. Degree of saturation is the ratio of the arrival rate of vehicles to the capacity of the approach. Degree of saturation is a useful and professionally accepted measure of intersection performance.

SIDRA provides analysis of the operating conditions that can be compared to the performance criteria set out in **Table 3** (being the RMS NSW method of calculation of Level of Service).

TABLE 3  LEVEL OF SERVICE CRITERIA FOR INTERSECTIONS  SIGNALISED INTERSECTIONS				
Level of Service	Average Delay per Vehicle (secs/veh)	Expected Delay		
SIGNALISED INTERSECTIONS AND ROUNDABOUTS				
Α	Less than 14	Little or no delay		
В	15 to 28	Minimal delay and spare capacity		
С	29 to 42	Satisfactory delays with spare capacity		
D 43 to 56 Satisfactory but near capacity				
E	57 to 70	At capacity, incidents will cause excessive delays		
F > 70 Extreme delay, unsatisfactory				

The existing conditions have been modelled utilising the peak hour traffic volumes presented within **Figure 3**. **Table 4** provides a summary of the SIDRA output data whilst more detailed summaries are included as **Appendix 5**.

TABLE 4			
SIDRA OUTPUT – EXISTING WEEKDAY PEAK HOUR PERFORMANCE INTERSECTION OF UNION ROAD & WORTH STREET			
	AM	PM	
Average Vehicular Delay (seconds)	26.7	29.9	
Degree of Saturation	0.54	0.71	
Level of Service	В	С	

**Table 4** indicates that the intersection of Union Road and Worth Street currently operates with a satisfactory level of service with spare capacity.

#### 4.3.2 Rodley Avenue Performance

Rodley Avenue accommodates low traffic demands commensurate with its access function within the local road hierarchy. In this regard, directional traffic demands have been observed to be less than one vehicle every minute. The Roads & Maritime Services' *Guide to Traffic Generating Developments* specifies that a two-way two-lane roadway accommodating directional traffic demands of less than 200 vehicles per hour provides a level of service 'A' during peak periods, representing free flow where drivers are virtually unaffected by others in the traffic stream.

Notwithstanding the above, the 7m width of the Rodley Avenue pavement in conjunction with the prevalence of kerb side parking along both alignments results in one direction of traffic flow being permitted at any one time. Opposing traffic flow therefore has been observed to occur under courtesy conditions whereby a vehicle will utilise a break in kerb side parking, generally associated with a driveway, to stop and allow a vehicle travelling in the opposite direction to pass. The low traffic demands combined with general diligence displayed by motorists has been observed to ensure that two way traffic flow within Rodley Avenue occurs in a reasonably safe and efficient manner

#### 4.3.3 Abutting Site Access Movements

Traffic movements from abutting development sites and Rodley Avenue have been observed to occur in a safe and efficient manner. These movements are assisted by the low traffic demands within Rodley Avenue resulting in regular and extended gaps in traffic flow, allowing motorists to undertake turning movements from abutting development driveways without unreasonable delay.

The proliferation of driveways servicing abutting development sites in the immediate vicinity result in trailing through traffic movements within Rodley Avenue being aware of the potential for vehicles to decelerate to access private properties. This situation, combined with the consistent vertical and horizontal alignment of Rodley Avenue in the immediate vicinity, results in vehicles being able to undertaken entry and egress movements between private abutting development sites and the public road in a safe and efficient manner.

#### 4.3.4 Arterial Road Network Connectivity

The local road network provides connectivity to the surrounding regional road network as follows:

- Worth Street intersects with High Street to the north under traffic signal control with all movements being facilitated;
- High Street intersects with Mulgoa Road to the north-west under traffic signal control with all movements being facilitated;
- High Street intersects with The Northern Road and Great Western Highway to the east under traffic signal control with all movements being facilitated;
- Both Ransley Street and Jamison Road (accessed via Station Street) intersect with Mulgoa Road, to the south-west under traffic signal with all movements being facilitated;
- Union Road intersects with Mulgoa Road under major / minor control to the north-west, albeit with right turn movements from Union Road being prohibited; and
- Rodley Avenue intersects with the southbound Mulgoa Road carriageway under major / minor priority control to the west, whereby left turn movements to the local road are facilitated.

Whilst traffic demands within the surrounding state road network (High Street, Mulgoa Road, The Northern Road and Great Western Highway) are considerable, commensurate with their functional hierarchy in the road network, the abovementioned precinct access controls provide motorists with safe and efficient connectivity to the surrounding regional and state road network.

# 4.4 Public Transport

#### 4.4.1 Heavy Rail

The site is located approximately 1km walking distance to the south-west of Penrith Railway Station. Penrith Railway Station provides access to train services which operate along the T1 (North Shore, Northern & Western) Line.

The T1 Line provides regular services between Penrith and the remainder of the Sydney Trains network, servicing The Blue Mountains to the west and Blacktown, Parramatta and The City to the east.

#### 4.4.2 Buses

The following bus routes operate along High Street, with the closest stop being located approximately 500m walking distance from the site:

- Route 688 between Penrith and Emu Heights;
- Route 689 between Penrith and Leonay;
- Route 691 between Penrith and Mount Riverview; and
- Route 690P between Penrith and Springwood.

Further, the following bus routes operate along Station Street, with the closest stop being located approximately 600m walking distance from the site:

- Route 770 between Penrith and Mount Druitt;
- Route 774 between Penrith ad Mount Druitt via Nepean Hospital;
- Route 775 between Penrith and Mount Druitt via Erskine Park;
- Route 776 between Penrith and Mount Druitt via St Clair;
- Route 781 between Penrith and St Marys via Glenmore Park;
- Route 791 between Penrith and Jamisontown via South Penrith;
- Route 793 between Penrith and Jamisontown;
- Route 794 between Penrith and Glenmore via The Northern Road;
- Route 795 between Penrith and Warragamba;
- Route 797 between Penrith and Glenmore Park;
- Route 799 between Penrith and Glenmore Park via Regentville; and
- Route S13 between Penrith and Mountainview Village.

#### 4.4.3 Pedestrians / Cyclists

Pedestrians and cyclists are provided with the following access and mobility infrastructure within the immediate vicinity of the subject site:

- A footpath is provided along the northern side of Rodley Avenue;
- A footpath is provided along the western side of Worth Street between Rodley Avenue and Union Road;

- Signalised pedestrian crossings are provided over all approaches of the intersection of Union Road and Worth Street;
- Footpaths are provided along both sides of Worth Street to the north of Union Road and along both sides of Union Road;
- A shared path is provided along the eastern side of Mulgoa Road; and
- Signalised pedestrian crossings are provided over all approaches of the intersection of High Street and Worth Street.

# 5. PROJECTED TRAFFIC CONDITIONS

#### 5.1 Traffic Generation

Traffic generation rates for various land-uses have been established through extensive surveys undertaken throughout NSW and published within their *Guide to Traffic Generating Developments* and the more recently released *Technical Direction TDT 203/04a*. The following sub-sections provide a summary of the traffic generating potential of the previous and proposed site uses with respect to those rates established by the Roads & Maritime Services.

#### 5.1.1 Existing Site Uses

Section 1.3.3 of this report presented that the subject site currently contains two detached residential dwellings.

The Roads & Maritime Services' *Technical Direction TDT 203/04a* specifies average traffic generation rates of 0.95 peak hour vehicle movements per dwelling during the morning peak and 0.99 peak hour vehicle movements per dwelling during the evening peak.

For the purposes of this assessment and for reasons of simplicity, a traffic generation rate of 1 peak hour vehicle trip per dwelling has been applied to detached residential dwellings. The current site development is therefore capable of generating up to two vehicle trips to and from the site during weekday commuter peaks periods.

#### 5.1.2 Proposed Development

The proposed development involves the provision of a medium density residential apartment building, accommodating 16 dwellings. The *Guide to Traffic Generating Developments* provides the following peak hour generation rates for medium density residential development:

Medium Density Residential Dwellings 0.5 trips per one and two bedroom dwelling 0.65 trips per three bedroom dwelling.

The peak hour traffic generating ability of the proposed development is accordingly estimated to be in the order of 9 trips.

#### 5.2 Traffic Impacts

The proposed development has been projected to generate up to 9 peak hour trips to and from the site, or 7 peak hour trips over and above that currently capable of being generated by the existing two detached dwellings provided onsite. This equates to approximately one vehicle movements every 7 minutes during commuter peaks, or one additional vehicle movement every 9 minutes over and above that capable of being generated by existing site uses. Such a level of additional traffic is not projected to, in itself, result in any unreasonable

impacts on the existing operational performance of the surrounding local road network. The previous assessment contained within this report has revealed that traffic demands within the surrounding local road network are reasonably low and accordingly motorists are provided with a good level of service with space capacity.

Whilst it is acknowledged that traffic demands within the surrounding arterial road network are considerable, the positive intersection control servicing connection to / from the surrounding regional and state road network allows motorist to access and egress the local precinct in a safe and efficient manner.

