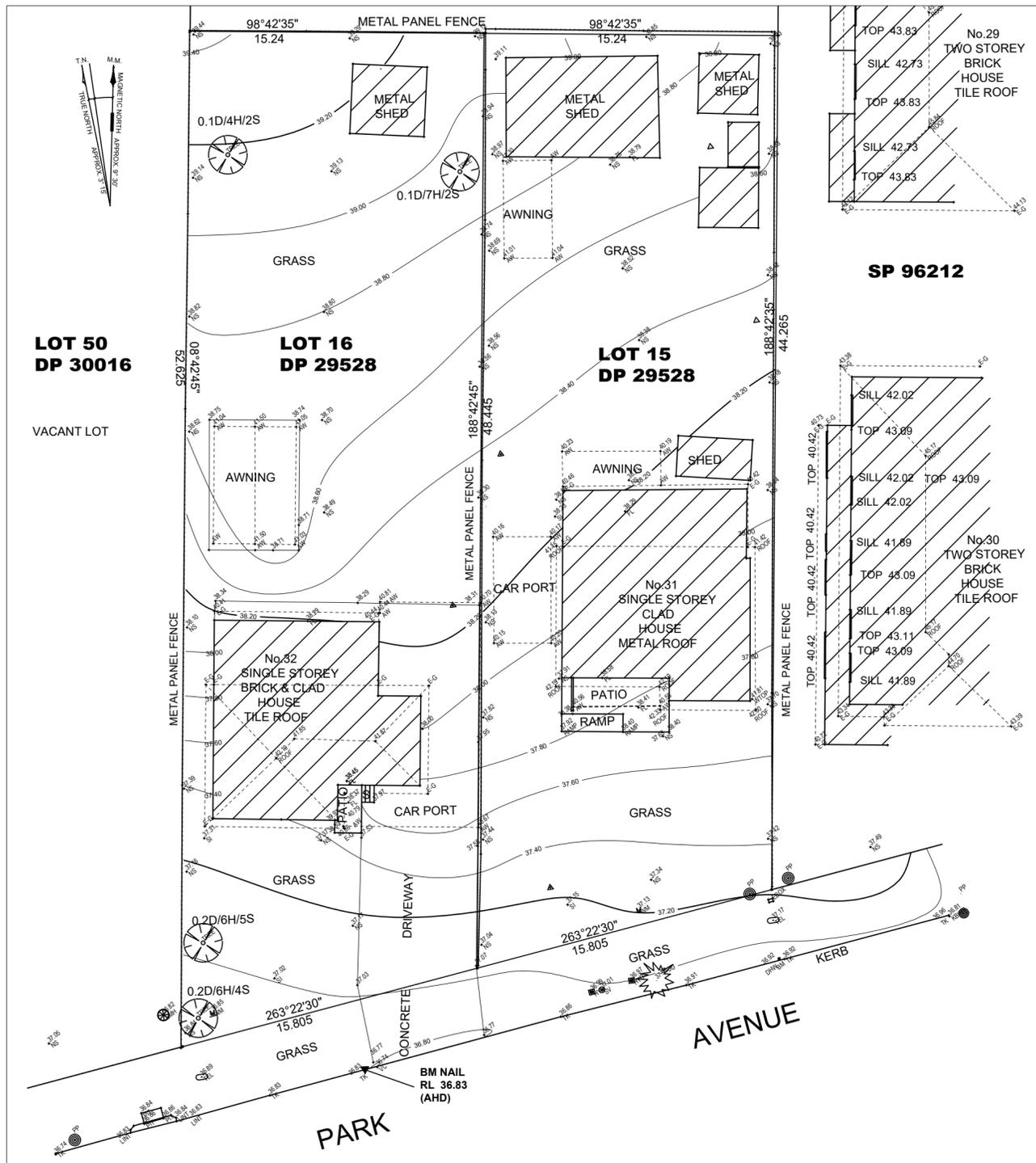


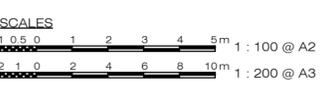
# STORMWATER MANAGEMENT PLAN

## SITE - LOTS 15-16 DP 29528 - 31-32 PARK AVENUE, KINGSWOOD



### SURVEY PLAN

SCALE 1:150 [SURVEY BY OTHERS]



- SURVEY & UTILITY SERVICES NOTES:**
1. SURVEY DATA PROVIDED BY OTHERS. MPC TAKES NO RESPONSIBILITY ON THE ACCURACY OF THE LEVELS & DETAILS SHOWN.
  2. ALL SERVICES AS SHOWN ON THIS PLAN ARE APPROXIMATE ONLY BY SURVEY. EXACT LOCATION SHALL BE ACCURATELY LOCATED BY CONTACTING DIAL BEFORE YOU DIG OR THE RELEVANT UTILITY PROVIDER BEFORE COMMENCING ANY EXCAVATION OR BUILDING WORKS.
  3. ALL WORKS UNDERTAKEN WITHIN THE ROAD RESERVE / CARRIAGEWAY SHALL HAVE PRIOR APPROVAL FROM THE RELEVANT ROAD AUTHORITY, PRIOR TO COMMENCING WORKS.
  4. ALL SURVEY LEVELS TO AUSTRALIAN HEIGHT DATUM - mAHD

STORMWATER DRAWING SCHEDULE		
SHEET No.	DRAWING NUMBER	TITLE
01	SW-2011A-DA-01	SURVEY, LOCALITY PLAN & DRAWING SCHEDULE
02	SW-2011A-DA-02	CONCEPT STORMWATER DRAINAGE PLAN
03	SW-2011A-DA-03	STORMWATER CATCHMENT PLAN
04	SW-2011A-DA-04	OSD DETAILS & CALCULATIONS
05	SW-2011A-DA-05	WATER SENSITIVE URBAN DESIGN DETAILS
06	SW-2011A-DA-06	BASEMENT LEVEL DRAINAGE PLAN
07	SW-2011A-DA-07	PUMPOUT DRAINAGE DETAILS & CALCULATIONS
08	SW-2011A-DA-08	EROSION AND SEDIMENT CONTROL PLAN

### LOCALITY PLAN

GOOGLE MAP - NOT TO SCALE

PO BOX 5468  
GREYSTANES NSW 2145  
M: 0419 242 726  
E: zulfik@bigpond.com  
www.multiproconsultant.com.au  
ABN: 57615352540

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ALL DESIGN PLANS AND REPORTS PREPARED BY MULTIPRO CONSULTANTS ARE NOT TO BE USED AND/OR REPRODUCED WHOLLY OR PART WITHOUT THE WRITTEN CONSENT OF MULTIPRO CONSULTANTS PTY LTD.

