



Date 27/02/2019

To Aubrey Chan, Jamie Gordon, Kylie Soltani and Zachary Karantonis

From Dov Ben-Avraham

Copy toGeorgia Ashdown, Mark Kuhne, Clement Ding and Melanie GostelowSubjectWestfield Penrith Mondo – Development Application Memorandum

Introduction

Arcadis has been engaged by Scentre Group Limited to prepare a stormwater management strategy to support the Development Application (DA) for the proposed Westfield Penrith Mondo infill development. The development will be confined to the Westfield site and consist of landscaping upgrades, expansion of external retail spaces and the development of a new food premise building that will serve as a landmark fronting High Street.

This memorandum provides a summary of the stormwater management strategy for the proposed development and addresses the following topics as they relate to the development:

- Stormwater requirements (including stormwater drainage upgrades, WSUD and OSD requirements);
- · Flooding constraints; and
- Flood planning controls and requirements.

This memorandum should be read in conjunction with the Civil DA drawings and MUSIC-link report included in Appendix A and B respectively, as well as the Penrith City Council Flood Letter (29 May 2018) supplied in Appendix C.

The overall Stormwater Management Strategy has been developed in accordance with the following:

- Penrith City Council Development Control Plan (2014);
- WSUD Technical Guidelines Version 3 (2015); and
- Stormwater Drainage Guidelines for Building Developments (2016).

Site Description

The proposed development is to be situated on an existing brownfield site located at 569 High Street, Penrith NSW 2750 (Lot 1, DP1137699) and is located within the Penrith City Council (PCC) Local Government Area (LGA). The proposed development site is approximately 0.377 hectares in area and is bounded by Westfield Penrith (North and East), High Street (South) and the Joan Sutherland Performing Arts Centre (West). The subject site and aerial is shown in Figure 1 on the next page.



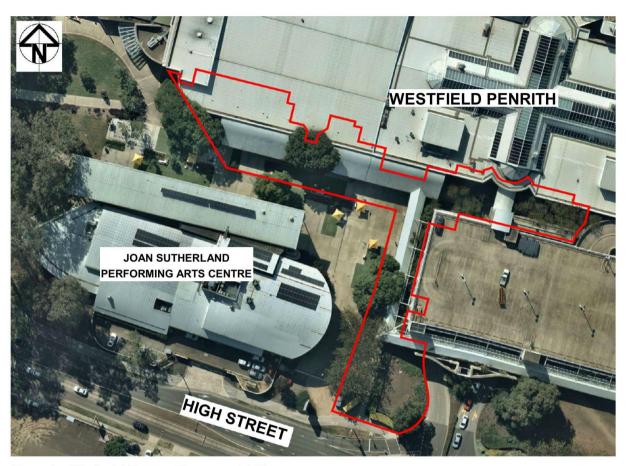


Figure 1 - Site Aerial (Source: Nearmaps, 2018)

The existing site generally grades to a sag at the centre of the Mondo community space where a series of grated trench drains and grated stormwater pits incorporated amongst the landscaping features collects stormwater runoff and conveys it into the existing twin 1350mm dia. trunk drainage pipeline (refer to Appendix A – Civil DA drawings). Furthermore, the areas fronting High Street bypass the existing stormwater drainage infrastructure within the site and are drained via road drainage along High Street.

Stormwater Requirements

Proposed Development

The surface treatments for the proposed private domain upgrades have been categorised as follows:

- Impervious (concrete paving, gravel, synthetic turf and roof); and
- Pervious (vegetated landscaping)

Referring to the Stormwater Catchment Plan (Appendix A – Drawing No. C221), the net total site imperviousness in the post-development scenario has increased by 479 m², which has triggered the requirement for Water Sensitive Urban Design (WSUD) and On-Site Detention (OSD) as per the guidelines and requirements of PCC.

On-Site Detention

To address stormwater detention objectives, an underground OSD tank has been designed (refer to Appendix A – Drawing No. C231) to attenuate stormwater peak flows for all stormwater events (up to

and including the 100-year ARI event) to pre-development levels in accordance with the PCC – Development Control Plan (2014), Section 3.6 Stormwater Management and Drainage. Furthermore, the OSD has been designed to be in compliance with the OSD storage and Permissible Site Discharge (PSD) requirements as detailed in PCC's – Stormwater Drainage Guidelines for Building Developments (2016), Section 4 On-Site Detention.

The OSD storage tank is proposed to be located on the South-West corner of proposed building R9 (refer to Appendix A – Drawing No. C201) and would collect stormwater runoff from the adjacent hardstand areas as well as the entire roof and associated awnings of building R9 (Appendix A – Drawing No. C231).

Catchment modelling has been undertaken using DRAINS modelling software to analyse and confirm that the OSD storage tank designed in accordance with PCC guidelines would indeed achieve PCC peak flow targets. Key parameters used in the DRAINS analysis are summarised in Table 1 below.

Table 1 – DRAINS modelling parameters

DRAINS Parameter	Value
Rainfall IFD	AR&R1987 IFD Data in accordance with Penrith City Council's – Stormwater Drainage Guidelines for Building Developments (2016), Appendix E
Paved Area Depression Storage	1 mm
Supplementary Area Depression Storage	1 mm
Pervious Area Depression Storage	5 mm
Antecedent Moisture Condition	3
Soil Type	3

A summary comparison of peak flow results for the pre-development and post-development site is detailed in Table 2 below.

Table 2 - Stormwater Peak Flow Results

Storm Event	Pre-Development Peak Flow (m³/s)	Post-Development Peak Flow Incl. OSD Tank (m³/s)
10-year ARI	0.139	0.123
100-year ARI	0.200	0.180

The DRAINS model results indicate that the proposed OSD storage would ensure that post-development discharge would achieve PCC stormwater peak flow requirements and ensure that site discharge would not exceed pre-development levels.