In consideration of the above, the impact of the development is most likely to be a result of the safety and efficiency with which motorists are capable of entering and exiting the development. The low traffic demands within Rodley Avenue combined with the good sight distance provisions is such that it is envisaged that motorists will be capable of entering and exiting the site in a safe and efficient manner.

#### 5.4 Transport Impacts

The subject site is located within reasonably close walking distance to a number of bus services and a 10 minute walk to train services. It is accordingly expected that a proportion of the future residents within the subject development will utilise the surrounding public transport infrastructure to access destinations throughout the Sydney metropolitan area. The capacity of the existing public transport system is however not envisaged to be measurably affected by any additional demand associated with the development, given its limited scale.

# 6. CONCLUSION

This report assesses the potential traffic and parking implications associated with a residential development containing 16 dwellings at 36 – 38 Rodley Avenue, Penrith. Based on this assessment, the following conclusions are now made:

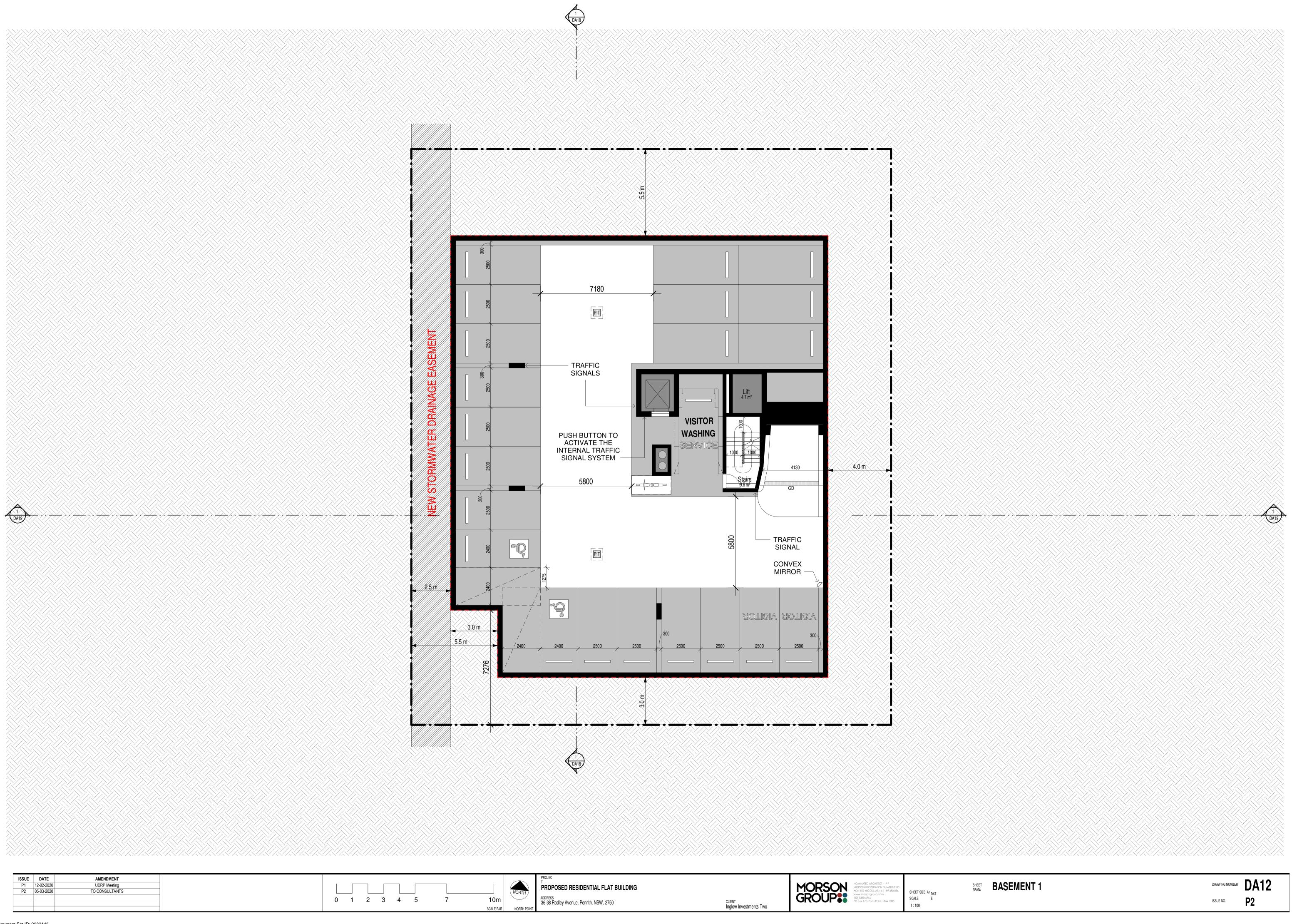
- The proposed site access arrangements are projected to result in motorists being capable of entering and exiting the subject site in a safe and efficient manner;
- The proposed off-street vehicular parking provision is considered to be satisfactory, given the requirements of DCP 2014;
- The internal passenger vehicle circulation arrangements are capable of providing for safe and efficient internal manoeuvring;
- The proposed indented bay within Rodley Avenue adjacent to the site is
  projected to safely and efficiently accommodate refuse servicing of the site
  and is consistent with that previously approved within the immediate
  precinct;
- The surrounding road network operates with a satisfactory level of service during peak periods;
- The subject development has been projected to generate up to 7 additional peak hour vehicle trips to and from the subject site over and above that capable of being generated by the existing site dwellings; and
- It is considered that the adjoining road network is capable of accommodating the traffic projected to be generated by the subject development.

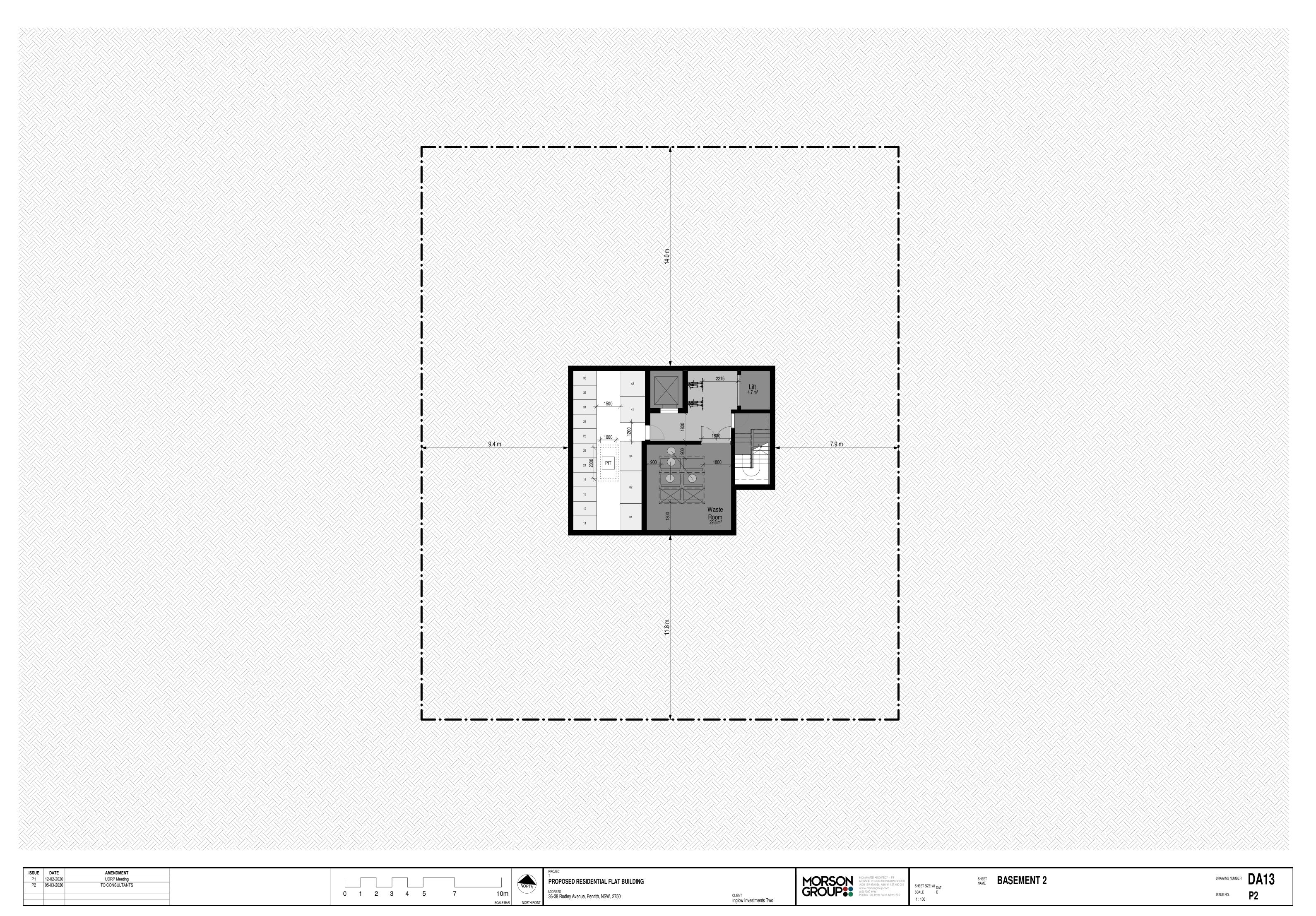
It is considered, based on the contents of this report and the conclusions contained herein, there are no traffic or parking related issues that should prevent approval of the subject application. This action is therefore recommended to Council.