APPROVED BY:  
ZULFIQAR KHAN  
MIEAust, NER (# 2500471)  
Accredited Certifier  
(Civil & Structural) (BP82925)

SIGNED:

PROJECT: 31-32 PARK AVENUE, KINGSWOOD  
TITLE: SURVEY, LOCALITY PLAN & DRAWING SCHEDULE

DESIGN BY: MPC  
CHECKED BY: Z KHAN

SCALE: 1:100 or As Shown  
SHEET: 1 OF 8

FILE: JOB 21-165  
DATE: 20 AUGUST 2021

APPLICANT/CLIENT:  
ARCHIDROME

ARCHITECT:  
ARCHIDROME

NOT FOR CONSTRUCTION

20/08/21 A FOR COUNCIL DA APPROVAL - PROPOSED CHILDCARE CENTRE

DATE ISSUE AMENDMENTS

www.dialbeforeyoudig.com.au  
**DIAL 1100**  
BEFORE YOU DIG

**PENRITH CITY COUNCIL**

DRAWING NUMBER:  
SW-2011A-DA-01

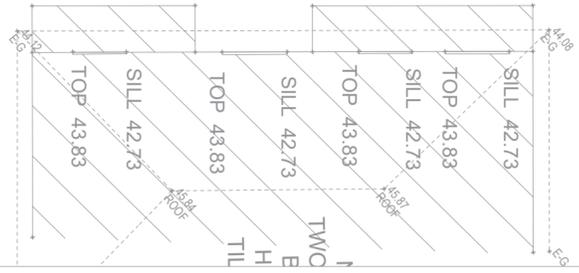
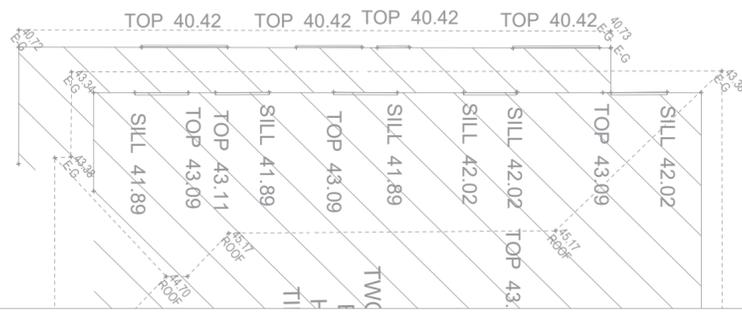
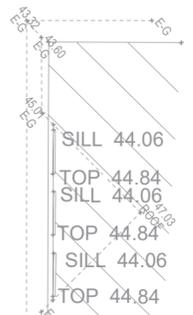
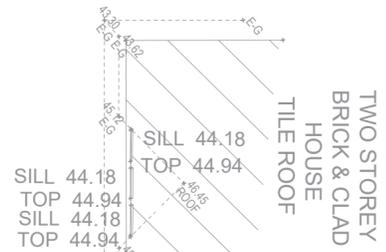
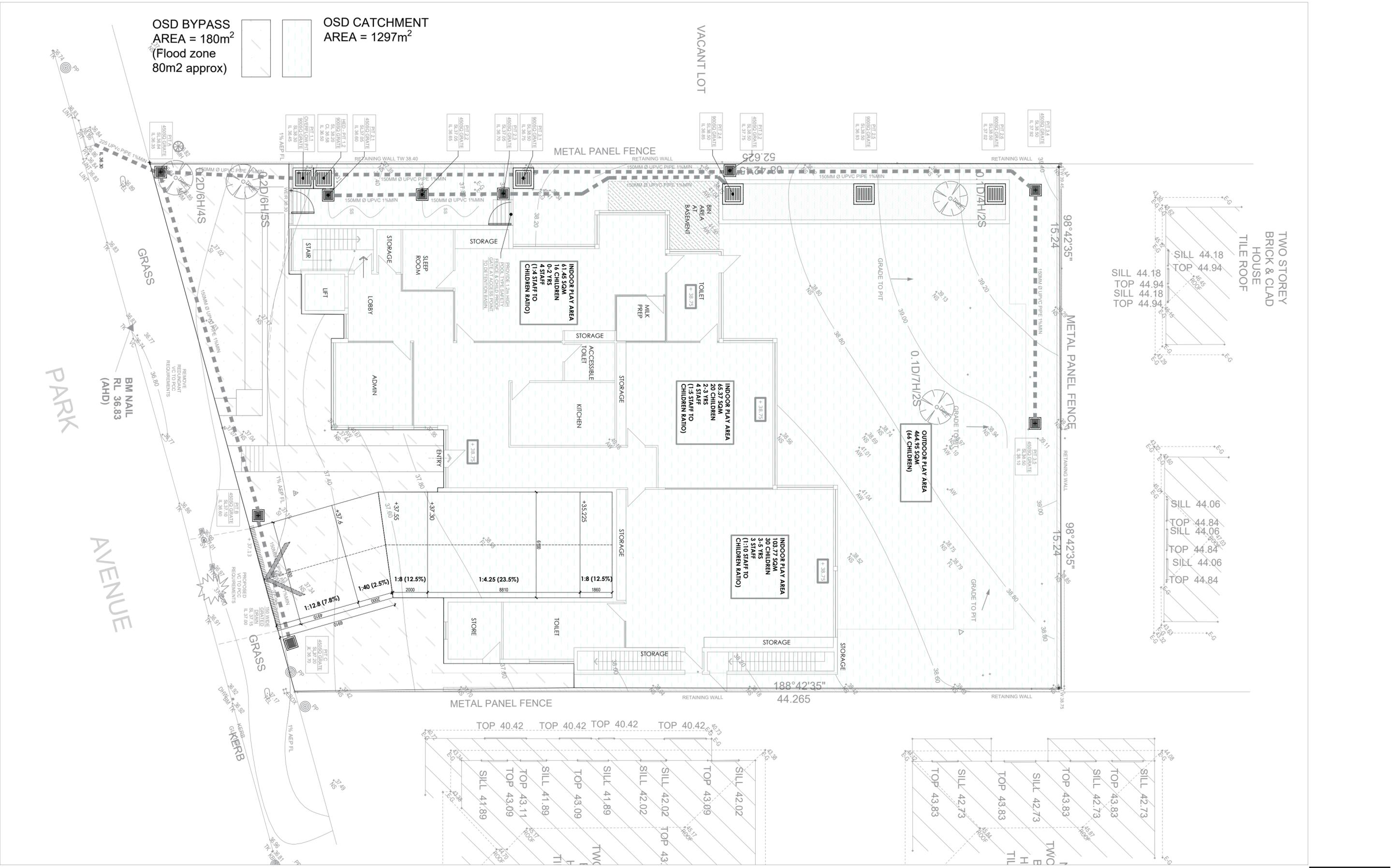
SHEET SIZE:  
A2

DESIGNER:  
MPC01



OSD BYPASS AREA = 180m<sup>2</sup>  
(Flood zone 80m<sup>2</sup> approx)

OSD CATCHMENT AREA = 1297m<sup>2</sup>



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APPROVED BY:  
ZULFIQAR KHAN  
MIEAust, NER (# 2500471)  
Accredited Certifier  
(Civil & Structural) (BPB2925)