Water Sensitive Urban Design

To address stormwater quality objectives, water sensitive urban design measures have been implemented into the proposed site redevelopment in order to meet PCC load reduction targets in accordance with the PCC WSUD Technical Guidelines Version 3 (2015).

A stormwater quality model was developed for the site using MUSIC modelling software (V6.3) with modelling parameters being adopted from the PCC MUSIC-Link and WSUD Technical Guidelines Version 3 (2015).

A summary of the treatment measures proposed for the site are detailed in Appendix A – Drawing No. C201. Key features of the proposed treatment strategy include a proposed rainwater tank reuse scheme for toilets within food premises R2, R3, R5, R6 and R9 as well as for landscape irrigation across the site. Furthermore, it is proposed to integrate $35m^2$ of bioretention within the proposed raised planter box in the North-East portion of the site (refer to Appendix A – Drawing No. C201) to treat roof runoff. An example of a planter box raingarden is shown below in Figure 2.

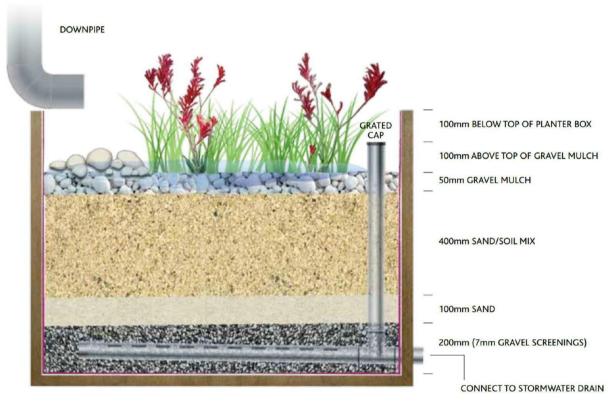


Figure 2 – Typical Cross-section of a planter box raingarden (Source: Melbourne Water - Building a planter box raingarden)

It should be noted that all existing areas within the private domain not subject to redevelopment works and any existing hardstand bypass areas were excluded from the MUSIC model as these areas will remain consistent with pre-development conditions. Table 3 on the next page summarises the achieved pollutant reductions.

Table 3 - Stormwater Quality Pollutant Reduction Results

Key Pollutant	Penrith City Council Load Reduction Targets Load Reduction Achieve	
Total Suspended Solids (TSS)	85%	85.8%
Total Phosphorus (TP)	60%	61.6%
Total Nitrogen (TN)	45%	57.4%
Gross Pollutants	90%	100%

The MUSIC model results indicate that the proposed water quality strategy would achieve PCC pollutant load reduction targets. The PCC MUSIC-link report has been included in Appendix B for reference.

Stormwater Drainage Infrastructure

All existing stormwater drainage infrastructure within the public domain will be retained and no modifications are proposed as part the redevelopment works.

No modifications to the trunk drainage infrastructure (twin 1350mm dia. stormwater pipes) contained within the 7.5m wide easement are proposed as part of the redevelopment of the site.

Flooding Constraints

Existing Flood Conditions

PCC flood mapping indicates that there is localised overland flooding in the north-eastern corner of the development area as seen in Figure 3 below (refer to Appendix C - Flood Letter from Penrith City Council).



Figure 3 – 1% AEP Local Overland Flow Flood Map (Penrith City Council Flood Letter, 29 May 2018)

The designated flood level is RL 27.2 and is either associated with surcharge from the local stormwater network within the site, or insufficient inlet capacity to drain runoff within the sag low point in the 1% AEP flood event (100-year ARI event).

Proposed Development

The proposed landscaping upgrades will not affect drainage capacity nor is it proposed to modify surface levels or gradients in the flood affected area.

Due to the inclusion of OSD infrastructure and Raingardens (treating roof runoff) with extended detention storage attenuating peak flows to less than existing levels, we expect there to be a minor improvement in site hydrology and flooding conditions in the proposed development scenario. Furthermore, despite the extension of the existing awning connected to the Westfield Penrith Shopping Centre, it will be extended over existing impervious areas and hence there will be no net change in site imperviousness in the flood affected private domain.

Flood Planning Controls and Requirements

Flood Planning Levels

As stated in PCC's *DCP* (2014), Section 3.5 Flood Planning, where possible, internal floor levels, access to internal stairs and lifts to basement levels shall be at least 0.5m above the 1% AEP flood event (100-year ARI event) level of RL 27.2m (refer to Appendix C – Flood Letter from Penrith City Council) Therefore, the flood planning level relevant to this site is RL 27.7m.

Scentre Group Limited have nominated extensions to the existing Westfield Penrith Shopping Centre, identified as proposed retail premises R1 and proposed food premises R2, R3, R5 and R6. These extensions will have a finished floor level (FFL) of RL 27.30 to match the existing FFL of the remaining Westfield Penrith Shopping Centre to which they are attached. Furthermore, proposed infill development building R9 will have a slightly higher FFL of RL 27.35 to account for raised levels in its proposed location, however will still be connected to the rest of the proposed extensions via a shared and continuous roof structure.

Referring to PCC's DCP (2014), Section 3.5C (7a) Industrial/Commercial – Extensions and Infill Development, PCC may approve of the development with floor levels below the 1% AEP flood event (100-year ARI event) if:

The raising of the floor levels would be out of character with adjacent buildings.

As the proposed retail and food extensions and infill development building R9 will be connected to the existing Westfield Penrith Shopping Centre, the FFL's should remain generally consistent between the existing and proposed developments rather than apply a proposed flood planning level of RL 27.7m, which is significantly higher than the existing Westfield Penrith Shopping Centre FFL of RL 27.30 and typically applied to new developments including independent structures. Furthermore, it should be noted that the proposed FFL's will still be above the 1% AEP flood event level of RL 27.2m.

