

731-769 Great Western Hwy, Werrington Stage 4 Residential

Traffic Impact Assessment

Prepared for: Statewide Planning Pty Ltd

2 February 2021

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731-769 Great Western Hwy, Werrington Stage 4 Residential Traffic Impact Assessment

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Table of Contents

1	Intro	oduction1
2	Bac	kground3
	2.1	Site Location
	2.2	Strategic Planning4
	2.3	DCP Objectives and Controls for Werrington Mixed Use Area
3	Exist	ting Transport Context
	3.1	Road Network
	3.2	Public Transport7
	3.3	Traffic Volumes (October 2016)9
	3.4	Existing Intersection Operation10
4	Stag	ge 4 Residential Development12
	4.1	Land Use and Layout12
		4.1.1 Road Width and Driveways
	4.2	Public Transport
	4.3	Pedestrians and Cyclists15
	4.4	Parking Requirements15
5	Traf	fic Assessment
	5.1	WELL Precinct Masterplan
	5.2	Proposed Traffic Generation17
		5.2.1 Traffic Distribution
	5.3	Intersection Modelling19
6	Cor	nclusion

Tables

Table 3.1:	Public Transport	. 8
Table 3.2:	AM and PM Peak Traffic Volumes at Intersection	. 9
Table 3.3:	Level of Service Criteria for Intersection Operation	10
Table 3.4:	Existing (2016) Intersection Operation	11
Table 5.1:	TMAP Intersection Modelling Results	16
Table 5.2:	Proposed Development Traffic	17
Table 5.3:	Cumulative Traffic Generation of Werrington Mixed Use Area	17
Table 5.4:	Post Development Intersection Operation	20

17254_r01v03_210202 Stg4 Residential TIA



Table 5.5:	Post Development	Intersection	Operation	WELL	PRECINCT TMAP	
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Figures

Figure 2.1:	Locality Map	3
Figure 2.2:	WELL Precinct - Sub-Precincts	4
Figure 2.3:	DCP road network	6
Figure 2.4:	DCP Pedestrian/ Bus Connections	6
Figure 3.1:	Existing Public Transport Network	8
Figure 3.2:	Existing Bus Network	9
Figure 3.3:	Peak Hour Volume Diagram	10
Figure 4.1:	Site Layout	12
Figure 4.2:	Swept Path at roundabout	13
Figure 4.2:	Local Road Section	14
Figure 5.1:	Development Traffic Volumes	19

APPENDICES

- A. TRAFFIC SURVEY DATA
- B. SIDRA RESULTS
- C. TMAP (2006) TRAFFIC GENERATION
- **D.** SWEPT PATHS



1 Introduction

A development application (DA) is to be submitted to Penrith City Council (Council) for a proposed residential subdivision located at 731-739 Great Western Highway, Werrington. The DA specifically relates to the Stage 4 Residential component of the subdivision known within Council's Development Control Plan as the Werrington Mixed-Use area.

The Werrington Mixed-Use area comprises a development yield of 360 low density residential dwellings, 2,600m² GLFA commercial floor area, 1,370 retail floor area, and a service station. The proposed Stage 4 Residential site forms the final stage of the residential component assigned to the Werrington Mixed-Use development site and would include 134 residential dwellings. Stages 1 to 3 of the Werrington Mixed-Use includes the remaining residential dwelling provision, which is currently under construction.

The commercial component of the Werrington Mixed-Use area, the Stage 4 Commercial site sits adjacent to the subject site (Stage 4 Residential site) and will be subject to a separate DA. Notwithstanding, the impact from the adjacent commercial site has been considered as part of this study to determine the cumulative impacts to the road network.

The Werrington Mixed-Use area forms a sub-precinct within the Werrington Enterprise Living and Learning (WELL) Precinct. The future development of Werrington has been recognised by the NSW Government and Penrith City Council which led to the development of the WELL Precinct which sets out the objectives, guiding development principles and funding strategies for infrastructure improvements within the precinct.

This Traffic Impact Assessment has been prepared to examine the external traffic implications of the proposed Stage 4 Residential subdivision with a comparative assessment of the approved masterplan for the WELL Precinct.

The WELL Precinct was informed by a number of studies, including the WELL Precinct TMAP (2007)¹. The following traffic study will set out the relative transport impacts of the subject proposal with the WELL Precinct development potential.

This report is set out as follows:

- Chapter 2 discusses the Site location and the strategic planning context for the Site.
- Chapter 3 describes the existing transport conditions surrounding the site.
- Chapter 4 provides an overview of the proposed development yield and layout for the Site

¹ Transport Management and Accessibility Plan for Werrington Enterprise Living and Learning Precinct, Maunsell / AECOM, May 2007 (WELL Precinct.)

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



- Chapter 5 presents the findings of the traffic assessment of the proposal both external and internal to the Site with consideration for cumulative impacts.
- Chapter 5 presents the assessment's conclusions.

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



2 Background

2.1 Site Location

The site is located in Werrington, NSW, with a southern frontage to the Great Western Highway. The site incorporates Stage 4 of the larger staged sub-division known as the Werrington Mixed Use Area.

The Stage 4 Residential site is bounded by French Street to the west and Saint Charbel Boulevard to the north. The Western Sydney University Werrington North campus lies to the east and TAFE NSW Nepean College to the south. Sydney Train Lines T1 / T5 run in an east-west direction to the north of the site, with Kingswood station being the closest station (1.6km).

The Penrith Council Local Environmental Plan stipulates that the site is zoned as 'R3 Medium Density Residential'.

The site and its surrounds are shown below in Figure 2.1.



Figure 2.1: Locality Map

Source: Google Maps, 2017



2.2 Strategic Planning

The WELL Precinct TMAP (2007) set out the transport infrastructure requirements for the WELL Precinct and these were adopted in the WELL Precinct S.94 Development Contributions Plan. Figure 2.2 shows the extent of land subject to the WELL Precinct S94 Plan along with the sub precincts of the WELL Precinct.





As shown in Figure 2.2 the proposed development site is located within the WELL Precinct and thus development on the precinct is the subject of S94 contributions for transport infrastructure as identified by the S94 Plan.

The WELL Precinct S94 plan has utilised sub precincts as a measure to apportion contributions for works relative to the location and impact of individual sub precincts (local improvements) as well as precinct wide regional improvements.

At Section 3.2.4 of the WELL Precinct S94, the principles for apportionment cited in the strategy include:

- Developers should be required to contribute to the extent necessary to ameliorate the impacts generated by their development; and
- Growth in background levels of demand for facilities and infrastructure should be met by government and not by developers.



The sub-precinct, Werrington Mixed Use Area, containing the subject proposed development, was required as part of the S94 plan to implement the extension of French Street which is located along the western border of the Werrington Mixed Use Area precinct.

This contribution has since been implemented as part of the early stages of the Werrington Mixed Use precinct development.

Thus, the transport assessment presented herein considers the traffic and transport implications associated directly with the development proposal, namely site access arrangements and pedestrian / cycle linkages to surrounding sites, noting that the broader regional and local contributions have been planned and funded through the Section 94 Plan mechanism.