PROJECT: 30-32 PARK AVENUE, KINGSWOOD  
TITLE: STORMWATER CATCHMENT PLAN

DESIGN BY: MPC	SCALE: 1:100 or As Shown	FILE: JOB 21-165
CHECKED BY: Z KHAN	SHEET: 3 OF 8	DATE: 20 AUGUST 2021

APPLICANT/CLIENT:  
ARCHIDROME

ARCHITECT:  
ARCHIDROME

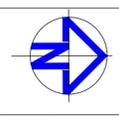
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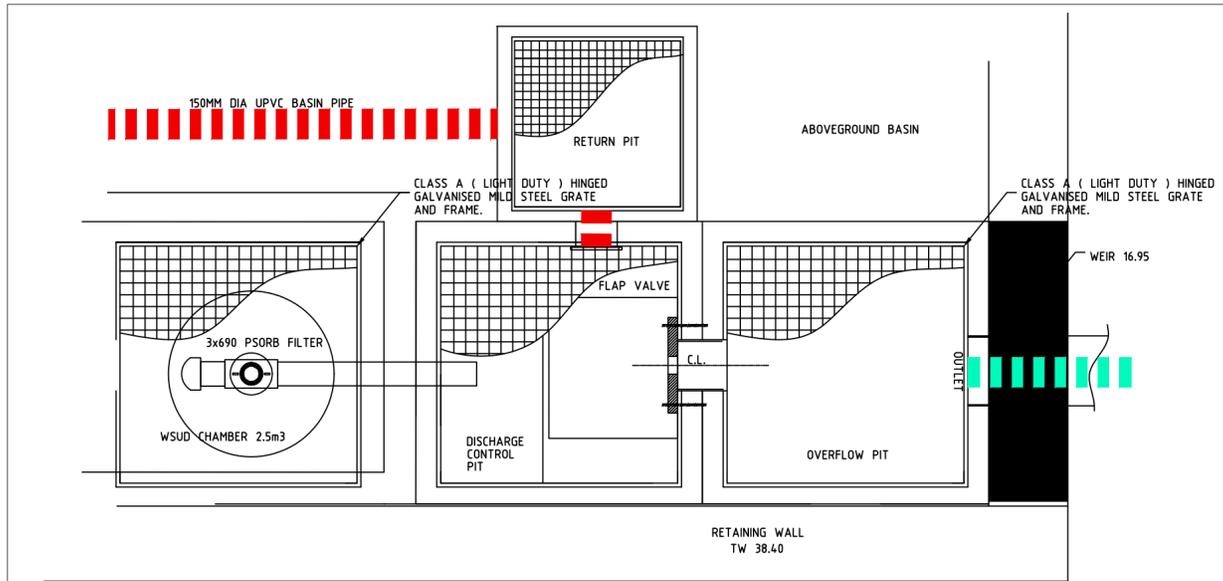
FOR COUNCIL DA APPROVAL - PROPOSED CHILDCARE CENTRE

DATE	ISSUE	AMENDMENTS	DESIGNER
20/08/21	A		MPC01

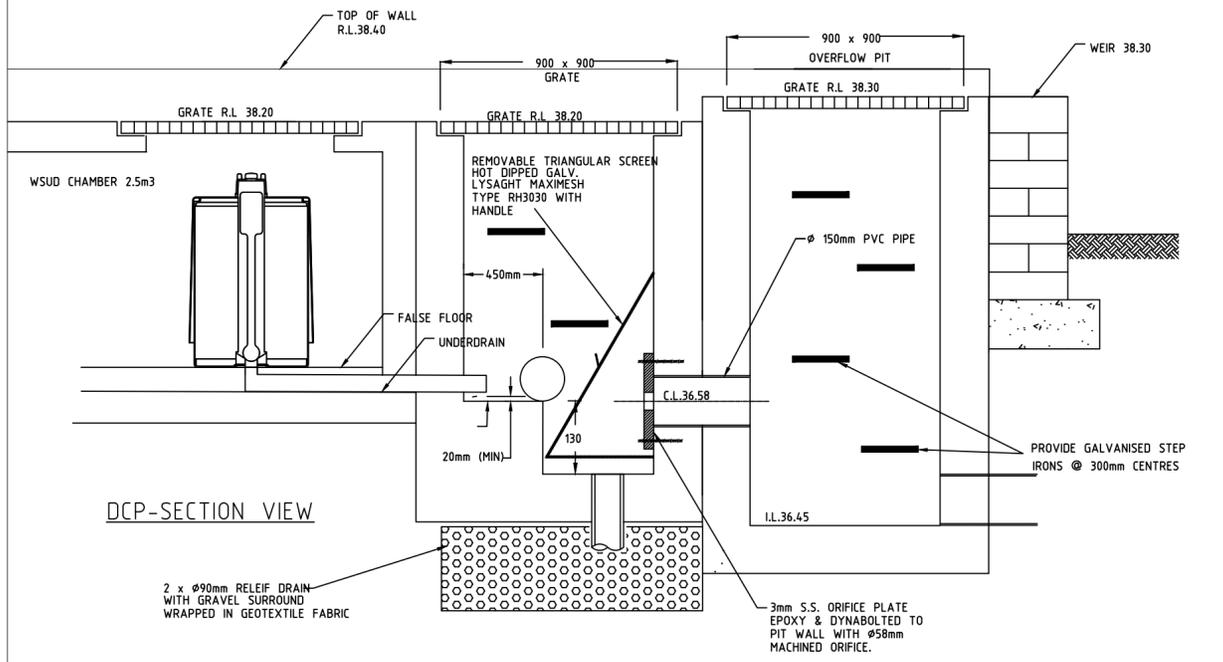
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SW-2011A-DA-03