Conclusion

Arcadis has been engaged by Scentre Group Limited to prepare a stormwater management strategy to support a DA for the proposed Mondo redevelopment at Westfield Penrith in accordance with Penrith City Council guidelines and requirements. This memorandum details existing flood conditions as well as stormwater requirements that will be applicable to the proposed development.

As the proposed redevelopment does result in a net increase in imperviousness from existing conditions, OSD and WSUD measures have been implemented to address Penrith City Council requirements and manage site runoff in terms of peak flow attenuation and water quality.

It is expected that the proposed infill development will not have an impact or worsen existing flood conditions and that existing surfaces, levels, grades and stormwater drainage infrastructure will be retained within the public domain. OSD and WSUD measures will offset increases to imperviousness and runoff within the private domain.

We trust the contents of this memorandum satisfies the requirements of our scope and objective. If you have any questions, please feel free to contact me on my number below.

Yours sincerely

Dov Ben-Avraham

Senior Engineer (02) 8907 8286

Appendix A. Civil DA Drawings

Appendix B. Penrith City Council MUSIC-link Report

Appendix C. Penrith City Council Flood Letter for Westfield

Penrith (dated 29 May 2018)





Appendix A - Civil DA Drawings





WESTFIELD PENRITH MONDO DEVELOPMENT APPLICATION

CIVIL DRAWING LIST

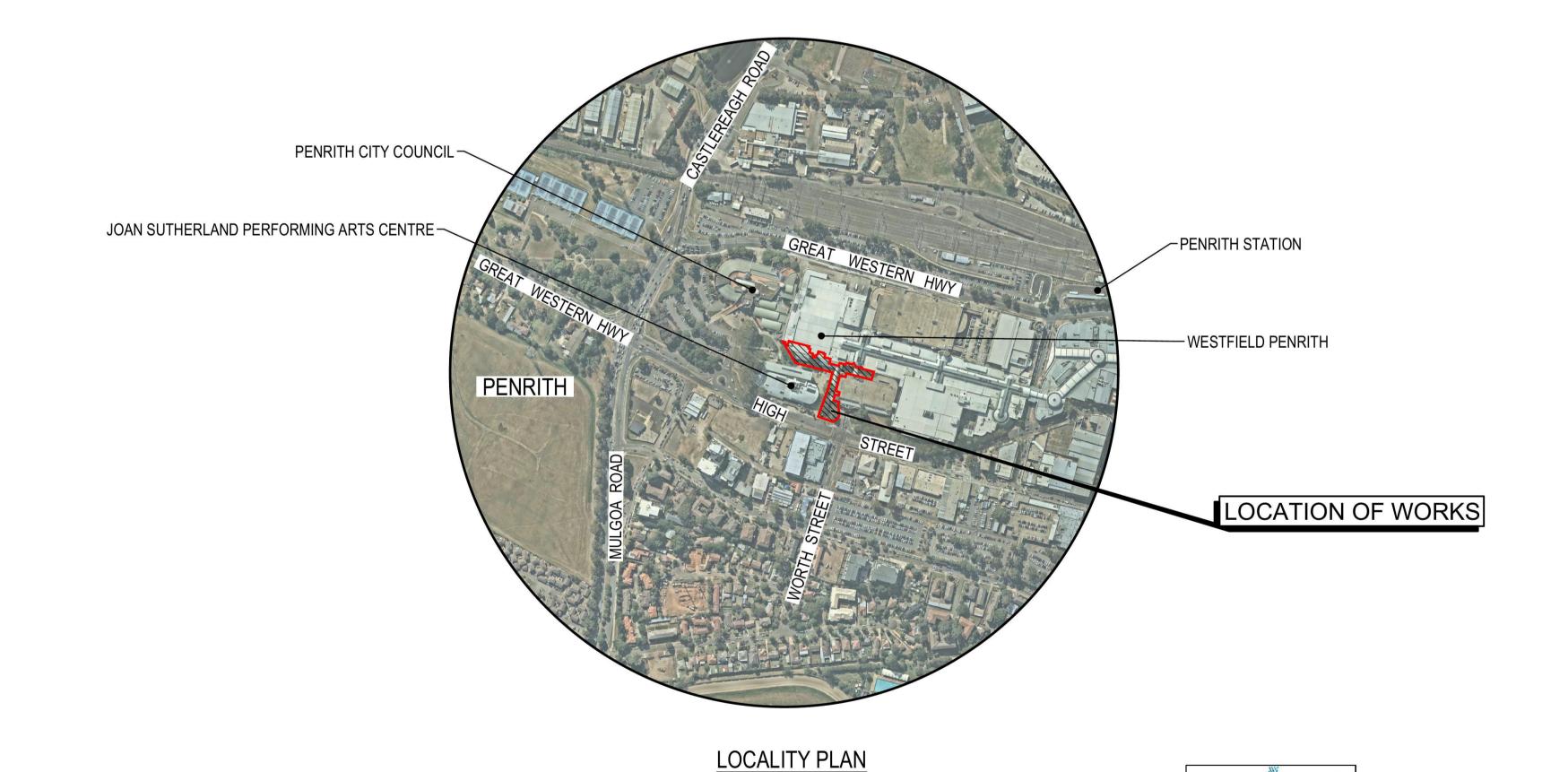
GENERAL

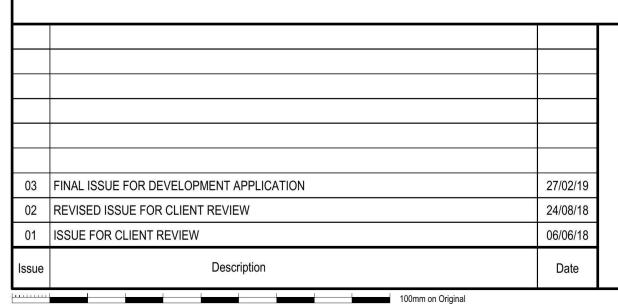
COVER SHEET AND DRAWING LIST

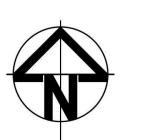
EROSION AND SEDIMENT CONTROL PLAN EROSION AND SEDIMENT CONTROL DETAILS

STORMWATER DRAINAGE

STORMWATER MANAGEMENT PLAN STORMWATER CATCHMENT PLAN OSD TANK PLAN, CATCHMENT PLAN AND DETAILS







SCENTRE GROUP Scales

NOT TO BE USED FOR CONSTRUCTION Current Issue Signatures 1:5000 Checked D.BEN-AVRAHAM D.BEN-AVRAHAM