2.3 DCP Objectives and Controls for Werrington Mixed Use Area

The site lies within the Werrington Mixed Use Area, which includes the land bounded by the Main Western Rail Line, Great Western Highway, French Street and University of Western Sydney (North Campus). Relevant objectives for the planning of the area, as outlined in the DCP, are:

- To create an urban environment that optimises residential and employment opportunities that will act as a catalyst for future development for the area, and provides a mix of residential and employment generating land uses.
- To optimise the potential for use of public transport by residents, employees and visitors to the site.
- To provide a high degree of accessibility that is safe and direct both within the site and between the site and the surrounding residential areas and educational institutions.
- To provide a permeable and interconnected street system, with direct access denied to and from the Great Western Highway and a network of public thoroughfares (street and car parking areas), that accommodate the needs of vehicles, bicycles and pedestrians for efficient, convenient and safe access to all areas.
- To require the preparation for a Concept Plan for each zone on the site and provide details on the information to be included in that plan.

The key controls of the DCP that are relevant to the Stage 4 residential subdivision are shown in Figure 2.3 and Figure 2.4.

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA





Figure 2.4: DCP Pedestrian/ Bus Connections





3 Existing Transport Context

3.1 Road Network

The major roads around the site are the Great Western Highway, French Street, and Saint Charbel Boulevard.

The **Great Western Highway** is a State Road that links the Sydney CBD to Bathurst in the west. The road is two-way with a carriageway of 24 metres and three lanes in each direction. The speed limit is 60 km/hr. To the east of the Stage 4 Residential site, the number of lanes in each direction falls to two, and the carriageway reduces to 16 metres. The posted speed limit increases to 80 km/hr. The road reserve width of the Great Western Highway is 40 metres.

French Street is a local collector road. It is a two-way, two-lane road (one lane in each direction) with kerbside parking, that forms the western frontage of the site. The carriageway length is 11 metres and the posted speed limit is 50 km/hr.

Saint Charbel Boulevard is a local road that runs in an east-west direction at the northern frontage of the site. A marked shared path is located along the southern side of this, as per the planning objectives of the DCP, as shown in Figure 2.4. The proposed development (Stage 4 Residential sub-division) would be accessed from Saint Charbel Boulevard.

3.2 Public Transport

The site is serviced by bus and train services. The closest train station is Kingswood train station (20-minute walk), which services trains on the T1 Western line.

The closest bus stops are located on the Great Western Highway and O'Connell Street and are both within walking distance from the site. These stops include:

- Stop 274731, Great Western Highway after French Street (1-minute walk) which serves routes 775, 776
- Stop 274786, Western Sydney Institute Nepean College (5-minute walk) which serves routes 775, 776 and 835.

Table 3.1 outlines the frequency of services and notable stops for each public transport option. Figure 3.1 shows the location of services relative to the subject site and Figure 3.2 illustrates the existing bus route network.

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



Dublic		Frequency		Но	urs of Operatio		
Transport	Route	On- peak	Off- peak	Weekday	Sat	Sun	Notable Stops
Train (Kingswood Station)	T1 Western Line	10-15 min	15-20 min	3:16am- 00:19am	3:48am- 00:02am	3:48am- 11:32pm	Emu Plains, Blacktown, Parramatta, Sydney CBD
Bus: 775	Mount Druitt to Penrith via Erskine Park	30 min	30 min	6:01am- 11:29pm	7:16am- 11:04pm	8:15am- 9:22pm	Penrith Station, St Marys, Esrkine Park, Mount Druitt
Bus: 776	Mount Druitt to Penrith via St Clair	25-30 min	30 min	6:16am- 10:56pm	5:48am- 11:38pm	7:47am- 8:59pm	Penrith Station, St Marys, St Clair, Colyton, Mount Druitt
Bus: 835	UWS to Prairiewood	30 min	30 min	7:49am- 3:23pm	Does not operate	Does not operate	TAFE NSW Nepean Campus, St Marys, Prairiewood

Table 3.1: Public Transport

Figure 3.1:

Existing Public Transport Network



Source: Open Street Maps





Figure 3.2: Existing Bus Network

Source: Transport NSW Greater Western Sydney Bus Network Map

3.3 Traffic Volumes (October 2016)

On the 20th of October 2016, TTPP commissioned traffic surveys to quantify the peak period traffic volumes at the Great Western Highway/ French Street/O'Connell Street intersection.

A summary of the peak period two-way traffic volumes for each direction is outlined in Table 3.2 and Figure 3.3. Full results are provided in Appendix A.

Peak I	nour	O'Connell Street	Connell Street Great Western Highway (West)		Great Western Highway (East)	Total
8:00A 9:00A	M- AM	308	1353	94	951	2706
4:30P 5:30F	PM-	376	1399	77	1228	3080

Table 3.2: AM and PM Peak Traffic Volumes at Intersection

Source: TTPP Surveys (October 2016)





3.4 Existing Intersection Operation

The existing operation of the nearby intersections to the Site have been assessed using SIDRA Intersection 7, a computer based modelling package which assesses intersection performance under prevailing traffic conditions.

SIDRA calculates intersection performance measures such as 'average delay' that vehicles encounter and the level of service (LoS). SIDRA provides analysis of the operating conditions which can be compared to the performance criteria set out in Table 3.3.

Level of Service	Average Delay (seconds per vehicle)	Traffic Signals, Roundabout	Give Way and Stop Signs
A	Less than 14	good operation	good operation
В	15 to 28	good with acceptable delays and spare capacity	acceptable delays and spare capacity
С	29 to 42	satisfactory	satisfactory, but accident study required
D	43 to 56	operating near capacity	near capacity and accident study required
E	57 to 70	at capacity At signals, incidents will cause excessive delays.	at capacity, requires other control mode
F	Greater than 71	unsatisfactory with excessive queuing	unsatisfactory with excessive queuing; requires other control mode

Source: RMS Guide to Traffic Generating Developments, 2002

17254_r01v03_210202 Stg4 Residential TIA



The results of the SIDRA intersection analysis are presented in Table 3.4. The results indicate that the key intersection of O'Connell Street/ French Street/ GHW operate satisfactorily with minimum delays and significant spare capacity.

Table 3.4: Existing (2016) Intersection Operation

	Intersection Control		AM Peak Hr		PM Peak Hr		
Intersection		Degree of Saturation	Ave. Delay (sec/veh)	Level of Service	Degree of Saturation	Ave. Delay (sec/veh)	Level of Service
O'Connell St/ French St / Great Western Highway	Signals	0.737	27	В	0.796	29	С

Source: SIDRA based on TTPP Surveys (October 2016)

The full SIDRA result is provided in Appendix B.

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



4 Stage 4 Residential Development

4.1 Land Use and Layout

The Stage 4 Residential sub-division of the Werrington Mixed Use area will include 134 medium density residential dwellings. The site layout and road network is as shown in Figure 4.1.



Source: Statewide Planning Pty Ltd

Access to the Stage 4 subdivision is provided off the main access road, Saint Charbel Boulevard, located along the northern boundary. Two access points are permitted including North South Road No. 3 which connects to Saint Charbel Boulevard as a roundabout controlled intersection and North South Road No. 2 which connects to Saint Charbel Boulevard as a westbound left-in/left-out, T intersection.