SHEET SIZE:  
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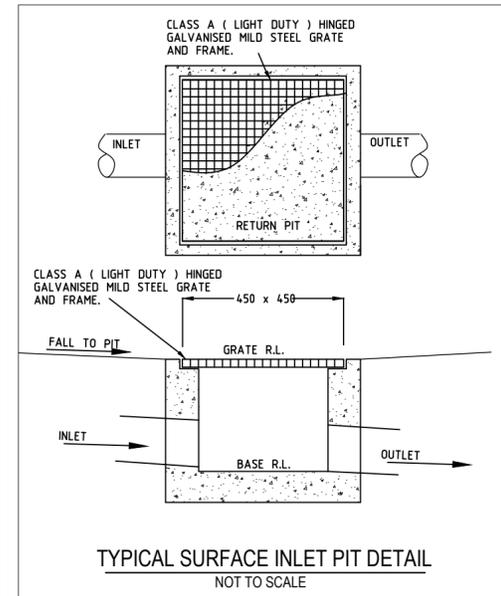
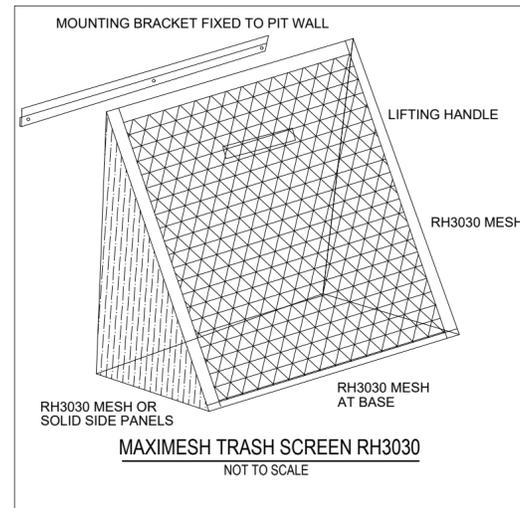
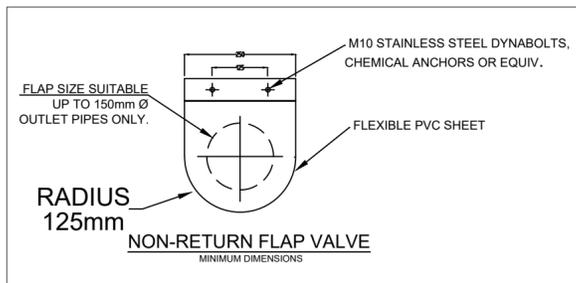
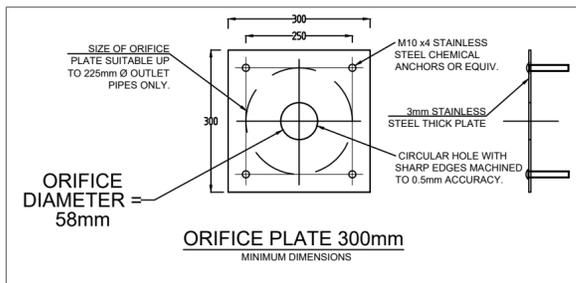




PLAN VIEW (Typical)



DCP-SECTION VIEW



### On-Site Detention Calculation Sheet

Project:	Proposed Childcare Centre		Lot No. 15-16	
Location:	31-32 Park Avenue, Kingswood		DP No. 29528	
Designer:	MultiPro Consultants		OSD No 1	
OSD Area:				
Site Area	0.138		0.138	Drowned
Basic Storage Volume	64.80		64.80	
Basic Discharge	6.45		11.03	
Area of Site to Storage	0.130	94%	0.130	94%
Percentage of Site	94.12		94.12	
Storage per ha of contributing area	499.34		499.34	
Volume/PSD Adjustment	73.75		73.75	
PSD for site	9.56		9.56	
Maximum Head to Orifice Centre	1.740		1.100	
Calculated Orifice Diameter	0.058		0.058	
Maximum discharge	9.563		7.608	
Head for high early discharge	1.640		1.000	
High Early Discharge	9.284	97%	7.254	76%
Mean Discharge	9.424		7.431	
Average Discharge per Hectare	72.620		57.266	
Final Site Storage Ratio	508		604	
Site Storage Volume	65.89		78.39	
Volume Provided	97.91	149%	97.910	125%

**GENERAL NOTES**

- ALL WORKS SHALL GENERALLY BE UNDERTAKEN IN ACCORDANCE WITH AS/NZ 3500.3.2:1998 AND COUNCIL'S SPECIFICATIONS &/OR ENGINEERING GUIDELINES;
- THIS STORMWATER PLAN SHALL BE READ IN CONJUNCTION WITH THE APPROVED ARCHITECTURAL, LANDSCAPE, SURVEY, AND SITE PLANS;
- ANY DISCREPANCIES OR OMISSIONS SHALL BE REFERRED TO MULTIPRO CONSULTANTS TO RESOLVE.
- ALL DRAINAGE LINES SHALL BE AS FOLLOWS -
  - 90mm DIAMETER WHERE THE LINE ONLY RECEIVES ROOF WATER;
  - 100mm DIAMETER WHERE THE LINE RECEIVES SURFACE RUNOFF OR IF THE LINE IS PART OF AN OSD SYSTEM;
  - A MINIMUM PIPELINE GRADE OF 1% FOR PIPES WITH A DIAMETER LESS THAN 150mm AND 0.5% FOR PIPES OF GREATER DIAMETER;
- ALL DRAINAGE LINES SHALL HAVE A MINIMUM COVER OF 100mm FOR PRIVATE PIPELINES AND 300mm FOR PUBLIC PIPELINES;
- ALL PITS WITHIN TRAFFICABLE AREAS (I.E DRIVEWAYS) SHALL BE HEAVY DUTY. ALL SURFACE AREAS SHALL BE GRADED TO THE SURFACE INLET PITS/DRAINS.
- LOCATION OF DOWN PIPES & FLOOR WASTES ARE INDICATIVE ONLY. DOWNPIPE & FLOOR WASTE SIZE, LOCATION & QUANTITY IS TO BE DETERMINED AT CC STAGE AND PRIOR TO CONSTRUCTION IN ACCORDANCE WITH THE RELEVANT STANDARDS.

**UTILITY SERVICES**

- ALL SERVICES AS SHOWN ON THIS PLAN ARE APPROXIMATE ONLY. EXACT LOCATION SHALL BE ACCURATELY LOCATED BY CONTACTING DIAL BEFORE YOU DIG OR THE RELEVANT UTILITY PROVIDER BEFORE COMMENCING ANY EXCAVATION OR BUILDING WORKS.
- ALL WORKS UNDERTAKEN WITHIN THE ROAD RESERVE / CARRIAGEWAY SHALL HAVE PRIOR APPROVAL FROM THE RELEVANT AUTHORITY, PRIOR TO COMMENCING WORKS.

**ONSITE DETENTION NOTES**

- ALL WALLS FORMING PART OF THE DETENTION BASIN SHALL BE OF MASONRY CONSTRUCTION AN WHOLLY WITHIN THE PROPERTY BOUNDARY;
- ALL MULCH WITHIN THE ABOVE GROUND DETENTION BASIN (IF PROPOSED) SHALL BE NON-FLOATABLE;
- ALL GRATES TO BE FITTED WITH CHILD PROOF J-LOCKS;
- THE CERTIFYING ENGINEER OR COUNCIL'S ENGINEER SHALL INSPECT THE OSD WORKS AT THE CRITICAL STAGES -
  - PRIOR TO COMMENCING WORK TO DISCUSS SITE CONSTRAINTS;
  - PRIOR TO LANDSCAPING THE DETENTION BASIN & POURING THE ROOF OF THE DETENTION TANK;
  - PRIOR TO INSTALLING THE FITTINGS TO THE DISCHARGE CONTROL UNIT;
  - PRELIMINARY INSPECTION - PRIOR TO CERTIFICATION
  - FINAL INSPECTION - FOR CERTIFICATION. WORK-AS-EXECUTED [WAE] SHALL BE PREPARED BY A REGISTERED SURVEYOR PRIOR TO FINAL INSPECTION.
- ALL MAINTENANCE WORK SHALL BE UNDERTAKEN IN ACCORDANCE WITH THE MAINTENANCE SCHEDULE APPROVED BY COUNCIL;
- DRAINAGE PIPE LINES ARE TO AVOID EXISTING TREES WHEREVER POSSIBLE - GENERALLY OUTSIDE THE DRIP LINE / CANOPY OF THE EXISTING TREE;