# **APPENDIX 1**

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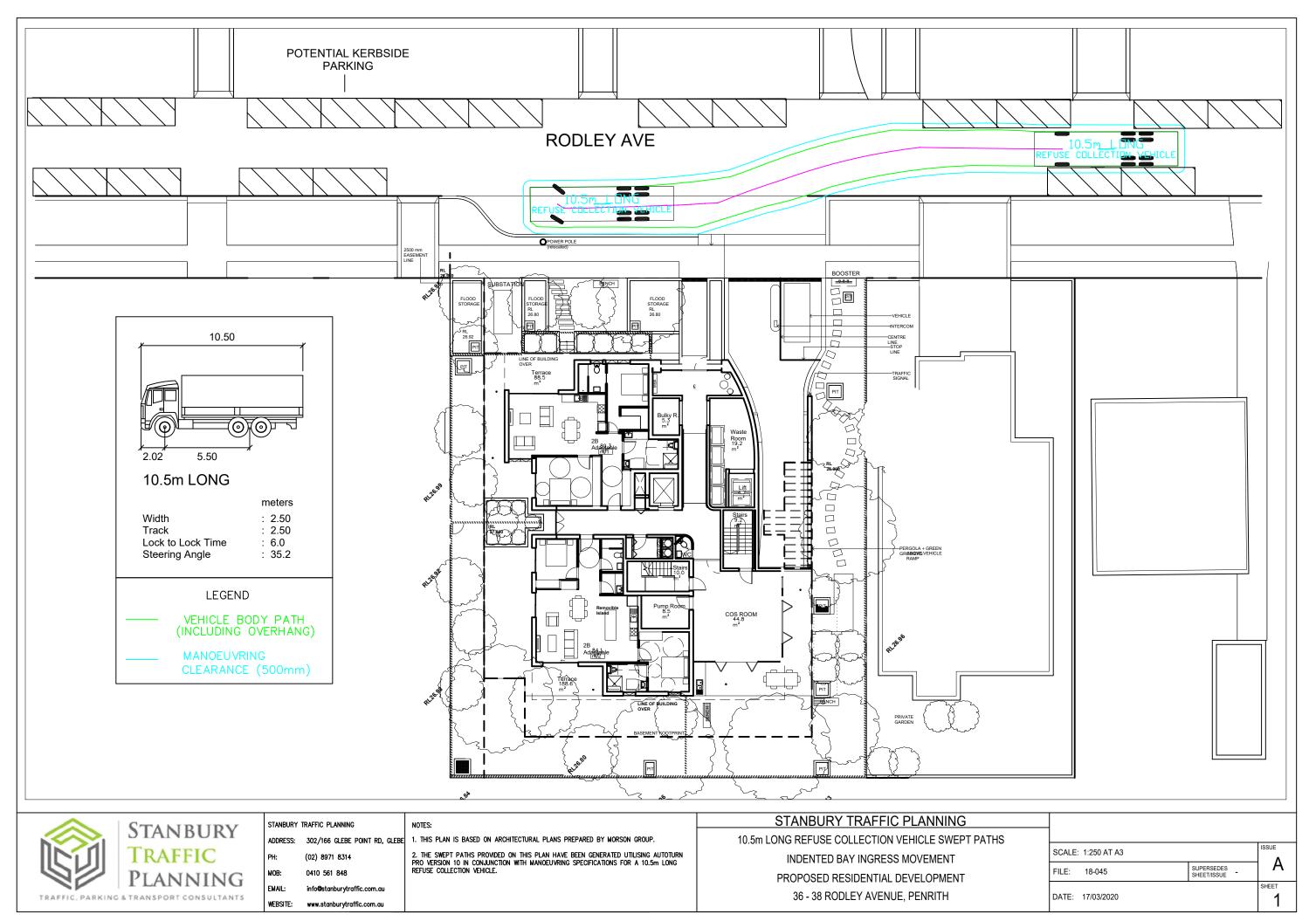