Saint Charbel Boulevard functions as an internal collector road in the wider Werrington Mixed Use area, forming a connection between French Street (to the west) and Kirsh Street (to the east), both roads providing access to the arterial road network.

French Street provides the primary access into the Werrington sub-division as it connects to the Great Western Highway as a signalised intersection. Kirsh Street forms secondary access with left-in/ left-out access restrictions from the Great Western Highway.

The road network and access into the site is consistent with Council's DCP, as shown in Figure 2.3. The internal road layout has been designed to accommodate Council's 12.5m long waste and recycling collection vehicle. Swept path analysis of Council's waste vehicle is provided in Appendix D.

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



Swept path analysis indicates that the existing northern median island at the roundabout intersection of Saint Charbel Boulevard and Abacus Parade, would need to be trimmed to accommodate a 12.5m long vehicle right turn from the site, as shown in Figure 4.2. It is understood that the existing roundabout would be modified as part of this development.



Figure 4.2: Swept Path at roundabout

4.1.1 Road Width and Driveways

Internal roads are designed as per the DCP (Part C10 Transport Access and Parking) requirements for a local road, which includes:

- 15.6m road reserve
- 3m wide carriageway)
- 2.5m wide parking lanes on both sides of the road
- 1.5m wide concrete pathways, on the dwelling side
- 3.8m wide verge widths on both sides of the road.

The section plan of a typical local road is shown in Figure 4.3.

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA





Source: Penrith Development Control Plan 2014 Part C10 Transport Access and Parking

The exception is the service lane which is to provide vehicular access to lots 34-39 and lot 70. This service lane is to be 6.5m wide and ensures that driveways are not located along French Street or within close proximity to French Street, on Saint Charbel Boulevard.

Driveways are generally 3.5m wide. At corner lots, driveways have been positioned at least 6m clear of the tangent points of intersection corners, as per AS2890.1:2004 requirements.

Based on the swept path analysis in Appendix D, "No Stopping" signage should be applied near intersection corners and bends in the road to ensure that cars to do not park in the statutory no stopping areas and that there is sufficient room for truck turning. This is in line with NSW Road Rules 2008, which requires no stopping 10m from an intersection unless specifically reduced by Council and TfNSW.

It is assumed that appropriate signage and linemarking plans would be prepared as part of the Construction Certification stage.

4.2 Public Transport

The WELL Precinct TMAP envisaged that additional development within the WELL Precinct would increase the demand for and viability of additional public transport services within the Precinct. Based on this expectation, French Street and St Charbel Boulevard are planned as future bus routes in the Penrith DCP as shown in Figure 2.4.

The WELL Precinct TMAP and Section 94 Plan has also identified a plan for a Western Sydney University railway station (northern end of French Street) which if implemented, together with

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



regular bus services, will significantly increase the attractiveness of public transport modes for travel to the site.

4.3 Pedestrians and Cyclists

The DCP 2014 envisages improved bicycle and pedestrian connections through the site and from the Great Western Highway.

The internal road network of the site has been developed with consideration of connections to existing and proposed linkages external to the site along with through site links as identified in the DCP (see Figure 2.3).

Pedestrian paths are provided along all roads within the subdivision. All footpaths are 1.5m wide as per DCP requirements.

Pedestrian permeability is ensured by the provision of a pathway between lot 27 and 28 that connects East West Road No. 2 and the Great Western Highway and French Street intersection.

The existing path along the southern side Saint Charbel Boulevarde is a 2.5m wide marked shared path, that currently runs between French Street and Kirsh Street. The statutory shared path signage is present at the intersections of French Street, Abacus Parade and Kirsh Street. New signage would be required at each end of the proposed subdivision intersections to notify pedestrians and cyclists of the presence of a shared path.

It is assumed that appropriate signage and linemarking plans for shared path signage would be prepared as part of the Construction Certification stage.

4.4 Parking Requirements

The Penrith Development Control Plan 2014 (DCP) sets minimum requirements for off-street car parking provision. For a residential dwelling house, the DCP requires two spaces per dwelling.

It is envisaged, that each residential dwelling will include a garage for at least one space, with the driveway allowing an additional stacked parking space.



5 Traffic Assessment

5.1 WELL Precinct Masterplan

The WELL Precinct Masterplan as per the TMAP², has allowed for a yield of 240 dwellings and 40,000m² GLFA commercial area (supporting an estimate of 646 jobs) for the Werrington Mixed Use Area.

Using traffic generation rates from the Roads and Maritime Services (RMS) Guide to Traffic Generating Developments 2002, the TMAP estimated a traffic generation of 717vph during the AM peak and 728vph during the PM peak. The full traffic generation estimate for the WELL Precinct as detailed in the TMAP is provided in Appendix C.

The impact of the WELL Precinct on the road network, was assessed in the TMAP with modelling undertaken by URS using NETANAL modelling software. The network modelling included two forecasted scenarios:

- Base Case: with WELL Precinct and committed developments including the upgrade of Werrington Arterial and associated M4 ramps by RMS3
- Network Improvement Case: Base Case plus additional network upgrades required to bring all intersections and links to LoS D or better.

The modelling results of the two scenarios are presented in Table 5.1.

Table 5.1: TMAP Intersection Modelling Results

O'Connell St/ French St / Great	AM Peo	ak Hour	PM Peak Hour		
Western Highway	Ave. Delay (sec/veh)	Level of Service	Ave. Delay (sec/veh)	Level of Service	
Base Case	188	F	203	F	
Network Improvement Case	49	D	46	D	

Source: Maunsell/ AECOM, 2006

As discussed in Section 2.2, the traffic modelling undertaken for the WELL Precinct identifies road network upgrades that would be required to enable the resultant growth from the WELL Precinct Masterplan. The upgrade requirements have been included in a S94 contributions plan, with the Werrington Mixed Use Area requiring only to implement the extension of French Street (now complete).

² Transport Management and Accessibility Plan for Werrington Enterprise Living and Learning Precinct, Maunsell / AECOM, May 2007 (WELL Precinct.)

³ RMS Werrington Arterial Road project has been completed in May 2017

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



5.2 Proposed Traffic Generation

Using the same rates as the TMAP, that is, 0.5 trips per dwelling and 2 trips per 100m² office floor area, the Werrington Mixed Use Area with its current yield would generate 550 and 571vph during the AM and PM peaks respectively. Therefore, the precinct has reduced in traffic generation since the master planning stage.

However, based on the latest practice and updated traffic data, the actual traffic generating characteristics of the site's land uses are considered to be higher than was estimated in the TMAP.

That is, latest engineering practice recommends the site to be taken as a low density subdivision rather than medium density when calculating the future traffic generation. Furthermore, updated survey data released by RMS in its 2013 Technical Direction TDT 2013/04a (herein RMS TDT) provides new rates for residential and commercial developments.

Based on the rates stipulated in the RMS TDT for commercial and low density residential, the estimated traffic generation of the subject development (Stage 4 Residential site) is summarised in Table 5.2 and the overall traffic generation of the Werrington Mixed Use area is summarised in Table 5.3.