**INSTALL SIGN NEAR OSD TANK**



**INSTALL WITHIN OSD BASIN**



**OSD PLATE**



MUSIC-link Report

Project Details		Company Details	
Project:	32 Park Avenue, Kingswood	Company:	Mull Pro Consultants
Report Export Date:	9/10/2021	Contact:	Zulf Khan
Catchment Name:	16762 - 32 Park Avenue	Address:	PO Box 5468 Greystanes NSW 2145
Catchment Area:	0.148ha	Phone:	0419 242 726
Impervious Area:	55.14%	Email:	zulk@bigpond.com
Rainfall Station:	67113 PDNS/TH		
Modeling Time-step:	5 Minutes		
Modeling Period:	10/1/1999 - 31/12/2006 11:54:00 PM		
Mean Annual Rainfall:	691mm		
Evapotranspiration:	1158mm		
MUSIC Version:	6.3.0		
MUSIC-link data Version:	6.33		
Study Area:	Penrith		
Scenario:	Penrith Development		

\* Takes into account area from all source nodes that link to the chosen reporting node, excluding Import Data Nodes

Treatment Train Effectiveness		Treatment Nodes		Source Nodes	
Node	Reduction	Node Type	Number	Node Type	Number
Flow	0.0269%	Sedimentation Basin Node	1	Urban Source Node	7
TSS	85.7%	Generic Node	1		
TP	66.9%	GPT Node	2		
TN	47.3%				
GP	100%				

Comments  
- The 'SF Chamber' node has been modified to represent the below ground Strainer chamber. Default 'K' values have been manually adjusted to 1 in order to eliminate any performance from the actual link, which would already be accounted for in the Filter Generic Node Target Elements. Not doing this would represent a duplication of the chamber attenuation effect. For any questions, please Contact Ocean Protect on 1300 354 722.

Passing Parameters

Node Type	Node Name	Parameter	Min	Max	Actual
GPT	2 x OceanGuard200um	H-Flow bypass rate (survived)	None	99	0.04
GPT	3 x OceanGuard200um	H-Flow bypass rate (survived)	None	99	0.06
Recycling	Recycling Node	% Load Reduction	None	None	0.00269
Recycling	Recycling Node	GP % Load Reduction	90	None	100
Recycling	Recycling Node	TN % Load Reduction	45	None	47.3
Recycling	Recycling Node	TP % Load Reduction	60	None	66.9
Recycling	Recycling Node	TSS % Load Reduction	85	None	85.7
Sedimentation	SF Chamber 2.5m	High Flow Bypass Out (ML/y)	None	None	0
Urban	Driveway - 30m <sup>2</sup> (100% Imp.)	Area Impervious (ha)	None	None	0.003
Urban	Driveway - 30m <sup>2</sup> (100% Imp.)	Area Permeable (ha)	None	None	0
Urban	Driveway - 30m <sup>2</sup> (100% Imp.)	Total Area (ha)	None	None	0.003
Urban	Driveway - 30m <sup>2</sup> (100% Imp.)	Area Impervious (ha)	None	None	0.004
Urban	Driveway - 30m <sup>2</sup> (100% Imp.)	Area Permeable (ha)	None	None	0
Urban	Driveway - 30m <sup>2</sup> (100% Imp.)	Total Area (ha)	None	None	0.004
Urban	Landscape - 189m <sup>2</sup> (100% Perv.)	Area Impervious (ha)	None	None	0
Urban	Landscape - 189m <sup>2</sup> (100% Perv.)	Area Permeable (ha)	None	None	0.019
Urban	Landscape - 189m <sup>2</sup> (100% Perv.)	Total Area (ha)	None	None	0.019
Urban	Landscape - 99m <sup>2</sup> (100% Perv.)	Area Impervious (ha)	None	None	0
Urban	Landscape - 99m <sup>2</sup> (100% Perv.)	Area Permeable (ha)	None	None	0.01
Urban	Landscape - 99m <sup>2</sup> (100% Perv.)	Total Area (ha)	None	None	0.01
Urban	Paved - 49m <sup>2</sup> (100% Imp.)	Area Impervious (ha)	None	None	0.005
Urban	Paved - 49m <sup>2</sup> (100% Imp.)	Area Permeable (ha)	None	None	0
Urban	Paved - 49m <sup>2</sup> (100% Imp.)	Total Area (ha)	None	None	0.005
Urban	Playground - 465m <sup>2</sup> (20% Imp.)	Area Impervious (ha)	None	None	0.009
Urban	Playground - 465m <sup>2</sup> (20% Imp.)	Area Permeable (ha)	None	None	0.037
Urban	Playground - 465m <sup>2</sup> (20% Imp.)	Total Area (ha)	None	None	0.047
Urban	Roof - 600m <sup>2</sup>	Area Impervious (ha)	None	None	0.06
Urban	Roof - 600m <sup>2</sup>	Area Permeable (ha)	None	None	0
Urban	Roof - 600m <sup>2</sup>	Total Area (ha)	None	None	0.06

Only certain parameters are reported when they pass validation

Failing Parameters

Node Type	Node Name	Parameter	Min	Max	Actual
Sedimentation	SF Chamber 2.5m	National Detention Time (hrs)	8	12	0.124
Sedimentation	SF Chamber 2.5m	Total Nitrogen - k (mg/y)	500	500	1
Sedimentation	SF Chamber 2.5m	Total Phosphorus - k (mg/y)	6000	6000	1
Sedimentation	SF Chamber 2.5m	Total Suspended Solids - k (mg/y)	8000	8000	1

Only certain parameters are reported when they pass validation



STORMFILTER DESIGN TABLE

- STORMFILTER TREATMENT CAPACITY VARIES BY NUMBER OF FILTER CARTRIDGES INSTALLED.
- THE STANDARD CONFIGURATION IS SHOWN. ACTUAL CONFIGURATION OF THE SPECIFIED STRUCTURE(S) PER CERTIFYING ENGINEER WILL BE SHOWN ON SUBMITTAL DRAWING(S).
- FILTER CARTRIDGES SHALL BE MEDIA-FILLED, PASSIVE, SIPHON ACTUATED, RADIAL FLOW, AND SELF-CLEANING. RADIAL MEDIA DEPTH SHALL BE 178mm.