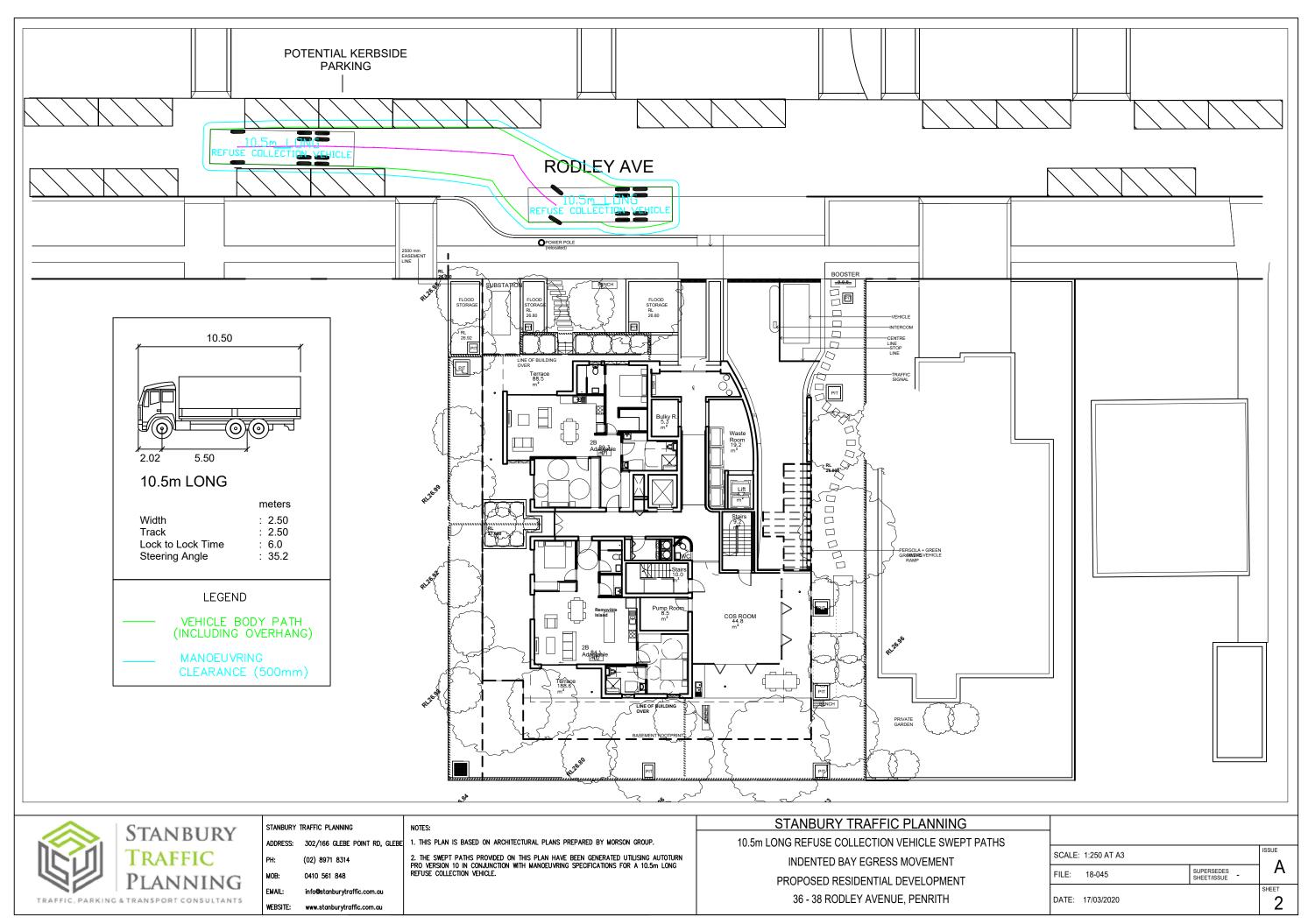


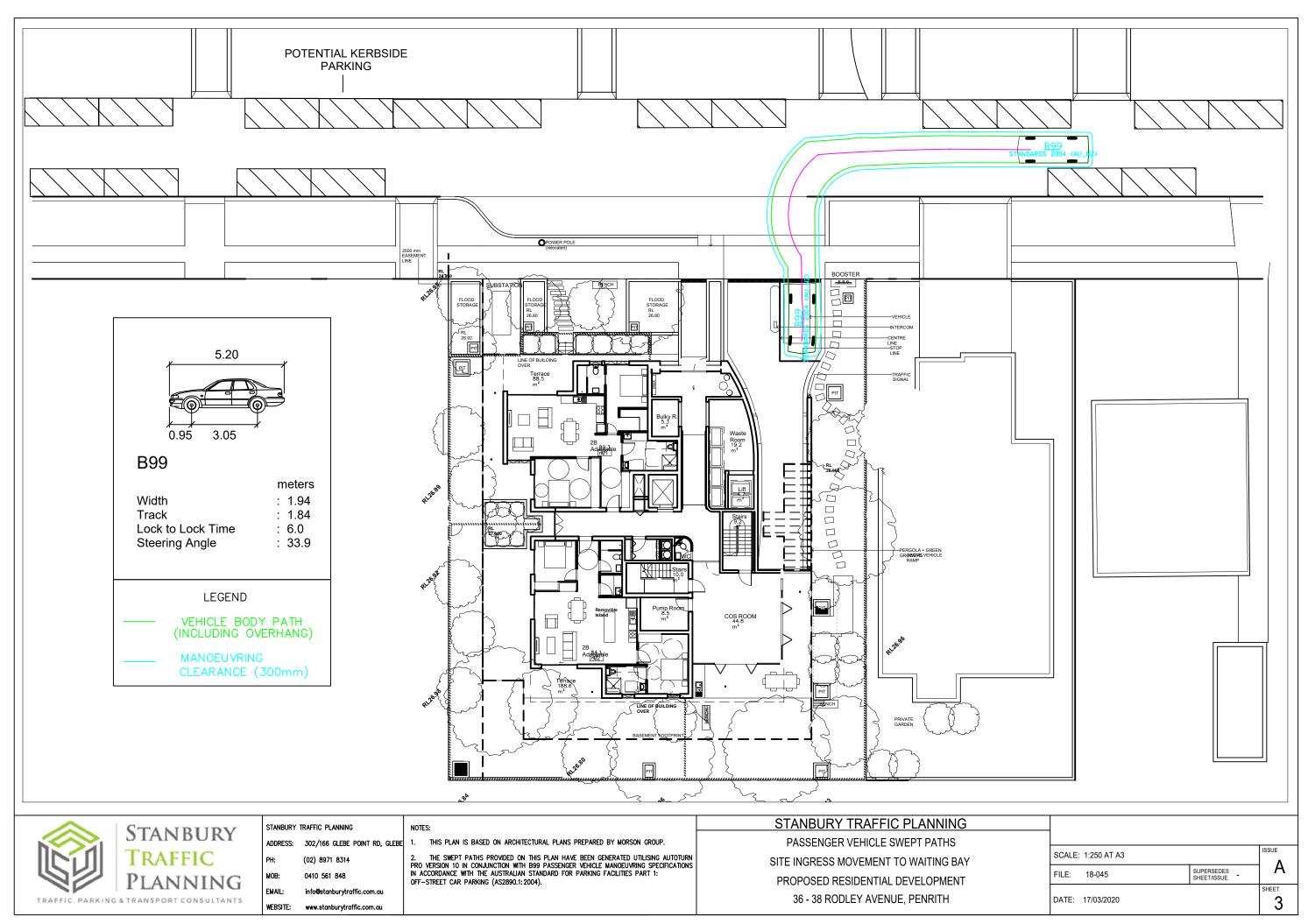


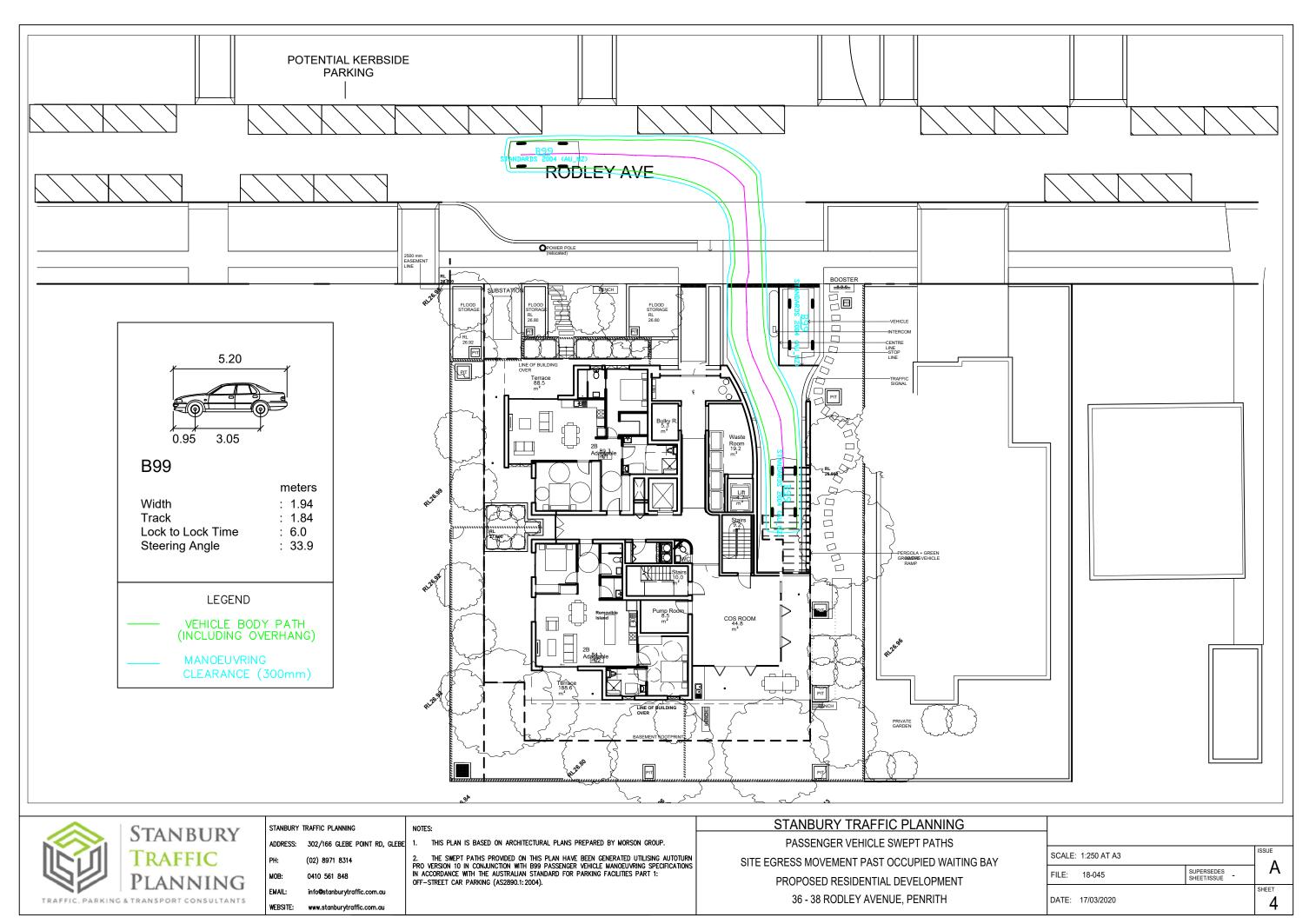
# **APPENDIX 2**

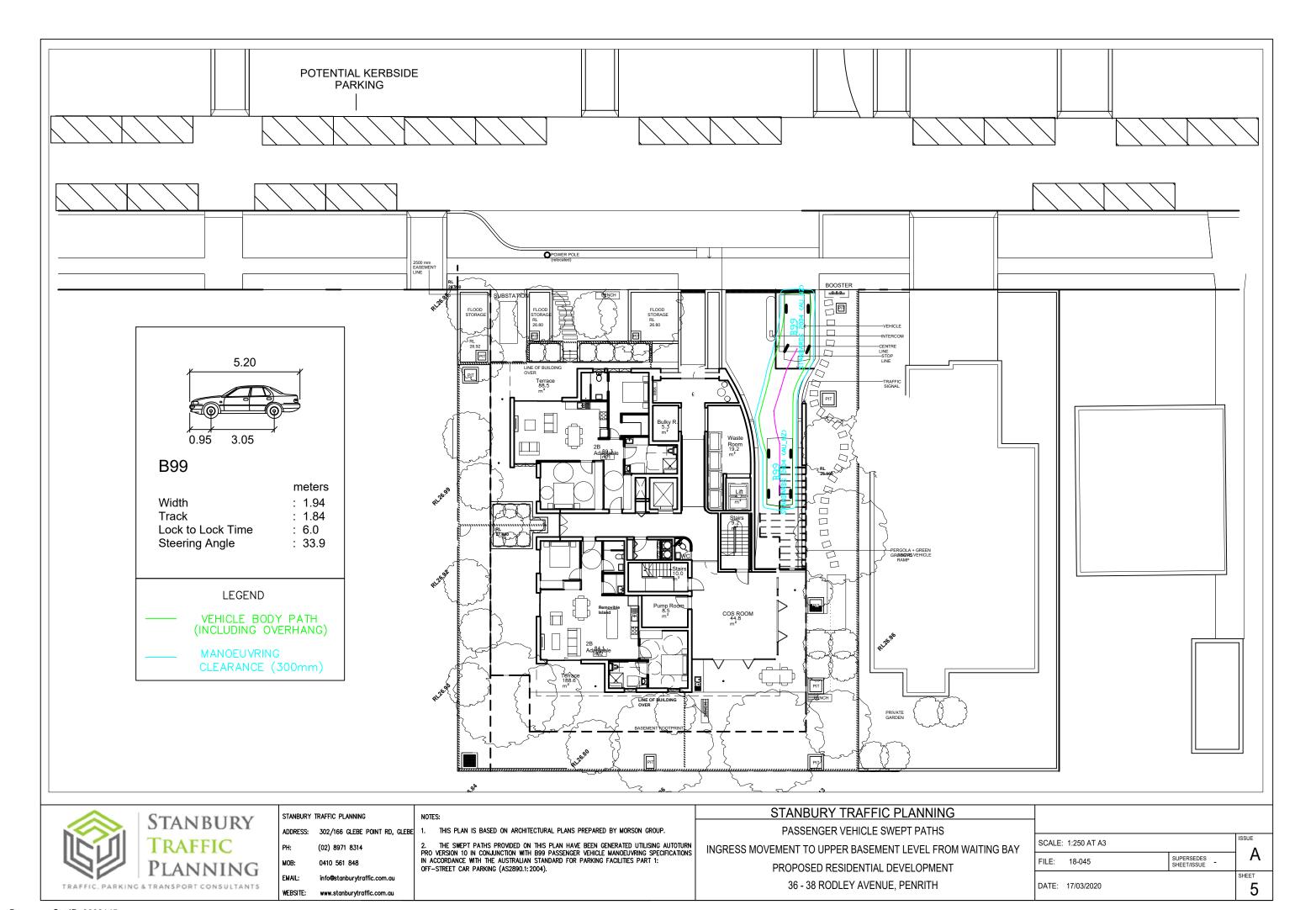
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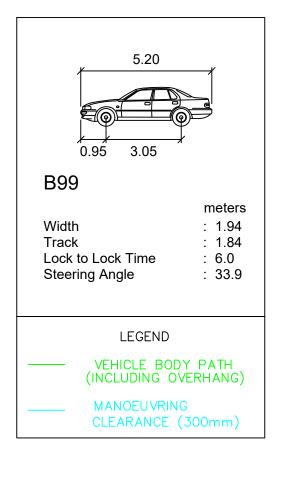


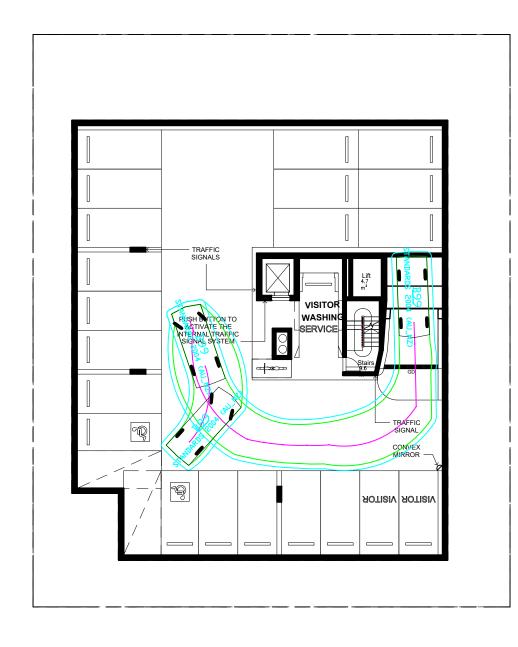


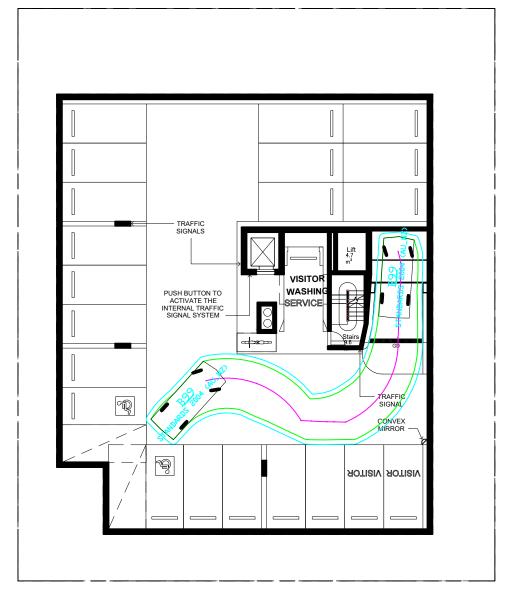