Table 5.2: Proposed Development Traffic

Land Use	Size	Trip R	ate	Trips (mph)		
Land Use	(units/sqm)	AM	PM	AM	PM	
Residential	134 units	0.95 per unit	0.99 per unit	127	133	

Table 5.3: Cumulative Traffic Generation of Werrington Mixed Use Area

Land Lise	Size	Trip R	ate	Trips	(veh)
Luna use	(units/sqm)	AM	PM	AM	PM
Residential	360 units	0.95 per unit	0.99 per unit	342	356
Commercial	2,600m ²	1.60 per 100m ²	1.20 per 100m ²	42	31
Retail	920m ²	2.30 per 100m ²	4.60 per 100m ²	21	42
Service Station	450m ²	0.66 x Retail GFA1	0.66 x Retail GFA	297	297
Total	-	-	-	702	727

1 The RMS guide stipulates a trip rate for Service Stations for the PM peak only. It has been conservatively assumed that the AM peak traffic generation would equate to the PM peak.

Table 5.2 indicates that the proposed Stage 4 Residential component of the Werrington subdivision will generate 127 and 133 vehicles per hour (mph) during the morning and afternoon peaks respectively.

However, to appropriately assess the impact of the proposed development, it is necessary to review the cumulative impact of the whole Werrington sub-division. In this regard, the overall Werrington sub-division, with 360 residential dwellings, 2,600m² commercial floor area, 920m²

¹⁷²⁵⁴_r01v03_210202 Stg4 Residential TIA



retail and a service station containing a 450m² convenience store, is estimated to generate 702 and 727vph during the morning and afternoon peaks respectively (Table 5.3).

The current estimate of the traffic generation of the Werrington Mixed Use area is therefore in line with the original estimate of the WELL Precinct TMAP (Section 5.1).

5.2.1 Traffic Distribution

Based on the cumulative traffic generation estimate presented in Table 5.3, the site generated traffic has been distributed based on typical in/ out splits:

- AM peak hour
 - Residential traffic: 20 per cent IN/ 80 per cent OUT
 - Commercial traffic: 80 per cent IN/ 20 per cent OUT
- PM peak hour
 - Residential traffic: 80 per cent IN/ 20 per cent OUT
 - · Commercial traffic: 20 per cent IN/ 80 per cent OUT.

The site traffic was then distributed into the road network based on Journey to Work data (Bureau of Transport Statistics) for the site and surrounds, which provided the following approximate directional distribution along the Great Western Highway:

- Residential traffic: 50 per cent to the west, 50 per cent to the east
- Commercial traffic: 60 per cent to the west, 40 per cent the east.

The model assumes all residential traffic will access/leave the site via the O'Connell Street/ French Street/ Great Western Highway intersection. The commercial and retail traffic will access the site via both the French Street and Kirsh Street access points, whereas the service station will have direct access from the Great Western Highway.

The development traffic volumes and distribution is shown in Figure 5.1.





5.3 Intersection Modelling

Notwithstanding the results of the TMAP, TTPP have undertaken a localised impact assessment on the key access road to the Werrington Mixed Use area; the intersection of O'Connell Street/ French Street/ Great Western Highway.

To assess the impact of the proposed development, the intersection was assessed for the following two scenarios:

- Scenario 1 Post Development Existing plus Werrington Mixed Use Area traffic
- Scenario 2 Cumulative WELL Precinct Existing plus Werrington Mixed Use Area and WELL Precinct

17254_r01v03_210202 Stg4 Residential TIA



Scenario 1 has been assessed based on the trip generation and distribution detailed in Section 5.2. Scenario 2 has been assessed by obtaining traffic volumes from a previous DA study for the WELL Precinct (TTPP, 2017, 46-66 & 46A O'Connell Street, Caddens, Concept Plan DA, Traffic Impact Assessment)

The results of the SIDRA model are summarised in Table 5.4 with full results in Appendix B.

		AM Peak Hr			PM Peak Hr	
Scenario	Degree of Saturation	Ave. Delay (sec/veh)	Level of Service	Degree of Saturation	Ave. Delay (sec/veh)	Level of Service
Post Development	0.832	33	С	0.979	37	С
Cumulative WELL Precincts	1.221	60	E	1.930	79.7	F

Table 5.4: Post Development Intersection Operation

Table 5.4 indicates that the key intersection to the development would operate satisfactorily with the estimated traffic of the Werrington Mixed Use Area.

The cumulative impacts from the WELL Precinct will result in the subject intersection to reach capacity. The result is close to the TMAP in this regard (See Table 5.5), however the SIDRA analysis as part of this assessment has identified lower delays than the TMAP's NETANAL model.

Table 5.5: Post Development Intersection Operation WELL PRECINCT TMAP

O'Connell St/ French St / Great	AM Peo	ak Hour	PM Peo	ık Hour
Western Highway	Ave. Delay (sec/veh)	Level of Service	Ave. Delay (sec/veh)	Level of Service
Base Case	188	F	203	F
Network Improvement Case	49	D	46	D

Source: Maunsell/ AECOM, 2006

As can be seen from Table 5.5, the scenario with the Network improvements will improve the performance to Level of Service D.

Based on this, it is expected that the road network improvements proposed as part of the TMAP and S94 plan would similarly address the capacity issues identified in the cumulative development scenario shown in Table 5.4.

Overall, the assessment indicates that the access arrangements via French Street and Kirsh Street are sufficient to accommodate the traffic from the Werrington Mixed Use area.



6 Conclusion

The above traffic impact assessment has been prepared to support a Development Application for the Stage 4 Residential sub-division of the Werrington Mixed Use Area, in Werrington, NSW. The Site is located within the area referred to as the WELL Precinct which is an area recognised by State Government and Penrith City Council for future potential redevelopment as an enterprise, living and learning precinct.

Based on the findings of this assessment the following conclusions have been made:

- The subject development application relates to a residential sub-division for 134 dwellings.
- The proposed site master plan will facilitate the provision of public transport networks (bus), cyclist and pedestrian routes as envisaged in the Penrith DCP 2014.
- Traffic generation of the overall Werrington Mixed Use Area is estimated to be in the order of 702 and 727 vehicles per hour during the AM and PM peak periods respectively.
- The estimated traffic generation of the Werrington Mixed Use Area is in line with original planning for WELL Precinct, which included a traffic generation of 717 and 728 vehicles per hour during the AM and PM peak periods respectively, as estimated in the TMAP.
- The existing site access arrangements via French Street and Kirsh Street is sufficient for the Werrington Mixed Use development.
- Road network improvements are proposed in the wider WELL Precinct as part of the S94 plan, which is expected to address capacity issues associated with traffic generated from the whole WELL precinct.