C001-10019736-04-nsd-CoverSheetAndDrawingList.dwg

DEVELOPMENT APPLICATION

WESTFIELD PENRITH MONDO **REDEVELOPMENT**

> **COVER SHEET AND** DRAWING LIST

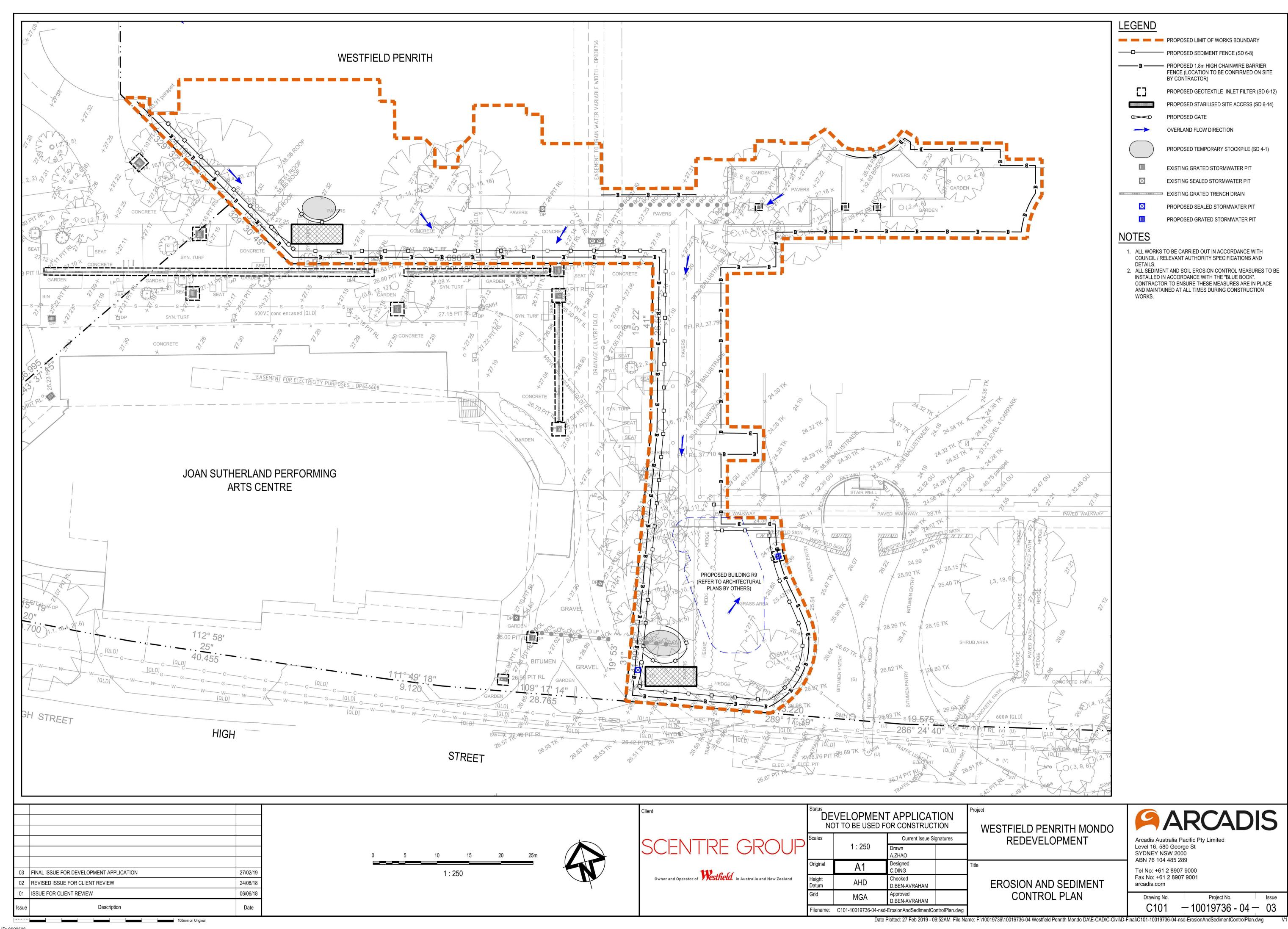
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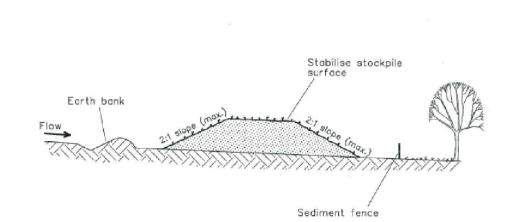
ARCADIS Arcadis Australia Pacific Pty Limited Level 16, 580 George St SYDNEY NSW 2000 ABN 76 104 485 289

Tel No: +61 2 8907 9000 Fax No: +61 2 8907 9001 arcadis.com

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-10019736 - 04 - 03Date Plotted: 27 Feb 2019 - 09:45AM File Name: F:\10019736\10019736-04 Westfield Penrith Mondo DA\E-CAD\C-Civil\D-Final\C001-10019736-04-nsd-CoverSheetAndDrawingList.dwg