CARTRIDGE NAME / SIPHON HEIGHT (mm)	690	460	310
CARTRIDGE PHYSICAL HEIGHT (mm)	840	600	600
TYPICAL WEIR HEIGHT [H] (mm)	920	690	540
CARTRIDGE FLOW RATE FOR ZPG MEDIA (L/s)	1.6	1.1	0.7
CARTRIDGE FLOW RATE FOR PSORB MEDIA (L/s)	0.9	0.46	0.39

PHYSICAL HEIGHT ↑  
SIPHON HEIGHT ↑

STORMFILTER CARTRIDGE DETAIL

**SITE SPECIFIC DATA REQUIREMENTS**

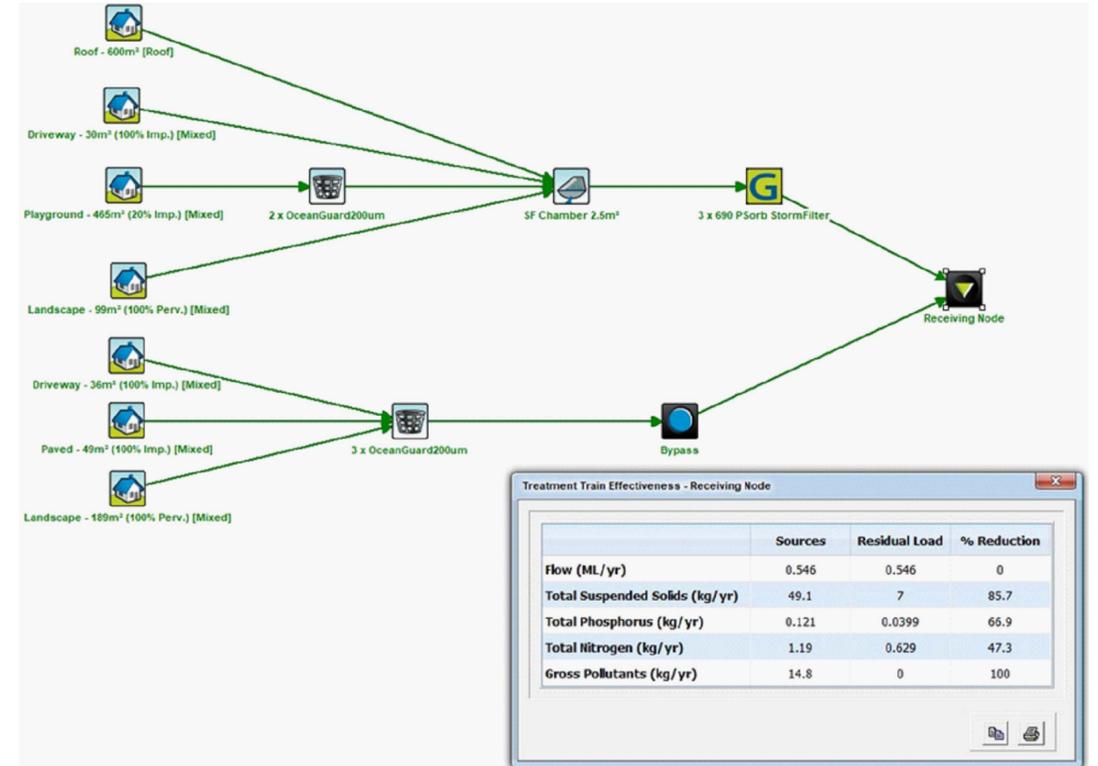
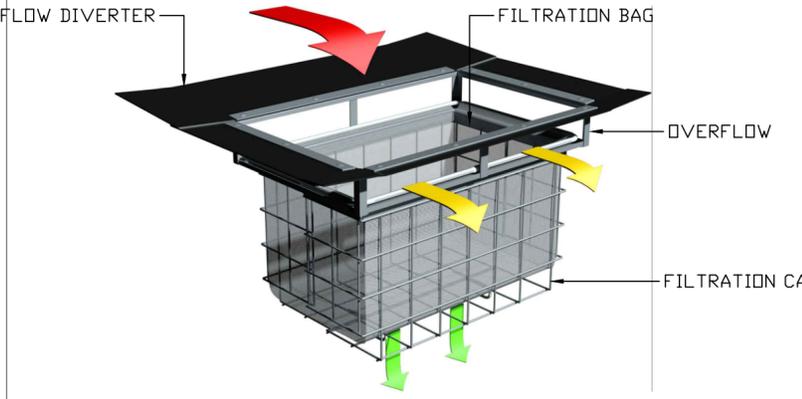
STRUCTURE ID	STRUCT_ID
No. CARTRIDGES REQ'D	C_NUM
SIPHON HEIGHT	C_HT
MEDIA TYPE (ZPG/PSORB)	MEDIA
WQ FLOW RATE (L/S)	WQFR
DIMENSION A	LOA
DIMENSION B	LOB

TOTAL CARTRIDGE BAY AREA (A x B)  
TO MATCH AREA REQUIRED BY MUSIC  
MODELLING OR COUNCIL SPECIFIC  
REQUIREMENTS

PLAN ID	MAXIMUM PIT PLAN DIMENSIONS
S	450mm x 450mm
M	600mm x 600mm
L	900mm x 900mm
XL	1200mm x 1200mm

DEPTH ID	BAG DEPTH	OVERALL DEPTH
1	170	270
2	300	450
3	600	700

PLAN ID	DEPTH ID		
	1	2	3
S	■	■	■
M	■	■	■
L	■	■	■
XL	■	■	■



GENERAL NOTES -STORMFILTER

- INLET AND OUTLET PIPES TO BE IN ACCORDANCE WITH APPROVED PLANS.
- A HIGH FLOW BYPASS ARRANGEMENT OR DISSIPATION STRUCTURE MAY BE REQUIRED TO MINIMISE RE-SUSPENSION OF SOLIDS OR ANY SIGNIFICANT INERTIAL FORCES ON THE CARTRIDGES.
- ALL WATER QUALITY TREATMENT DEVICES REQUIRE PERIODIC MAINTENANCE. REFER TO OPERATION AND MAINTENANCE MANUAL FOR GUIDELINES AND ACCESS REQUIREMENTS.
- SITE SPECIFIC PRODUCTION DRAWING WILL BE PROVIDED ON PLACEMENT OF ORDER.
- THE INVERT LEVEL OF THE INLET PIPE MUST BE GREATER THAN THE RL OF THE FALSE FLOOR WITHIN THE CARTRIDGE CHAMBER.
- CONCRETE STRUCTURE AND ACCESS COVERS DESIGNED AND PROVIDED BY OTHERS. ACCESS COVERS TO BE A MINIMUM 900 X 900 ABOVE CARTRIDGES. OH&S REGARDING ACCESS COVERS AND TANK ACCESS TO BE ASSESSED BY OTHERS ON SITE.
- THE STRUCTURE THICKNESSES SHOWN ARE FOR REPRESENTATIONAL PURPOSES.
- DRAWINGS NOT TO SCALE.