Appendix A

Traffic survey data

17254_r01v03_210202 Stg4 Residential TIA

Job No.	: N2749
Client	: TTPP
Suburb	: Kingswood
Location	: 1. O'Connell St / Great Western Hwy / French St
Day/Date	: Thursday, 20th October 2016
Weather	: Fine
Description	: Classified Intersection Count
	: Hourly Summary





Approach								O'Co	nnell St														G	reat We	stern H	wy						
Direction		Direc (Left	tion 1 Turn)			Direo (Thr	ction 2 ough)			Direc (Right	tion 3 t Turn)			Direct (U 1	tion 3U Turn)			Dire (Left	tion 4 Turn)			Direc (Thre	tion 5 ough)			Direc (Right	tion 6 t Turn)			Direct (U 1	tion 6U Furn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
7:00 to 8:00	66	1	0	67	9	1	0	10	163	4	7	174	0	0	0	0	102	3	7	112	634	28	3	665	15	0	0	15	0	0	0	0
7:15 to 8:15	71	2	0	73	7	1	0	8	208	5	7	220	0	0	0	0	133	2	8	143	715	25	3	743	15	0	0	15	0	0	0	0
7:30 to 8:30	80	1	0	81	7	0	0	7	229	3	6	238	0	0	0	0	198	3	9	210	819	25	3	847	12	0	0	12	0	0	0	0
7:45 to 8:45	86	1	0	87	8	0	0	8	214	1	7	222	0	0	0	0	265	2	8	275	929	24	2	955	12	0	0	12	0	0	0	0
8:00 to 9:00	73	4	0	77	7	0	0	7	216	1	7	224	0	0	0	0	343	2	9	354	959	24	4	987	11	1	0	12	0	0	0	0
AM Totals	139	5	0	144	16	1	0	17	379	5	14	398	0	0	0	0	445	5	16	466	1,593	52	7	1,652	26	1	0	27	0	0	0	0
16:00 to 17:00	110	0	1	111	10	1	0	11	297	1	10	308	0	0	0	0	225	1	9	235	1,172	24	2	1,198	17	2	0	19	0	0	0	0
16:15 to 17:15	96	0	1	97	14	1	0	15	285	1	9	295	0	0	0	0	226	1	9	236	1,130	21	3	1,154	18	1	0	19	0	0	0	0
16:30 to 17:30	92	0	1	93	15	1	0	16	255	3	9	267	0	0	0	0	233	1	8	242	1,110	22	4	1,136	21	0	0	21	0	0	0	0
16:45 to 17:45	83	0	1	84	14	0	0	14	253	3	8	264	0	0	0	0	228	3	9	240	1,154	20	3	1,177	32	0	0	32	0	0	0	0
17:00 to 18:00	81	0	0	81	12	0	0	12	232	2	7	241	0	0	0	0	222	2	10	234	1,149	16	4	1,169	34	0	0	34	0	0	0	0
PM Totals	191	0	1	192	22	1	0	23	529	3	17	549	0	0	0	0	447	3	19	469	2,321	40	6	2,367	51	2	0	53	0	0	0	0

Approach								Fren	ch St														Gi	reat We	stern Hv	vy						
Direction		Direc (Left	tion 7 Turn)			Direct (Thro	tion 8 ough)			Direc (Right	tion 9 Turn)			Direct (U T	ion 9U 'urn)			Direct (Left	ion 10 Turn)			Direct (Thro	ion 11 ough)			Directi (Right	ion 12 Turn)			Directi (U T	on 12U 'urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
7:00 to 8:00	29	0	0	29	2	1	0	3	34	4	0	38	0	0	0	0	9	4	0	13	713	34	9	756	42	3	4	49	0	0	0	0
7:15 to 8:15	30	1	0	31	5	2	0	7	37	4	0	41	0	0	0	0	7	4	0	11	786	33	9	828	55	1	5	61	0	0	0	0
7:30 to 8:30	26	1	0	27	7	2	0	9	38	3	0	41	0	0	0	0	10	3	0	13	777	27	6	810	79	0	7	86	0	0	0	0
7:45 to 8:45	22	2	0	24	12	1	0	13	49	1	0	50	0	0	0	0	15	2	0	17	782	27	3	812	101	0	8	109	0	0	0	0
8:00 to 9:00	22	2	0	24	20	1	0	21	48	1	0	49	0	0	0	0	12	2	0	14	759	29	1	789	140	0	8	148	0	0	0	0
AM Totals	51	2	0	53	22	2	0	24	82	5	0	87	0	0	0	0	21	6	0	27	1,472	63	10	1,545	182	3	12	197	0	0	0	0
16:00 to 17:00	17	1	0	18	8	0	0	8	36	0	0	36	0	0	0	0	8	0	0	8	973	13	1	987	101	1	8	110	0	0	0	0
16:15 to 17:15	15	1	0	16	9	0	0	9	44	0	0	44	0	0	0	0	9	0	0	9	1,029	14	1	1,044	115	0	8	123	0	0	0	0
16:30 to 17:30	20	2	0	22	12	0	0	12	43	0	0	43	0	0	0	0	9	0	0	9	1,091	11	1	1,103	108	0	8	116	0	0	0	0
16:45 to 17:45	22	2	0	24	15	0	0	15	40	0	0	40	0	0	0	0	9	0	0	9	1,043	13	1	1,057	100	0	7	107	0	0	0	0
17:00 to 18:00	19	1	0	20	15	0	0	15	39	0	0	39	0	0	0	0	7	0	0	7	998	13	2	1,013	86	0	5	91	0	0	0	0
PM Totals	36	2	0	38	23	0	0	23	75	0	0	75	0	0	0	0	15	0	0	15	1,971	26	3	2,000	187	1	13	201	0	0	0	0



Appendix B

SIDRA Results

17254_r01v03_210202 Stg4 Residential TIA

Site: 101 [O'Connell St / Great Western Hwy / French St - Ex AM Peak]

Signals - Fixed Time Isolated	Cycle Time = 110 seconds	(User-Given Phase Times)
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Move	ement P	erformance	e - Vehio	cles							
Mov	OD Mov	Demand	I Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
<u> </u>	IVIOV	veh/h	пv %	v/c	sec	Service	venicies veh	Distance	Queueu	per veh	speed km/h
South	: O'Conr	nell St - S									
1	L2	81	5.2	0.125	26.8	LOS B	3.0	21.6	0.66	0.69	38.3
2	T1	7	0.0	0.125	22.2	LOS B	3.0	21.6	0.66	0.69	36.8
3	R2	236	3.6	0.668	44.4	LOS D	11.5	83.3	0.94	0.84	34.0
Appro	ach	324	3.9	0.668	39.5	LOS C	11.5	83.3	0.87	0.80	35.0
East:	Great W	estern Hwy -	E								
4	L2	373	3.1	0.470	29.1	LOS C	14.1	101.3	0.76	0.80	37.8
5	T1	1039	2.8	0.627	25.6	LOS B	21.7	155.6	0.83	0.74	42.2
6	R2	13	8.3	0.159	64.0	LOS E	0.7	5.3	0.99	0.68	27.8
Appro	ach	1424	3.0	0.627	26.9	LOS B	21.7	155.6	0.81	0.75	40.8
North:	French	St - N									
7	L2	25	8.3	0.051	30.1	LOS C	1.1	8.1	0.69	0.65	38.7
8	T1	22	4.8	0.180	31.9	LOS C	2.9	20.5	0.78	0.70	33.7
9	R2	52	2.0	0.180	38.6	LOS C	2.9	20.5	0.81	0.71	34.6
Appro	ach	99	4.3	0.180	34.9	LOS C	2.9	20.5	0.77	0.69	35.3
West:	Great W	/estern Hwy -	W								
10	L2	15	14.3	0.352	21.3	LOS B	7.3	52.9	0.59	0.52	43.6
11	T1	831	3.8	0.352	16.0	LOS B	10.8	78.1	0.61	0.52	52.7
12	R2	156	5.4	0.737	60.2	LOS E	8.7	63.5	1.00	0.86	28.7
Appro	ach	1001	4.2	0.737	23.0	LOS B	10.8	78.1	0.67	0.58	46.5
All Ve	hicles	2848	3.5	0.737	27.2	LOS B	21.7	155.6	0.77	0.69	41.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - P	edestrians						
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P2	East Full Crossing	53	44.6	LOS E	0.1	0.1	0.90	0.90
P4	West Full Crossing	53	44.6	LOS E	0.1	0.1	0.90	0.90
All Pe	destrians	105	44.6	LOS E			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [O'Connell St / Great Western Hwy / French St - Ex PM Peak]