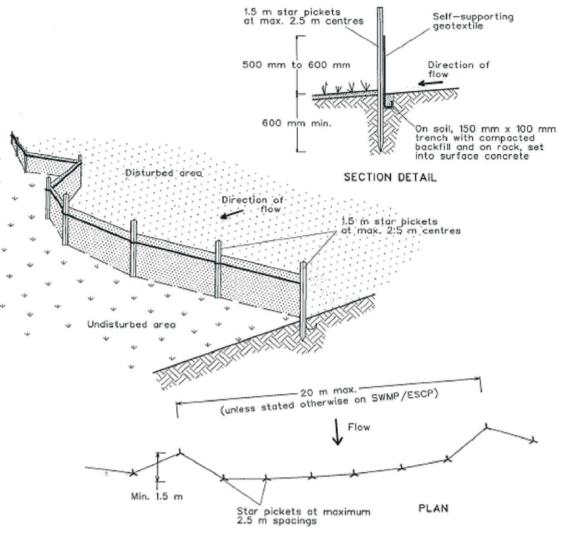




Construction Notes

- Place stockpiles more than 2 (preferably 5) metres from existing vegetation, concentrated water flow, roads and hazard areas.
- 2. Construct on the contour as low, flat, elongated mounds.
- Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.
- Where they are to be in place for more than 10 days, stabilise following the approved ESCP or SWMP to reduce the C-factor to less than 0.10.
- Construct earth banks (Standard Drawing 5-5) on the upslope side to divert water around stockpiles and sediment fences (Standard Drawing 6-8) 1 to 2 metres downslope.

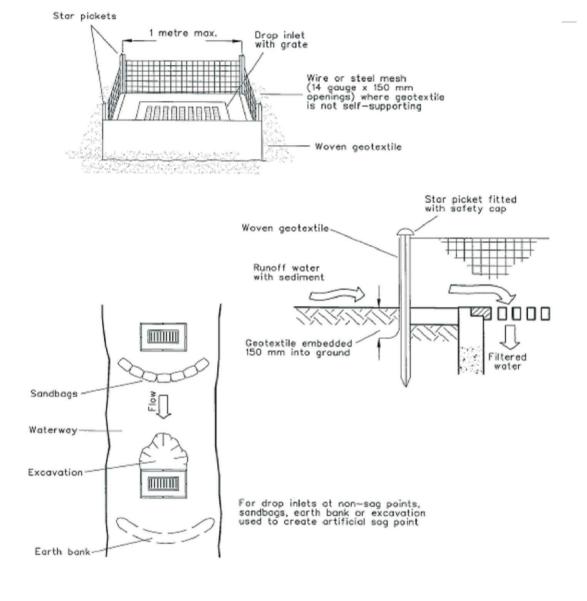
TEMPORARY STOCKPILES (SD 4-1)



Construction Notes

- Construct sediment fences as close as possible to being parallel to the contours of the site, but with small returns as shown in the drawing to limit the catchment area of any one section. The catchment area should be small enough to limit water flow if concentrated at one point to 50 litres per second in the design storm event, usually the 10-year event.
- Cut a 150-mm deep trench along the upslope line of the fence for the bottom of the fabric to be entrenched.
- Drive 1.5 metre long star pickets into ground at 2.5 metre intervals (max) at the downslope edge of the trench. Ensure any star pickets are fitted with safety caps.
- 4. Fix self-supporting geotextile to the upslope side of the posts ensuring it goes to the base of the trench. Fix the geotextile with wire ties or as recommended by the manufacturer. Only use geotextile specifically produced for sediment fencing. The use of shade cloth for this purpose is not satisfactory.
- 5. Join sections of fabric at a support post with a 150-mm overlap.
- Backfill the trench over the base of the fabric and compact it thoroughly over the geotextile.

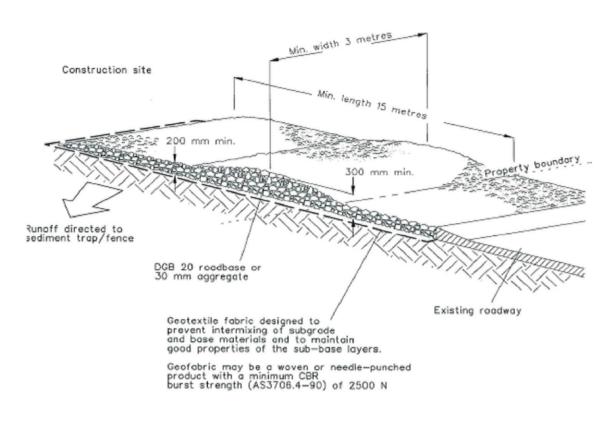
SEDIMENT FENCE (SD 6-8)



Construction Notes

- 1. Fabricate a sediment barrier made from geotextile or straw bales.
- Follow Standard Drawing 6-7 and Standard Drawing 6-8 for installation procedures for the straw bales or geofabric. Reduce the picket spacing to 1 metre centres.
- 3. In waterways, artificial sag points can be created with sandbags or earth banks as shown in the drawing.
- 4. Do not cover the inlet with geotextile unless the design is adequate to allow for all waters to bypass it.