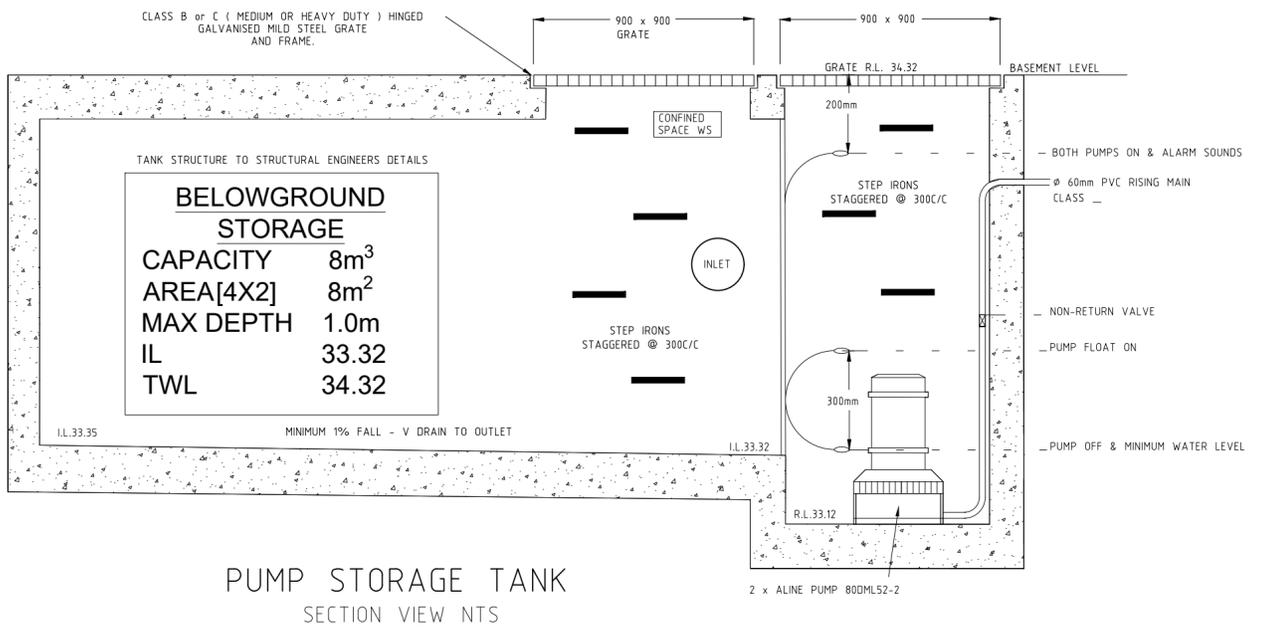
INSTALLATION NOTES

- UNDERDRAIN AND FALSE FLOOR INSTALLED BY OCEAN PROTECT.

GENERAL NOTES - 200MICRON OG

- THE MINIMUM CLEARANCE DEPENDS ON THE CONFIGURATION (SEE NOTE 2) AND THE LOCAL COUNCIL REQUIREMENTS.
- CLEARANCE FOR ANY PIT WITHOUT AN INLET PIPE (ONLY USED FOR SURFACE FLOW) CAN BE AS LOW AS 50mm. FOR OTHER PITS, THE RECOMMENDED CLEARANCE SHOULD BE GREATER OR EQUAL TO THE PIPE OVERTOP SO AS NOT TO INHIBIT HYDRAULIC CAPACITY.
- OCEAN PROTECT PROVIDES TWO FILTRATION BAG TYPES:- 200 MICRON BAGS FOR HIGHER WATER QUALITY FILTERING AND A COARSE BAG FOR TARGETING GROSS POLLUTANTS.
- DRAWINGS NOT TO SCALE.





**Single Channel Impeller - 76mm passage**

**Model DML Submersible Sewage Pumps**

**Specifications**

- Submersible pump with non dog single channel impeller
- Maximum solids handling: 76 mm
- Maximum liquid temperature: 40 °C
- Maximum submergence: 8 m

**Materials**

- Pump casing: Cast Iron
- Impeller: Cast Iron
- Wear ring: Bronze
- Shaft: 403 Stainless Steel
- Motor frame: Cast Iron
- Fasteners: 304 Stainless Steel
- Mechanical seal: Double mechanical seal in of chamber, Silicon Carbide lower faces, Carbon/Ceramic upper faces.

**Motor Data**

- Air filled, dry submersible, 4 pole, 50 Hz
- 1.6 Star-Delta (DOL for 2.2 kW)
- Insulation class F
- IP68 protection
- 3 Phase - 400 Volt

**Range**

- 80 to 150 mm Ø discharge
- 2.2 to 22 kW - 3 Phase (Manual only)

**Supply**

- Cable(s): 10 m
- Discharge elbow with companion flange (Ø 4 x 100 mm standard, 150 mm with opt)

**Accessories**

- Quick Discharge Connectors (QDC) available

**Impeller Type & Solids Handling**

**Enclosed Single Channel**

Model	Ø	80	100	100
Free Impeller Passage (mm)		76	76	76

- GENERAL NOTES**
- ALL WORKS SHALL GENERALLY BE UNDERTAKEN IN ACCORDANCE WITH AS/NZ 3500.3.2:1998 AND COUNCIL'S SPECIFICATIONS &/OR ENGINEERING GUIDELINES;
  - THIS STORMWATER PLAN SHALL BE READ IN CONJUNCTION WITH THE APPROVED ARCHITECTURAL, LANDSCAPE, SURVEY, AND SITE PLANS;
  - ANY DISCREPANCIES OR OMISSIONS SHALL BE REFERRED TO MULTIPRO CONSULTANTS TO RESOLVE.
  - ALL DRAINAGE LINES SHALL BE AS FOLLOWS -
    - 90mm DIAMETER WHERE THE LINE ONLY RECEIVES ROOF WATER;
    - 100mm DIAMETER WHERE THE LINE RECEIVES SURFACE RUNOFF OR IF THE LINE IS PART OF AN OSD SYSTEM;
    - A MINIMUM PIPELINE GRADE OF 1% FOR PIPES WITH A DIAMETER LESS THAN 150mm AND 0.5% FOR PIPES OF GREATER DIAMETER;
  - ALL DRAINAGE LINES SHALL HAVE A MINIMUM COVER OF 100mm FOR PRIVATE PIPELINES AND 300mm FOR PUBLIC PIPELINES;
  - ALL PITS WITHIN TRAFFICABLE AREAS (I.E DRIVEWAYS) SHALL BE HEAVY DUTY. ALL SURFACE AREAS SHALL BE GRADED TO THE SURFACE INLET PITS/DRAINS.
  - LOCATION OF DOWN PIPES & FLOOR WASTES ARE INDICATIVE ONLY. DOWNPIPE & FLOOR WASTE SIZE, LOCATION & QUANTITY IS TO BE DETERMINED AT CC STAGE AND PRIOR TO CONSTRUCTION IN ACCORDANCE WITH THE RELEVANT STANDARDS.

- UTILITY SERVICES**
- ALL SERVICES AS SHOWN ON THIS PLAN ARE APPROXIMATE ONLY. EXACT LOCATION SHALL BE ACCURATELY LOCATED BY CONTACTING DIAL BEFORE YOU DIG OR THE RELEVANT UTILITY PROVIDER BEFORE COMMENCING ANY EXCAVATION OR BUILDING WORKS.
  - ALL WORKS UNDERTAKEN WITHIN THE ROAD RESERVE / CARRIAGEWAY SHALL HAVE PRIOR APPROVAL FROM THE RELEVANT AUTHORITY, PRIOR TO COMMENCING WORKS.