Signals - Fixed Time Isolated	Cycle Time = 150 seconds (User-Given Phase Times)
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Move	ment P	erformance -	Vehio	cles							
Mov ID	OD Mov	Demand F Total veh/h	lows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	O'Conr	nell St - S									
1	L2	98	1.1	0.211	46.3	LOS D	6.2	44.1	0.79	0.74	31.9
2	T1	17	6.3	0.211	41.7	LOS C	6.2	44.1	0.79	0.74	30.8
3	R2	281	0.0	0.523	53.4	LOS D	16.8	117.8	0.88	0.80	31.6
Appro	ach	396	0.5	0.523	51.1	LOS D	16.8	117.8	0.86	0.78	31.7
East: (Great W	estern Hwy - E									
4	L2	255	3.7	0.483	28.8	LOS C	21.6	155.1	0.68	0.71	39.0
5	T1	1196	2.3	0.483	23.1	LOS B	22.3	159.1	0.67	0.62	43.3
6	R2	22	0.0	0.223	82.2	LOS F	1.6	11.4	0.99	0.71	24.5
Appro	ach	1473	2.5	0.483	24.9	LOS B	22.3	159.1	0.68	0.64	42.0
North:	French	St - N									
7	L2	23	9.1	0.049	41.8	LOS C	1.3	10.0	0.72	0.66	34.3
8	T1	13	0.0	0.170	47.0	LOS D	3.2	22.5	0.81	0.71	29.6
9	R2	45	0.0	0.170	55.7	LOS D	3.2	22.5	0.85	0.73	29.7
Appro	ach	81	2.6	0.170	50.4	LOS D	3.2	22.5	0.81	0.71	30.9
West:	Great W	/estern Hwy - V	V								
10	L2	9	0.0	0.528	23.5	LOS B	13.0	91.6	0.57	0.50	42.6
11	T1	1161	1.1	0.528	18.7	LOS B	19.8	140.2	0.59	0.52	50.8
12	R2	122	6.9	0.796	84.7	LOS F	9.5	70.2	1.00	0.88	24.1
Appro	ach	1293	1.6	0.796	25.0	LOS B	19.8	140.2	0.63	0.55	45.9
All Vel	nicles	3242	1.9	0.796	28.8	LOS C	22.3	159.1	0.68	0.62	41.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pe	destrians						
Mov ID	Description	Demand Flow	Average Delav	Level of Service	Average Bacl Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P2	East Full Crossing	53	57.3	LOS E	0.2	0.2	0.88	0.88
P4	West Full Crossing	53	57.3	LOS E	0.2	0.2	0.88	0.88
All Pe	destrians	105	57.3	LOS E			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [O'Connell St / Great Western Hwy / French St - Post Dev AM Peak]

Move	ment P	Performance	- Vehio	cles							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	O'Conr	nell St - S									
1	L2	81	5.2	0.105	21.6	LOS B	2.6	18.9	0.58	0.67	40.5
2	T1	7	0.0	0.105	17.0	LOS B	2.6	18.9	0.58	0.67	38.8
3	R2	236	3.6	0.686	43.2	LOS D	11.6	83.6	0.93	0.85	34.3
Approa	ach	324	3.9	0.686	37.2	LOS C	11.6	83.6	0.84	0.80	35.8
East: 0	Great W	/estern Hwy - E									
4	L2	373	3.1	0.564	35.7	LOS C	16.0	115.0	0.86	0.83	35.4
5	T1	1039	2.8	0.778	34.6	LOS C	26.5	190.0	0.94	0.86	38.3
6	R2	69	1.5	0.832	70.7	LOS F	4.2	29.9	1.00	0.90	26.5
Approa	ach	1481	2.8	0.832	36.6	LOS C	26.5	190.0	0.92	0.85	36.8
North:	French	St - N									
7	L2	198	1.1	0.322	24.1	LOS B	6.4	45.3	0.65	0.73	41.9
8	T1	22	4.8	0.550	30.6	LOS C	8.1	57.2	0.82	0.77	34.0
9	R2	172	0.6	0.550	35.1	LOS C	8.1	57.2	0.82	0.77	35.5
Approa	ach	392	1.1	0.550	29.3	LOS C	8.1	57.2	0.73	0.75	38.4
West:	Great W	Vestern Hwy - \	W								
10	L2	54	3.9	0.446	26.4	LOS B	8.6	62.1	0.68	0.62	40.8
11	T1	831	3.8	0.446	21.6	LOS B	13.9	100.5	0.71	0.62	48.6
12	R2	156	5.4	0.737	60.2	LOS E	8.7	63.5	1.00	0.86	28.7
Approa	ach	1040	4.0	0.737	27.6	LOS B	13.9	100.5	0.75	0.65	43.6
All Veh	nicles	3237	3.1	0.832	32.9	LOS C	26.5	190.0	0.83	0.77	38.8

Signals - Fixed Time Isolated Cycle Time = 110 seconds (User-Given Phase Times)

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Peo	destrians						
Mov		Demand	Average	Level of	Average Bacl	c of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P2	East Full Crossing	53	37.7	LOS D	0.1	0.1	0.83	0.83
P4	West Full Crossing	53	37.7	LOS D	0.1	0.1	0.83	0.83
All Pe	destrians	105	37.7	LOS D			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [O'Connell St / Great Western Hwy / French St - Post Dev PM Peak]

Move	ment P	Performance	- Vehio	cles							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	O'Conr	nell St - S									
1	L2	98	1.1	0.209	46.3	LOS D	6.1	43.7	0.79	0.74	31.9
2	T1	17	6.2	0.513	42.1	LOS C	16.8	117.8	0.79	0.74	30.7
3	R2	281	0.0	0.513	53.2	LOS D	16.8	117.8	0.88	0.80	31.7
Approa	ach	396	0.5	0.513	51.0	LOS D	16.8	117.8	0.85	0.78	31.7
East: 0	Great W	/estern Hwy - E									
4	L2	255	3.7	0.516	28.2	LOS B	23.8	170.9	0.68	0.70	39.3
5	T1	1196	2.3	0.516	21.9	LOS B	24.5	174.9	0.66	0.61	43.9
6	R2	205	0.0	0.979	112.0	LOS F	19.2	134.4	1.00	1.06	20.4
Approa	ach	1656	2.2	0.979	34.0	LOS C	24.5	174.9	0.70	0.68	37.8
North:	French	St - N									
7	L2	68	3.1	0.113	38.4	LOS C	3.6	25.7	0.70	0.70	35.9
8	T1	13	0.0	0.396	44.8	LOS D	6.2	43.2	0.80	0.74	30.0
9	R2	91	0.0	0.396	60.9	LOS E	6.2	43.2	0.91	0.77	28.4
Approa	ach	172	1.2	0.396	50.7	LOS D	6.2	43.2	0.82	0.74	31.1
West:	Great W	Vestern Hwy - \	Ν								
10	L2	132	0.0	0.692	31.3	LOS C	16.7	117.5	0.68	0.66	38.3
11	T1	1161	1.1	0.692	27.5	LOS B	28.9	204.1	0.73	0.66	45.0
12	R2	122	6.9	0.941	101.8	LOS F	10.6	78.9	1.00	1.01	21.6
Approa	ach	1415	1.5	0.941	34.2	LOS C	28.9	204.1	0.75	0.69	40.6
All Veh	nicles	3638	1.7	0.979	36.7	LOS C	28.9	204.1	0.74	0.70	37.6