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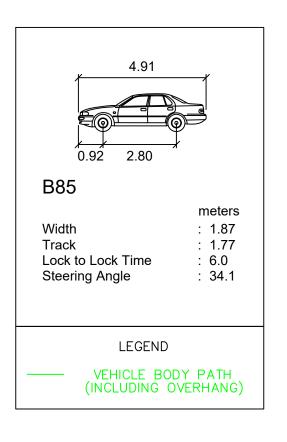
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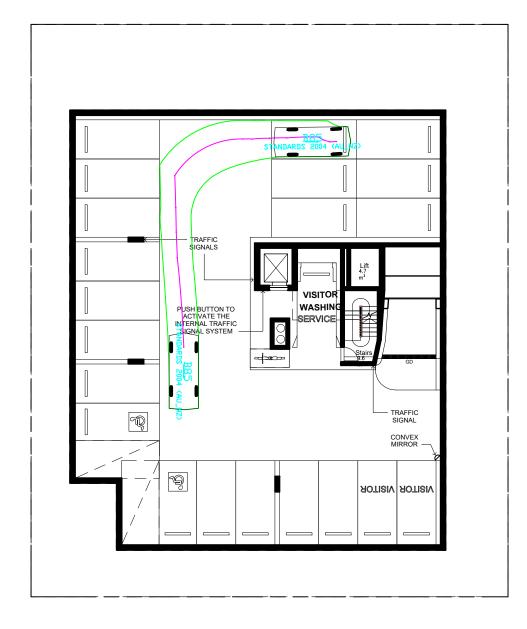
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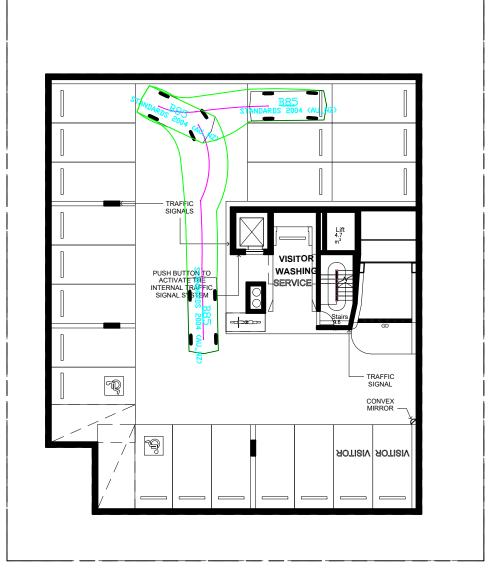
STANBURY TRAFFIC PLANNING

PASSENGER VEHICLE SWEPT PATHS UPPER BASEMENT LEVEL TURNAROUND MOVEMENTS PROPOSED RESIDENTIAL DEVELOPMENT 36 - 38 RODLEY AVENUE, PENRITH