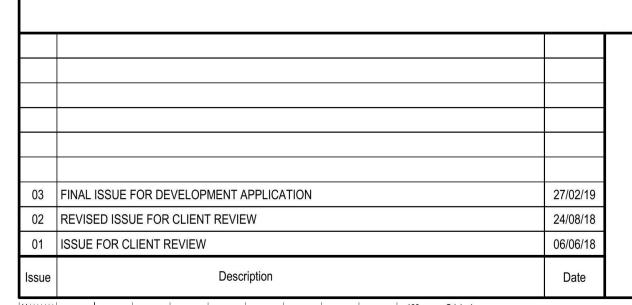
GEOTEXTILE INLET FILTER (SD 6-12)



Construction Notes

- Strip the topsoil, level the site and compact the subgrade.
- 2. Cover the area with needle-punched geotextile.
- 3. Construct a 200-mm thick pad over the geotextile using road base or 30-mm aggregate.
- 4. Ensure the structure is at least 15 metres long or to building alignment and at least 3 metres
- Where a sediment fence joins onto the stabilised access, construct a hump in the stabilised access to divert water to the sediment fence

STABILISED SITE ACCESS (SD 6-14)



SCENTRE GROUP Scales
Original

N.T.S.

Drawn
A.ZHAO

Original

A1

Designed
C.DING

Checked
Datum

AHD

Checked
D.BEN-AVRAHAM

Grid

MGA

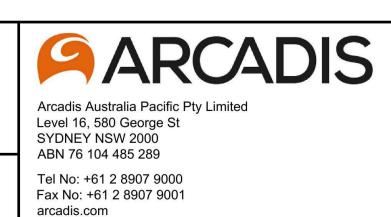
Approved
D.BEN-AVRAHAM

Filename: C111-10019736-04-nsd-ErosionAndSedimentControlDetails.dwg

DEVELOPMENT APPLICATION
NOT TO BE USED FOR CONSTRUCTION

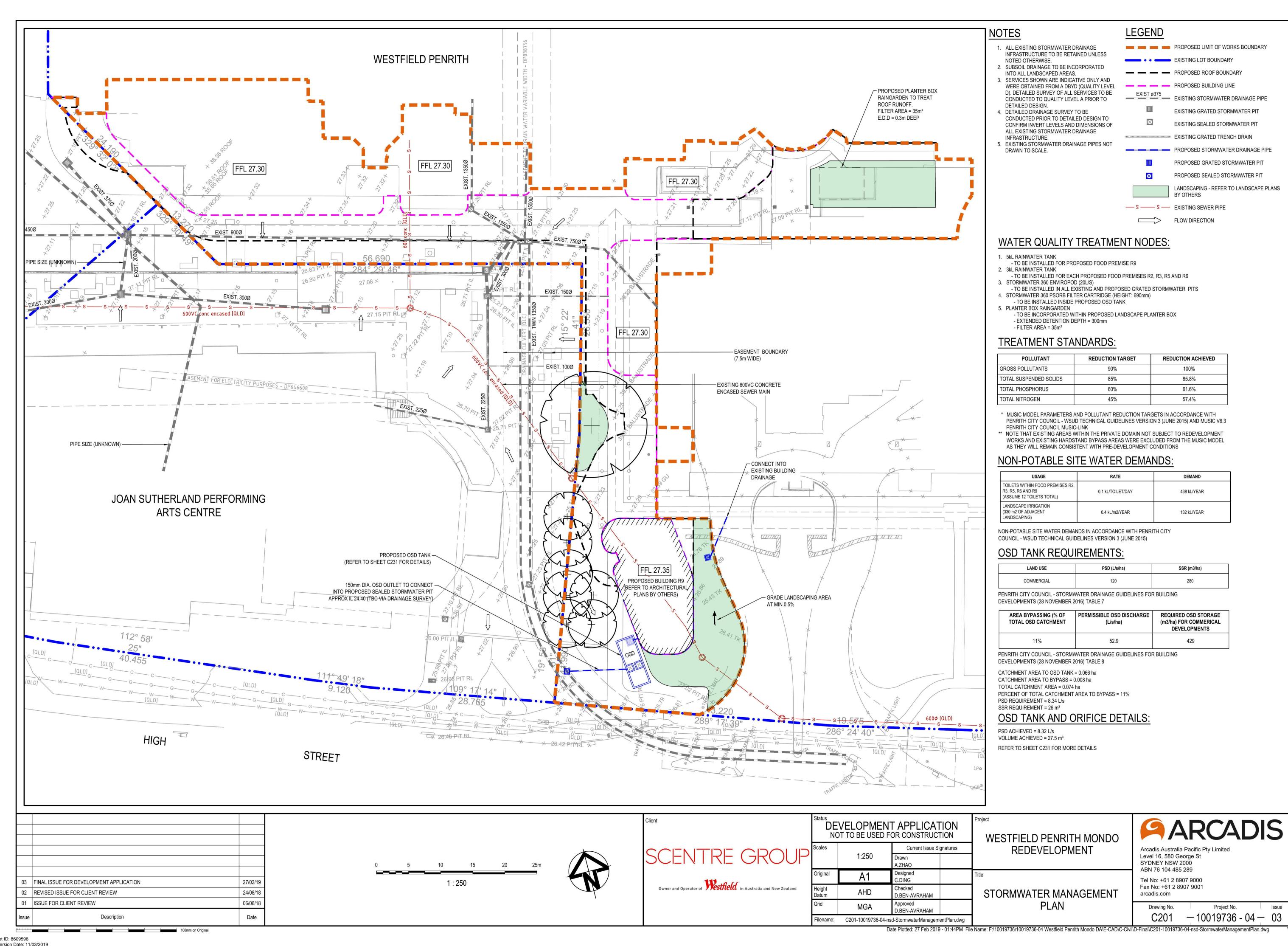
WESTFIELD PENRITH MONDO REDEVELOPMENT

EROSION AND SEDIMENT CONTROL DETAILS



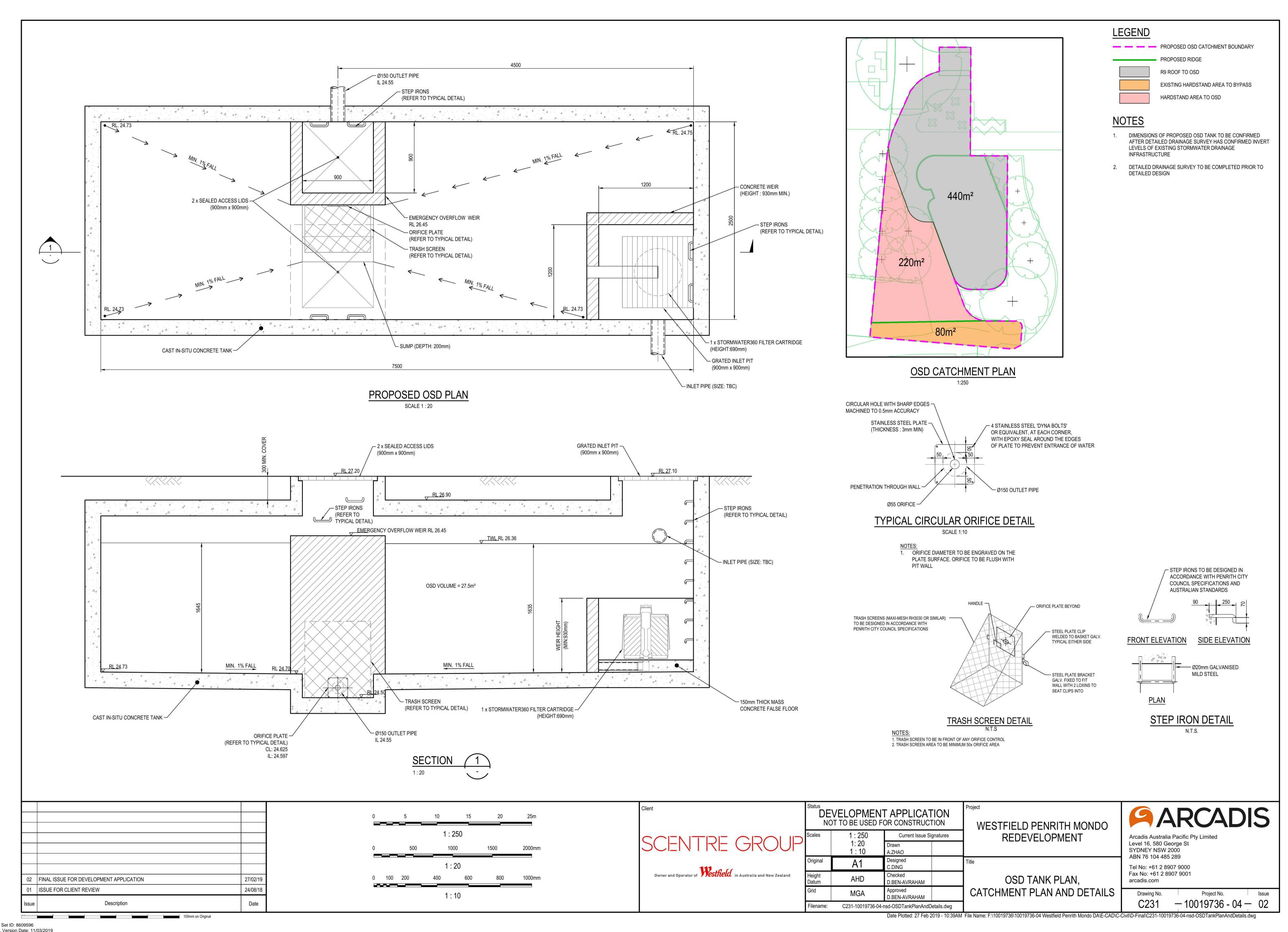
Drawing No. Project No. Issue C111 — 10019736 - 04 — 03

Document Set ID: 8609596 Version: 1, Version Date: 11/03/2019 Current Issue Signatures





Version: 1, Version Date: 11/03/2019







Appendix B - Penrith City Council MUSIC-link Report







MUSIC-*link* Report

Project Details Company Details			Details
Project:	Westfield Penrith Mondo Redevelopment	Company:	Arcadis Australia Pacific Pty Ltd