Signals - Fixed Time Isolated Cycle Time = 150 seconds (User-Given Phase Times)

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pe	destrians						
Mov ID	Description	Demand Flow	Average Delav	Level of Service	Average Bacl Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P2	East Full Crossing	53	57.3	LOS E	0.2	0.2	0.88	0.88
P4	West Full Crossing	53	57.3	LOS E	0.2	0.2	0.88	0.88
All Pe	destrians	105	57.3	LOS E			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [O'Connell St / Great Western Hwy / French St - WELL Precinct + Post Dev AM Peak]

Signals - Fixed Time Isolated Cycle Time = 110 seconds (User-Given Phase Times)

Move	ement Pe	erformance	- Vehic	les							
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Ocuth	010	veh/h	%	V/C	sec		veh	m		per veh	km/h
South	: O'Conne	ell St - S									
1	L2	135	3.1	0.171	22.2	LOS B	4.4	31.8	0.61	0.69	40.3
2	T1	12	0.0	0.171	17.6	LOS B	4.4	31.8	0.61	0.69	38.5
3	R2	394	2.1	1.221	267.0	LOS F	56.4	401.8	1.00	1.83	10.9
Appro	bach	540	2.3	1.221	200.6	LOS F	56.4	401.8	0.89	1.52	13.5
East:	Great We	estern Hwy - E									
4	L2	493	2.4	0.742	38.8	LOS C	23.2	165.9	0.93	0.87	34.3
5	T1	1039	2.8	0.778	34.6	LOS C	26.5	190.0	0.94	0.86	38.3
6	R2	69	1.5	0.832	70.7	LOS F	4.2	29.9	1.00	0.90	26.5
Appro	bach	1601	2.6	0.832	37.4	LOS C	26.5	190.0	0.94	0.86	36.3
North	: French S	St - N									
7	L2	198	1.1	0.335	24.1	LOS B	6.4	45.4	0.65	0.73	41.9
8	T1	28	3.7	0.629	32.8	LOS C	8.7	61.6	0.85	0.78	33.3
9	R2	172	0.6	0.629	37.3	LOS C	8.7	61.6	0.85	0.78	34.8
Appro	ach	398	1.1	0.629	30.4	LOS C	8.7	61.6	0.75	0.75	37.9
West:	Great We	estern Hwy - \	Ν								
10	L2	54	3.9	0.446	26.4	LOS B	8.6	62.1	0.68	0.62	40.8
11	T1	831	3.8	0.446	21.6	LOS B	13.9	100.5	0.71	0.62	48.6
12	R2	205	4.1	0.963	85.1	LOS F	14.4	104.4	1.00	1.10	24.0
Appro	bach	1089	3.9	0.963	33.8	LOS C	14.4	104.4	0.76	0.71	40.4
All Ve	hicles	3628	2.8	1.221	59.9	LOS E	56.4	401.8	0.86	0.90	29.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Ped	estrians						
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P2	East Full Crossing	53	37.7	LOS D	0.1	0.1	0.83	0.83
P4	West Full Crossing	53	37.7	LOS D	0.1	0.1	0.83	0.83
All Pe	destrians	105	37.7	LOS D			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [O'Connell St / Great Western Hwy / French St - WELL Precinct + Post Dev PM Peak]

Signals - Fixed Time Isolated Cycle Time = 150 seconds (User-Given Phase Times)

Move	ment P	erformance ·	- Vehic	cles							
Mov	OD	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 (1)	010	veh/h	%	v/c	sec		veh	m		per veh	km/h
South	: O'Conn	ell St - S									
1	L2	106	1.0	0.225	45.8	LOS D	6.7	47.5	0.79	0.74	32.1
2	T1	18	5.9	0.225	41.2	LOS C	6.7	47.5	0.79	0.74	30.9
3	R2	306	0.0	0.603	54.9	LOS D	18.7	131.2	0.90	0.81	31.2
Appro	ach	431	0.5	0.603	52.1	LOS D	18.7	131.2	0.87	0.79	31.4
East:	Great We	estern Hwy - E									
4	L2	537	1.8	0.640	32.9	LOS C	31.7	225.6	0.78	0.82	36.6
5	T1	1196	2.3	0.640	26.1	LOS B	33.0	235.6	0.74	0.68	41.9
6	R2	205	0.0	1.184	251.8	LOS F	30.3	212.1	1.00	1.37	11.1
Appro	ach	1938	1.9	1.184	51.9	LOS D	33.0	235.6	0.78	0.79	31.4
North:	French \$	St - N									
7	L2	68	3.1	0.116	39.1	LOS C	3.6	25.8	0.71	0.70	35.7
8	T1	26	0.0	0.408	49.5	LOS D	6.9	48.6	0.85	0.75	29.0
9	R2	91	0.0	0.408	58.3	LOS E	6.9	48.6	0.89	0.77	29.1
Appro	ach	185	1.1	0.408	50.0	LOS D	6.9	48.6	0.82	0.74	31.2
West:	Great W	estern Hwy - V	V								
10	L2	132	0.0	0.683	27.8	LOS B	16.6	117.1	0.64	0.63	39.8
11	T1	1161	1.1	0.683	23.0	LOS B	23.4	165.6	0.66	0.60	47.7
12	R2	257	3.3	1.415	446.0	LOS F	51.7	372.1	1.00	1.68	6.8
Appro	ach	1549	1.4	1.415	93.5	LOS F	51.7	372.1	0.72	0.78	23.8
All Ve	hicles	4103	1.5	1.415	67.5	LOS E	51.7	372.1	0.77	0.78	28.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Ped	estrians						
Mov	Description	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P2	East Full Crossing	53	57.3	LOS E	0.2	0.2	0.88	0.88
P4	West Full Crossing	53	57.3	LOS E	0.2	0.2	0.88	0.88
All Pe	destrians	105	57.3	LOS E			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Appendix C

TMAP (2006) Traffic Generation

17254_r01v03_210202 Stg4 Residential TIA

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WELL Precinct Traffic Generation