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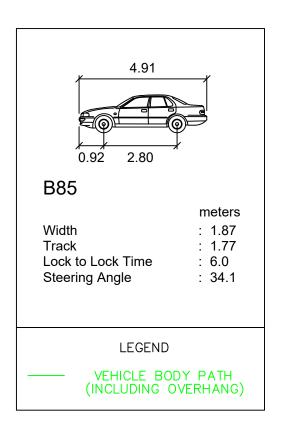
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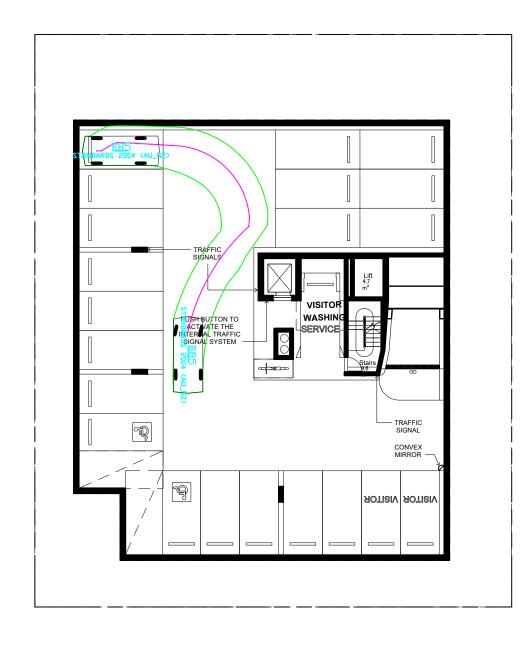
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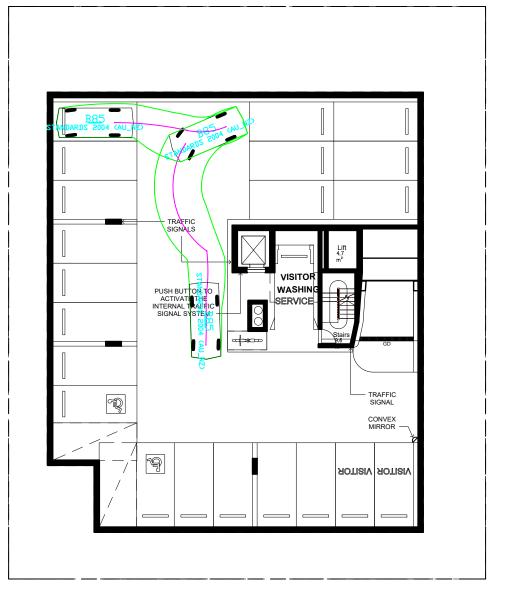
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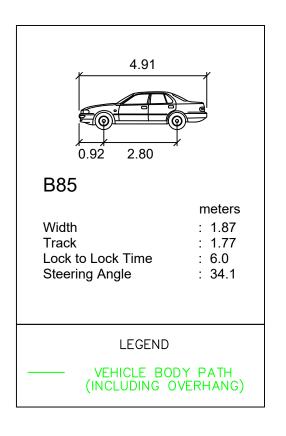
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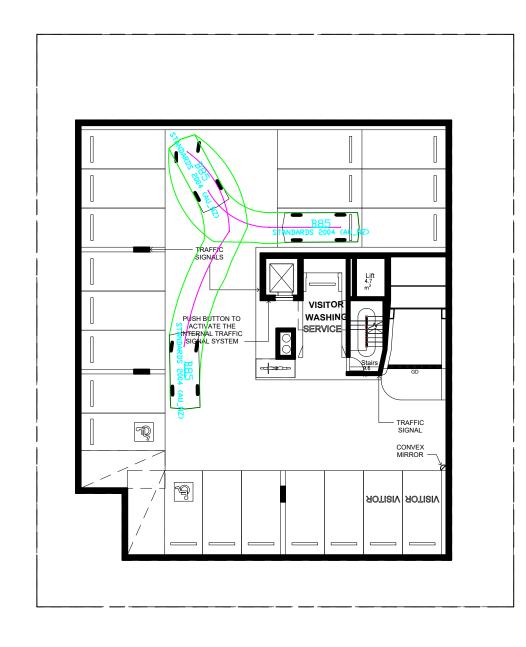
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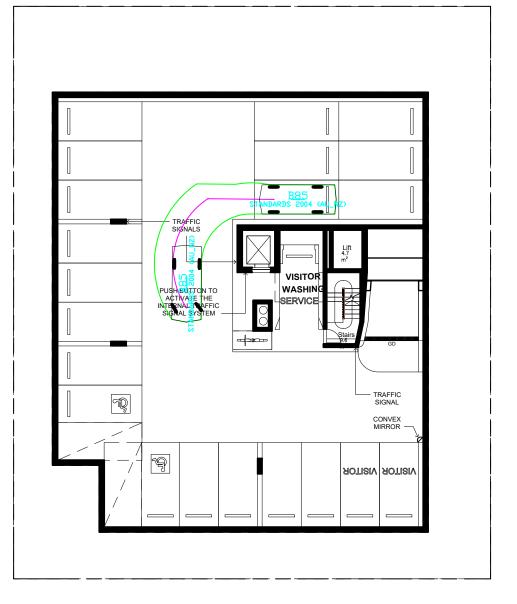