Report Export Date:	27/02/2019	Contact:	Dov Ben-Avraham
Catchment Name:	Westfield Penrith Mondo - Water Quality Model - Final	Address:	Level 16, 580 George Street, Sydney NSW 2000, Australia
Catchment Area:	0.305ha	Phone:	02 8907 8286
Impervious Area*:	89.18%	Email:	Dov.BenAvraham@arcadis.com
Rainfall Station:	67113 PENRITH		
Modelling Time- step:	6 Mnutes		
Modelling Period:	1/01/1999 - 31/12/2008 11:54:00 PM		
Mean Annual Rainfall:	691mm		
Evapotranspiration:	1158mm		
MUSIC Version:	6.3.0		
MUSIC-link data Version:	6.31		
Study Area:	Penrith		

^{*} takes into account area from all source nodes that link to the chosen reporting node, excluding Import Data Nodes

Penrith Development

Treatment Train Effectiveness		Treatment Nodes		Source Nodes	
Node: Post-Development Node	Reduction	Node Type	Number	Node Type	Number
How	22.6%	Rain Water Tank Node	2	Urban Source Node	6
TSS	85.8%	Bio Retention Node	1		
TP	61.6%	Generic Node	1		
TN	57.4%	GPT Node	3		
GP CP	100%				

Comments

Scenario:

A non-conformance with the proposed rainwater tank reuse demand has been identified. However, for the purposes of this model, we have simply attempted to maximise reuse potential with available end uses i.e. toilet flushing within proposed food premises and landscape irrigation. Furthermore, we have opted for rainwater tank sizes typical of similar developments and hence have opted for larger rainwater tanks that have lower reuse demand met.