						Dev	stonment Or	portunities	DOI									End atz	ste Trip Genv	eration				1
LAND USE	PARCEL	PARCEL T	Total GFA	GFA	F8R (2)	0LFA	Jobe	GLFAJob	Land area	Denctty (4)	dwellings	enrolmente	AMRIA	PMRIA		AM PEAK	HOUR TRI	P.S		-	M PEAK HO	UR TRIPS		
	NO.	NO.	(m2)	0.000				(3)			No. of Concession		Late	Late	Total	% m %	6 Out	ē	out Tot	* IN	0* 5	E H	out	
North Wernington Medium Density Residential	÷												05.0	0:0		10%	90%		0	6	96 10%	0	•	-
Medium Density Residential	2	D2							3.1	8	93		0:50	0:50	12	10%	90%	5	42 47	7 90	96 1D%	42	un.	
Sub total				0		0	0				83	0			47			6	42 47	7		42	9	
Werrington Mixed Use Area									1		Same S		No.				1000				1	No.	2	10
Medium Density Residential	m	5							ŋ	8	117		05.0	0.50	5	9601	80%	0	8	6	201 101 101	8	ø	
Medium Density Residential	4	5			10700				4	R	123		050	05.0	3	10%	906	ø	3	8	200 F	8	ω	
Mixed Use/Employment High Tech	ŝ	Ψ	Surgers.	38,342	0.50	19,171	-	1	10004	į			0.02	0.02	383	9658	15%	378	38	0 1	#58 #2#	89	326	
Mixed UselEmployment High Tech	w	Ē	80,000	14,378	0.50	7,189	9	3					0.02	0.02	4	9658	15%	1 1 1	4	4	% 85%	1	121	
Mixed Use/Employment High Tech (Industrial)	5,6	Ē	Story -	27,280	0.50	13,640	220	ä	20	1	31		0.32	0.37	R	9658	15%	5	0	15	458 96	1	8	33
Sub total			10 0	80,000	100	40.000	646		2		240	0	10		212	1	100	620 1	88 72			196	628	
UWS North Wernington Campus																								Г
Employment High Tech	1	۵		101,740	0.50	50,870	1						0.015	0.015	763	9658	15%	649	14 76	15	158 %	114	649	
Employment High Tech	-00	C	582.000	388.511	0.50	154,255	5,167	47					0.015	0.015	2914	9536	15%	4 7740	37 291	14 15	96 859	437	2477	
Employment High Tech (Industrial)	7,8	1 61		91,749	05.0	45,874	195	ł					0.32	0.37	308	958	15	12	1 22	- m	158 %		8	
Sub total	00000	10000		682,000	0111 10111	281,000	8,134		100		0	0	Description of the	10000	1,884	10.000	1	1,387 6	88 4,0	30	100-100 m	604	3,426	1
South Werrington Urban Village		10000								1000	1.000			1.000							100			
LowMedium Density Residential	57.6	5							252	\$	378		09.0	0.60	227	10%	90%	R	22 22	106	% 10%	202	8	
Employment Service (Industrial)	.00	۵	157,000	157,000	0.50	78,500	565	132			Contraction of the	Î	0.32	0.37	189	9658	15%	161	28	7 15	45 B 854	33	185	10
Sub total				167,000		78,600	696				378				418		1	183 2	32 44	1		237	207	
Manufacture & adds Accounts		ľ																						Г
werrington sourchampus Employment Creative Entreprises	13,21-24	4	393,000	000'E6E	0:50	196,500	*	6,200					1,00	1.00	38	9658	15%	R	8	15	458 %	9	R	S
LowMedium Density Residential	25-28	5	in the second			Ŕ	8	ŝ	58	\$	134	100000	09.0	0.60	8	10%	906	0	22	8	201 B	5	00	
TAFE	9								ÿ		S.	2,500	0.10	0.07	250	9508	20%	200	17	4	#09 %	2	105	
LowMedium Density Residential/Employment (1)	¢ ¢	5 8	07 NU	07 NN		40 CUU	uto	5	5.7	\$	146	i i i	0.60	8.0	6	10%	90%	σ. c	50 0	6	104 1050	ρ.c	σι c	. 3
COMINGUININ DEVICE REPORTING IN PROVIDENT (1)		0	nnn' is	nnn' is	nem	nns'et	0.0	8					20.02	20.0		91.00	e o	,			0 0 0	-	2	Т
Sub total				480,000		246,000	1,008		-		278	2,600			455			248 2	06 35	0		226	164	2
UWS Kingswood													ALC: NO.	1	-	-					-		2	
Student Accommodation/Precinct Centre	F										100		8	0.0		960	80%			8			• •	
Cpen space	2	15.55							3	200			3	0.00		5	5				8	-		
Medium Density Residential	10-10	5							5	ų.	m B		8.0	0.60	un (950	\$0%				5 (ω :	
UNO Built Joint				•							100	1007	0.10	0.0	107	0.00	1	012	0 0 0	; ;	600	2 207	940	÷
ond total			8	-		-			2		001	470'7			010	3	220	014	07 00		2	180 N	0	1
Caddone Release Low/Medium Density Residential	14,19,20	ö	Ì						8	\$	1,344		09:0	0.60	908	10%	904	81	26 80	6 90	96 10%	126	20	- 20
8 ub total				0		0	0				1,344	0		0	808			81 7	28 80	8		726	81	
Claremont Meadows	Î	3							S age		S Start		No.		0.00		() ()		3	2		20	2	1
stage 2 Residential		81							2		3		0.85	38.0	A 1	9601	205					31	4	
Stage 2 Residential		ő							82	00	14		0.85	0.85	3	950	808		8	8	# #	8	ø	
Stage 2 Residential		D3		3		3	2 2 2		45.5	60	364	2	0.85	0.85	309	10%	90%	31 2	78 30	6 6	101	278	h	
Sub total		10000		0	- 20	0	•		Sector Annual		478	0	Statute and	Contraction of the second	407	- A Second	Section of the sectio	41 3	87 40	T 84		387	41	
Presingt Centre (6)				2													-							
Mixed Use Residential	Ŧ								6.7		134		050	0.50	6	10%	80%	-	9	6	10 Hora	8	-	
Office	+					100	5	L,					0.020	0.020	#	9658	15%	ę	4		36 B54	•	9	
Retail (Specialty)	11					9,000							0.020	0.046	180	9658	15%	15	17 41	4	807	Ř	166	٦
Sub total				D		8,700	323				134				261			172 8	88 48	9		112	164	
Total			00	1,308,000		684,200	8,706				3,140	6,124	00		,412			1,862 2,	680 7,5	76	15	2.83	6 4,741	

Unounfirmed Land Use (Accumed to be 100% residential for parcel 12)
 FSR are accumed by Anothectus
 - Strate accumed by Anothectus
 - Constry (development accumed by 50% Employment Report accept for Creative Precind (4, - constry (development accumed by Anothectus
 - Enveloped Canter (B) is accumed to be located in parcel 11 of O'Connell 8t. (8) - Previote Canter (B) is accumed to be located in parcel 11 of O'Connell 8t. (8) - Freidord Canter (B) is accumed to be located in parcel 11 of O'Connell 8t.



Appendix D

Swept Paths







Version: 1, Version Date: 31/05/2021



Version: 1, Version Date: 31/05/2021



Version: 1, Version Date: 31/05/2021



Version: 1, Version Date: 31/05/2021





Version: 1, Version Date: 31/05/2021



Version: 1, Version Date: 31/05/2021





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