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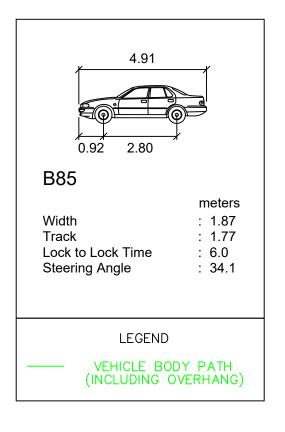
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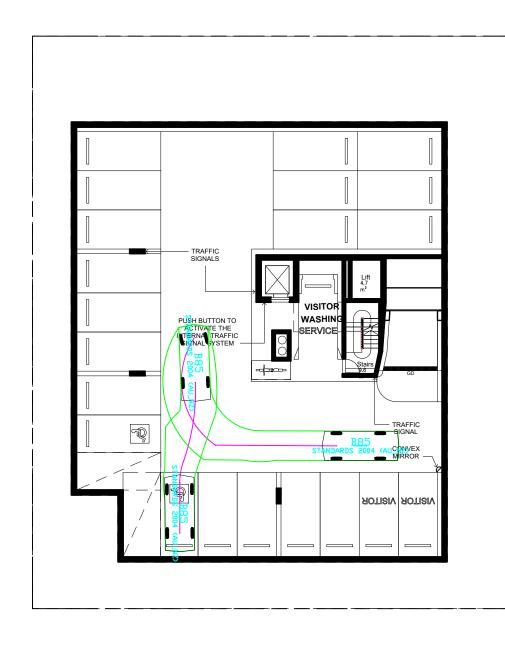
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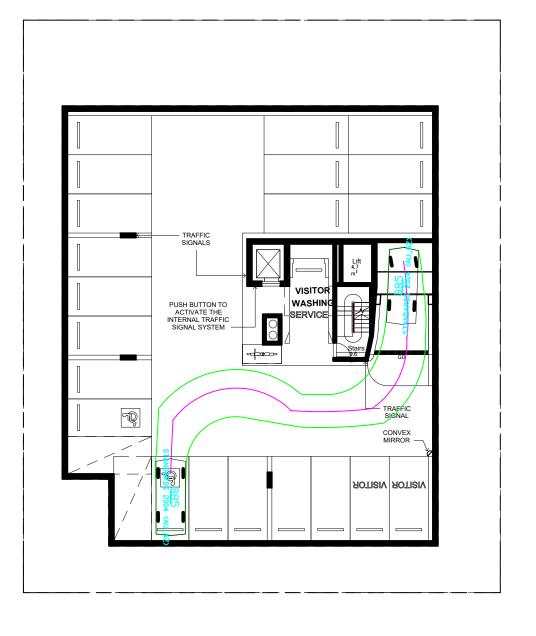
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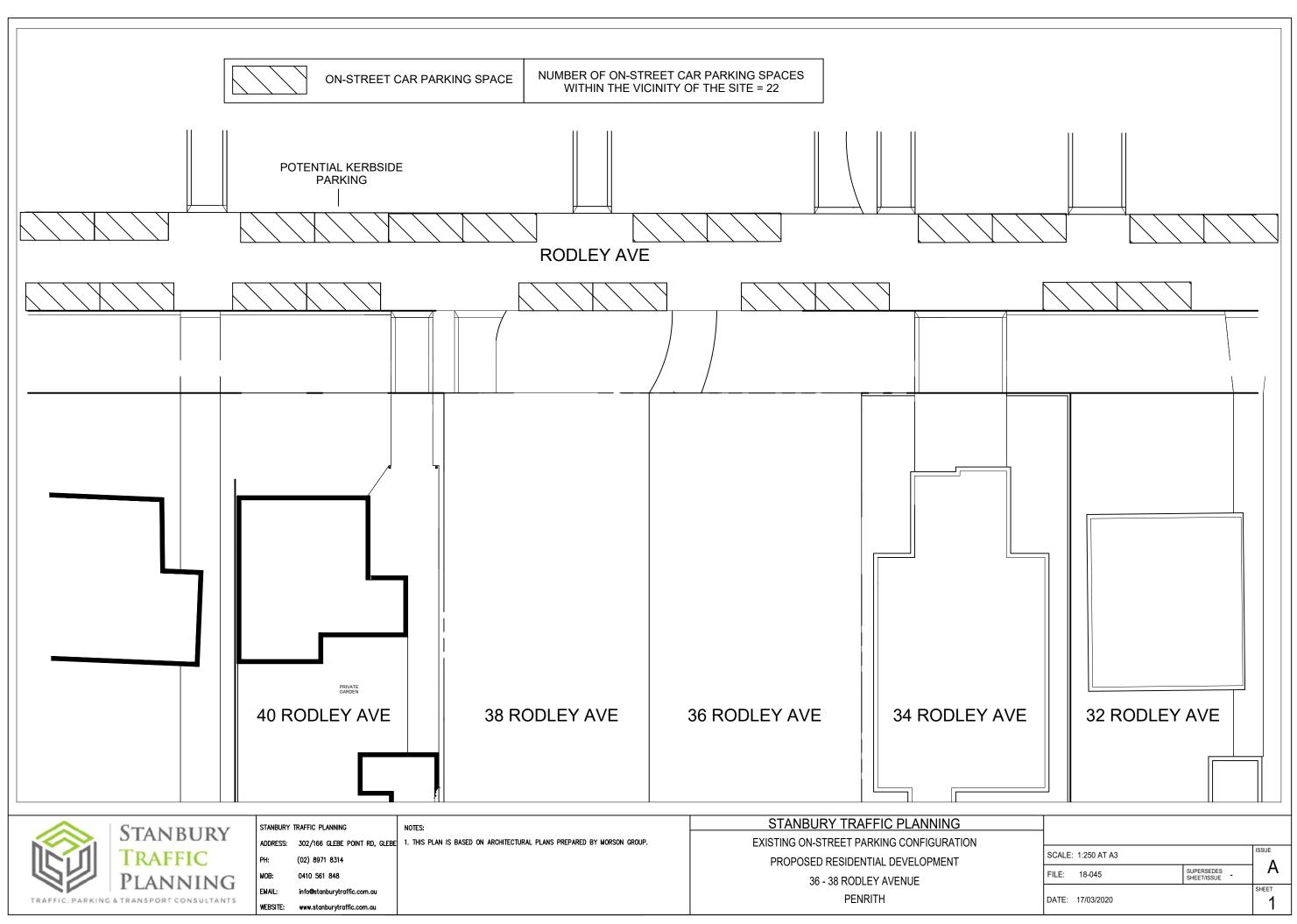
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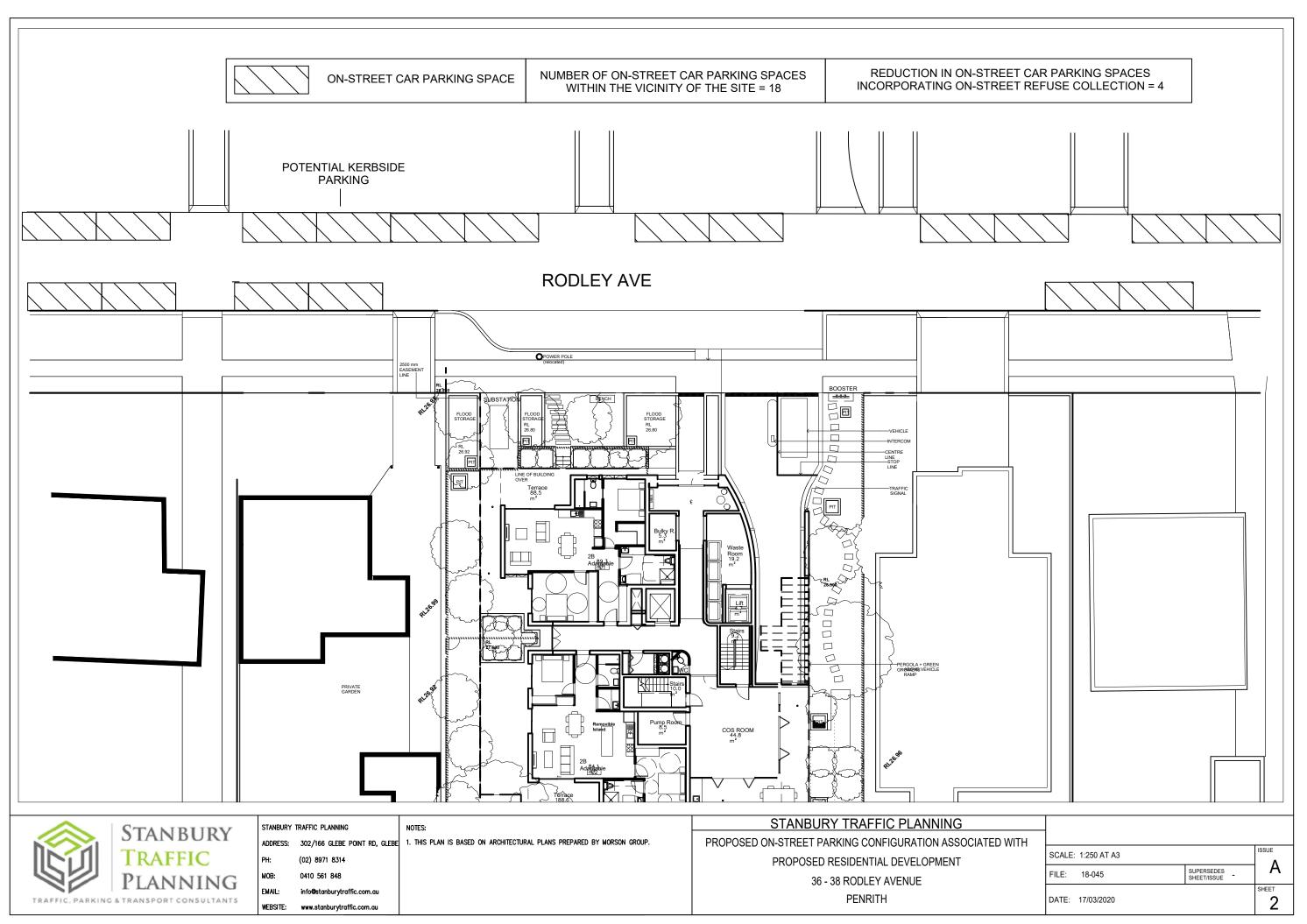
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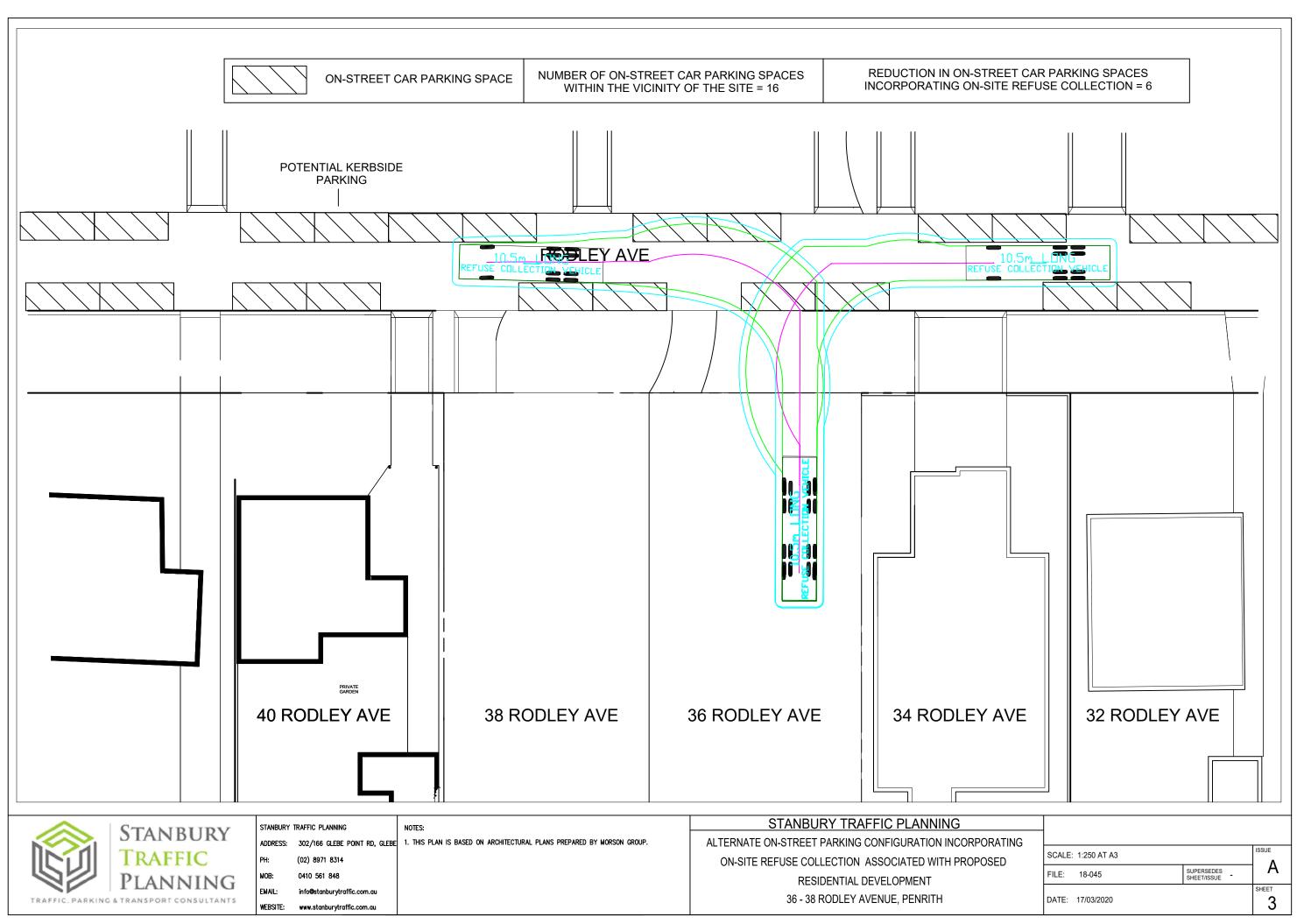
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# **APPENDIX 3**