Passing Parameters							
Node Type	Node Name	Parameter	Min	Max	Actual		
Bio	Planter Box Raingarden	Hi-flow bypass rate (cum/sec)	None	99	99		
Bio	Planter Box Raingarden	PET Scaling Factor	2.1	2.1	2.1		
GPT	1 x EnviroPod 200	Hi-flow bypass rate (cum/sec)	None	99	0.02		
GPT	1 x EnviroPod 200	Hi-flow bypass rate (cum/sec)	None	99	0.02		
GPT	1 x EnviroPod 200	Hi-flow bypass rate (cum/sec)	None	99	0.02		
Post	Post-Development Node	% Load Reduction	None	None	22.6		
Post	Post-Development Node	GP % Load Reduction	90	None	100		
Post	Post-Development Node	TN % Load Reduction	45	None	57.4		
Post	Post-Development Node	TP % Load Reduction	60	None	61.6		
Post	Post-Development Node	TSS % Load Reduction	85	None	85.8		
Urban	50% Roof	Area Impervious (ha)	None	None	0.103		
Urban	50% Roof	Area Impervious (ha)	None	None	0.103		
Urban	50% Roof	Area Pervious (ha)	None	None	0		
Urban	50% Roof	Area Pervious (ha)	None	None	0		
Urban	50% Roof	Total Area (ha)	None	None	0.103		
Urban	50% Roof	Total Area (ha)	None	None	0.103		
Urban	Landscaping	Area Impervious (ha)	None	None	0		
Urban	Landscaping	Area Pervious (ha)	None	None	0.014		
Urban	Landscaping	Total Area (ha)	None	None	0.014		
Urban	R9 Hardstand	Area Impervious (ha)	None	None	0.022		
Urban	R9 Hardstand	Area Pervious (ha)	None	None	0		
Urban	R9 Hardstand	Total Area (ha)	None	None	0.022		
Urban	R9 Landscaping	Area Impervious (ha)	None	None	0		
Urban	R9 Landscaping	Area Pervious (ha)	None	None	0.019		
Urban	R9 Landscaping	Total Area (ha)	None	None	0.019		
Urban	R9 Roof	Area Impervious (ha)	None	None	0.044		
Urban	R9 Roof	Area Pervious (ha)	None	None	0		
Urban	R9 Roof	Total Area (ha)	None	None	0.044		





Failing Parameters						
Node Type	Node Name	Parameter	Min	Max	Actual	
Rain	Rainwater Tank	% Reuse Demand Met	80	None	44.2041	
Rain	Rainwater Tank	% Reuse Demand Met	80	None	59.3155	
Only certain parameters are reported when they pass validation						





Appendix C – Penrith City Council Flood Letter for Westfield Penrith (dated 29 May 2018)





Our Reference: ECM 8199005 Contact: Ratnam Thilliyar

Telephone: 4732 7988

29 May 2018

Clement Ding Level 16 580 George Street SYDNEY NSW 2000

Dear Sir/Madam

Flood Level Enquiry Lot 1033 DP 849297 - No. 597-599 High Street, Penrith

Please find enclosed Flood Level information for the above property.

Should you require any further information please do not hesitate to contact me on 4732 7988.

Yours sincerely

Ratnam Thilliyar

Engineering Stormwater Supervisor

Penrith City Council PO Box 60, Penrith NSW 2751 Australia T 4732 7777 F 4732 7958 penrithcity.nsw.gov.au



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Flood Information Lot 1033 DP 849297 No. 597-599 High Street, Penrith

Date of issue: 29 May 2018

The 1% AEP local overland flow flood level affecting the above property is estimated to be RL27.2m AHD.

Property less than 0.5m above the 1% AEP flood level is subject to Penrith Development Control Plan 2014 Section C3.5 Flood Planning. The Penrith Development Control Plan 2014 is available from Council's website www.penrithcity.nsw.gov.au.



Definitions

AEP - Annual Exceedance Probability - the chance of a flood of this size occurring in any one year.

AHD – Australian Height Datum – A standard level datum used throughout Australia, approximately equivalent to mean sea level.

Legend

Extent of 1% AEP local catchment overland flow path. Generally depths less than 150mm is not shown.

Notes:

- The contours shown above in yellow numbering are at 0.5m intervals and are based on Aerial Laser Scanning (ALS) Survey undertaken in 2002. The contour levels are approximate and for general information only. Accurate ground levels should be obtained by a Registered Surveyor.
- The flood level is based on current information available to Council at the date of issue. The flood level may change in the
 future if new information becomes available. The 1% AEP flood is the flood adopted by Council for planning controls. Rarer
 and more extreme flood events will have a greater effect on the property.
- Council's studies are reflected in flood mapping for the City which show properties potentially affected by overland flows in excess of 150mm.
- 4. This property is shown on Council's flood mapping as potentially so affected.
- Council imposes flood related development controls where, in its opinion, such controls are justified. Such controls may or may not be imposed with respect to this property in the event of an application for development consent.
- 6. If a development proposal is submitted with respect to this property, Council will consider the possibility of flood or overland flow in the context of the application. Council may impose a requirement that the applicant for development consent carry out a detailed assessment of the possible overland water flows affecting the property (a flood study) and/or may impose other controls on any development designed to ameliorate flood risk.
- 7. You are strongly advised if you propose to carry out development upon the property, that you retain the assistance of an experienced flooding engineer and have carried out a detailed investigation.
- 8. Council accepts no liability for the accuracy of the flood levels (or any other data) contained in this certificate, having regard to the information disclosed in Notes "1" to "4". As such you should carry out and rely upon your own investigations.

Penrith City Council PO Box 60, Penrith NSW 2751 Australia T 4732 7777 F 4732 7958 penrithcity.nsw.gov.au

Ratnam Thillivar

Engineering Stormwater Supervisor



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