# **APPENDIX 4**

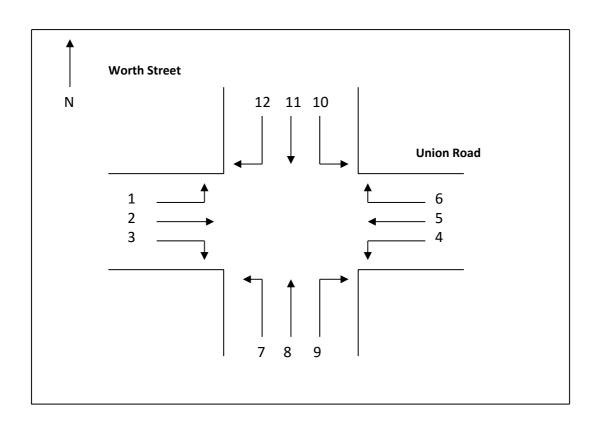


TRAFFIC COUNTS AT: Union Road & Worth Street, Penrith

DATE: 28<sup>th</sup> May 2018

TIME: Fine

Time					Directi	on of Ve	ehiculai	r Traffic				
	1	2	3	4	5	6	7	8	9	10	11	12
7.00 – 7.15pm	19	98	4	4	8	35	3	2	3	39	3	9
7.15 – 7.30pm	20	90	3	5	7	25	7	5	5	49	2	8
7.30 – 7.45pm	15	101	2	2	9	30	5	3	3	45	1	6
7.45 – 8.00pm	18	108	4	4	8	22	2	6	4	44	1	10
TOTAL	72	397	13	15	32	112	17	16	15	177	7	33
8.00 – 8.15pm	28	110	5	6	10	37	6	8	7	55	5	9
8.15 – 8.30pm	22	90	6	12	9	45	5	5	8	51	3	6
8.30 – 8.45pm	35	85	9	9	10	49	7	7	5	57	5	12
8.45 – 9.00pm	33	65	12	12	5	55	5	6	2	41	3	11
TOTAL	118	350	32	39	34	186	23	26	22	204	16	38
4.00 – 4.15pm	25	29	3	5	25	95	5	1	5	67	5	40
4.15 – 4.30pm	24	35	2	5	29	80	6	3	5	75	4	38
4.30 – 4.45pm	34	34	2	4	23	91	8	3	6	57	7	45
4.45 – 5.00pm	22	29	4	3	17	83	3	7	3	83	4	54
TOTAL	105	127	11	17	94	349	22	14	19	282	20	177
5.00 – 5.15pm	29	25	5	6	25	101	8	5	5	79	5	55
5.15 – 5.30pm	25	30	7	5	30	84	5	2	6	80	3	55
5.30 – 5.45pm	20	36	4	4	17	99	5	1	7	71	5	35
5.45 – 6.00pm	8	30	4	4	10	90	8	2	6	50	2	28
TOTAL	82	121	20	19	82	374	26	10	24	280	15	173



# **APPENDIX 5**

## **MOVEMENT SUMMARY**

Site: [Union Road & Worth Street]

Existing AM

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Optimum Cycle Time - Minimum Delay)

Mov Turn Demand Flows		Deg. Average Level		Level of	05% Back	of Queue	Prop.	Effective	Aver. No. Average			
ID	Tuiti	Total	HV	Satn	Delay	Service	Vehicles	Distance		Stop Rate		Speed
10		veh/h	%	v/c	sec	001 1100	veh	m	Queucu	Otop Hato	O y 0,00	km/h
South	: Worth	Street South										
1	L2	23	5.0	0.350	47.3	LOS D	3.0	22.1	0.96	0.75	0.96	33.8
2	T1	26	5.0	0.350	41.7	LOS C	3.0	22.1	0.96	0.75	0.96	34.4
3	R2	22	5.0	0.350	47.3	LOS D	3.0	22.1	0.96	0.75	0.96	33.7
Appro	ach	71	5.0	0.350	45.3	LOS D	3.0	22.1	0.96	0.75	0.96	34.0
East:	Union R	oad East										
4	L2	39	5.0	0.079	17.5	LOS B	1.6	11.7	0.54	0.57	0.54	47.0
5	T1	34	5.0	0.079	11.9	LOS A	1.6	11.7	0.54	0.57	0.54	48.2
6	R2	186	5.0	0.529	30.6	LOS C	6.7	48.6	0.84	0.81	0.84	39.1
Appro	ach	259	5.0	0.529	26.2	LOS B	6.7	48.6	0.75	0.74	0.75	41.2
North	: Worth S	Street North										
7	L2	204	5.0	0.539	40.5	LOS C	8.2	59.7	0.94	0.81	0.94	35.4
8	T1	16	5.0	0.141	31.5	LOS C	2.0	14.3	0.85	0.71	0.85	38.0
9	R2	38	5.0	0.141	37.1	LOS C	2.0	14.3	0.85	0.71	0.85	37.2
Appro	ach	258	5.0	0.539	39.4	LOS C	8.2	59.7	0.92	0.79	0.92	35.8
West:	Union R	load West										
10	L2	118	5.0	0.135	19.5	LOS B	2.9	21.2	0.59	0.71	0.59	44.5
11	T1	350	5.0	0.441	16.7	LOS B	11.0	80.3	0.71	0.63	0.71	46.8
12	R2	32	5.0	0.441	22.2	LOS B	11.0	80.3	0.71	0.63	0.71	45.5
Appro	ach	500	5.0	0.441	17.7	LOS B	11.0	80.3	0.68	0.65	0.68	46.2
All Ve	hicles	1088	5.0	0.539	26.7	LOS B	11.0	80.3	0.77	0.71	0.77	41.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Bacl Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P2	East Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P4	West Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
All Pedestrians		211	39.3	LOS D			0.94	0.94					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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## **MOVEMENT SUMMARY**

Site: [Union Road & Worth Street]

Existing PM

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Site Optimum Cycle Time - Minimum Delay)

Mov	Turn	Demand F	loveo	Dog	Average	Level of	OF9/ Book	of Queue	Dron	C#cotive	Aver No	Aver. No. Average	
ID	Turri	Total	HV	Deg. Satn	Average Delay	Service	Vehicles	Distance	Prop.	Stop Rate		Speed	
טו		veh/h	%	v/c	sec	OCI VICE	verildies	m	Queueu	Otop Mate	Cycles	km/h	
South	: Worth	Street South		.,,									
1	L2	22	5.0	0.272	46.9	LOS D	2.3	16.9	0.95	0.74	0.95	33.8	
2	T1	14	5.0	0.272	41.3	LOS C	2.3	16.9	0.95	0.74	0.95	34.4	
3	R2	19	5.0	0.272	46.8	LOS D	2.3	16.9	0.95	0.74	0.95	33.7	
Appro	ach	55	5.0	0.272	45.4	LOS D	2.3	16.9	0.95	0.74	0.95	33.9	
East:	Union R	oad East											
4	L2	17	5.0	0.126	19.5	LOS B	2.7	19.6	0.59	0.51	0.59	47.1	
5	T1	94	5.0	0.126	13.9	LOS A	2.7	19.6	0.59	0.51	0.59	48.3	
6	R2	349	5.0	0.704	29.4	LOS C	13.0	94.9	0.86	0.85	0.90	39.6	
Appro	ach	460	5.0	0.704	25.8	LOS B	13.0	94.9	0.80	0.77	0.82	41.4	
North	: Worth S	Street North											
7	L2	282	5.0	0.708	42.4	LOS C	12.0	87.7	0.98	0.86	1.04	34.7	
8	T1	20	5.0	0.492	33.7	LOS C	7.7	56.4	0.92	0.80	0.92	36.7	
9	R2	177	5.0	0.492	39.3	LOS C	7.7	56.4	0.92	0.80	0.92	36.0	
Appro	ach	479	5.0	0.708	40.9	LOS C	12.0	87.7	0.96	0.84	0.99	35.3	
West:	Union R	load West											
10	L2	105	5.0	0.077	8.6	LOS A	1.2	8.5	0.27	0.64	0.27	51.2	
11	T1	127	5.0	0.164	14.8	LOS B	3.5	25.3	0.61	0.52	0.61	48.0	
12	R2	11	5.0	0.164	20.4	LOS B	3.5	25.3	0.61	0.52	0.61	46.6	
Appro	ach	243	5.0	0.164	12.4	LOS A	3.5	25.3	0.47	0.57	0.47	49.3	
All Ve	hicles	1237	5.0	0.708	29.9	LOS C	13.0	94.9	0.80	0.76	0.82	39.6	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

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P2	East Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P3	North Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
P4	West Full Crossing	53	39.3	LOS D	0.1	0.1	0.94	0.94					
All Pedestrians		211	39.3	LOS D			0.94	0.94					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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