**BASEMENT PUMPOUT SPECIFICATIONS & NOTES**

THE PUMP OUT SYSTEM SHALL BE DESIGNED TO BE OPERATED IN THE FOLLOWING MANNER: -

- THE PUMPS SHALL BE PROGRAMMED TO WORK ALTERNATELY TO ALLOW BOTH PUMPS TO HAVE AN EQUAL OPERATION LOAD AND PUMP LIFE.
- A FLOAT SHALL BE PROVIDED TO ENSURE THAT THE MINIMUM REQUIRED WATER LEVEL IS MAINTAINED WITHIN THE SUMP AREA OF THE BELOW GROUND TANK. IN THIS REGARD THIS FLOAT WILL FUNCTION AS AN OFF SWITCH FOR THE PUMPS AT THE MINIMUM WATER LEVEL. THE SAME FLOAT SHALL BE SET TO TURN ONE OF THE PUMPS ON UPON THE WATER LEVEL IN THE TANK RISING TO APPROXIMATELY 300MM ABOVE THE MINIMUM WATER LEVEL. THE PUMP SHALL OPERATE UNTIL THE TANK IS DRAINED TO THE MINIMUM WATER LEVEL.
- A SECOND FLOAT SHALL BE PROVIDED AT A HIGH LEVEL, WHICH IS APPROXIMATELY THE ROOF LEVEL OF THE BELOW GROUND TANK. THIS FLOAT SHALL START THE OTHER PUMP THAT IS NOT OPERATING AND ACTIVATE THE ALARM.
- AN ALARM SYSTEM SHALL BE PROVIDED WITH A FLASHING STROBE LIGHT AND A PUMP FAILURE WARNING SIGN WHICH ARE TO BE LOCATED AT THE DRIVEWAY ENTRANCE TO THE BASEMENT LEVEL. THE ALARM SYSTEM SHALL BE PROVIDED WITH A BATTERY BACK-UP IN CASE OF POWER FAILURE.
- A CONFINED SPACE DANGER SIGN SHALL BE PROVIDED AT ALL ACCESS POINTS TO THE PUMP OUT STORAGE TANK..

**PUMPOUT SYSTEM CALCULATIONS - 31-32 PARK AVENUE, KINGSWOOD**

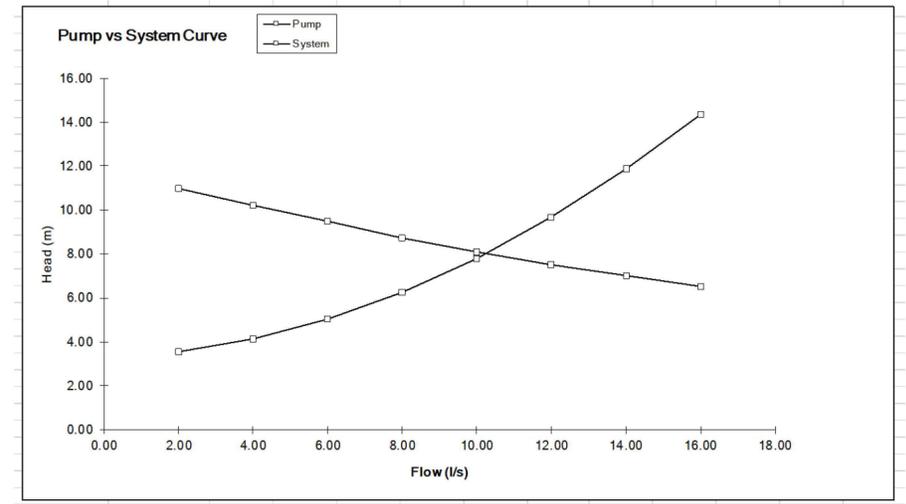
HL=	(3.35x10e6 x Q/d <sup>2.63</sup> x C) <sup>1.852</sup>	Hazen-Williams C	150
	HL (m/100m), Q(L/s), d(mm)	Pipe Diameter	60 mm
		Pipe Length	12 m
h1=	kv <sup>2</sup> /2g	Elevation Head	3.35 m
	k(cum), v(m/s), g(gravity(m/s))		
		Bend Losses, Kb	1.53 90 bend=0.51
		Valve Losses, Kv	0.14
H=	Hf+h1+Elevation Head (static head)	Entry/Exit Losses, Ke	2.00
(Total Head)		Cum Losses, K	3.67
		Start Flow 2	
		Increment 2	
Q (L/s)		2	4
HL (m/100m)		0.89	3.22
Hf (m)	HL x pipe length/	0.11	0.39
v (m/s)	Q(m) x area of pi	0.71	1.41
h1 (m)	k(cum) x Q(m) <sup>2</sup>	0.09	0.37
H (m)		3.55	4.11

Note: Hazen-Williams constant 125-140 Commercial Steel Pipe  
135-140 Bitumen lined cast iron pipe  
140-145 Copper Tube  
145-150 PVC

**Rising main =60mm Diameter**

**Selected Pump = 2 x Aline Submersible Pumps - 80DML 52-2 (2.2Kw 415 Volt)**

Pump	Q(l/s)	2.00	4.00	6.00	8.00	10.00	12.00	14.00	16.00	18	20	22
	H(m)	11.00	10.20	9.50	8.75	8.10	7.50	7.00	6.50	6.00	5.50	5.00
System	Q(l/s)	2.00	4.00	6.00	8.00	10.00	12.00	14.00	16.00	18.00	20.00	22.00
	H(m)	3.55	4.11	5.01	6.24	7.80	9.68	11.87	14.38	17.20	20.33	23.77



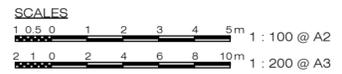
**MPC - PUMP STORAGE REQUIREMENT**

Property : 31-32 Park Avenue, Kingswood

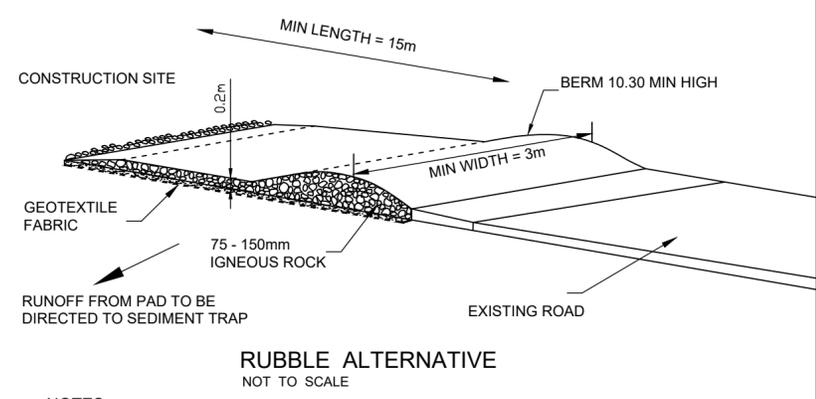
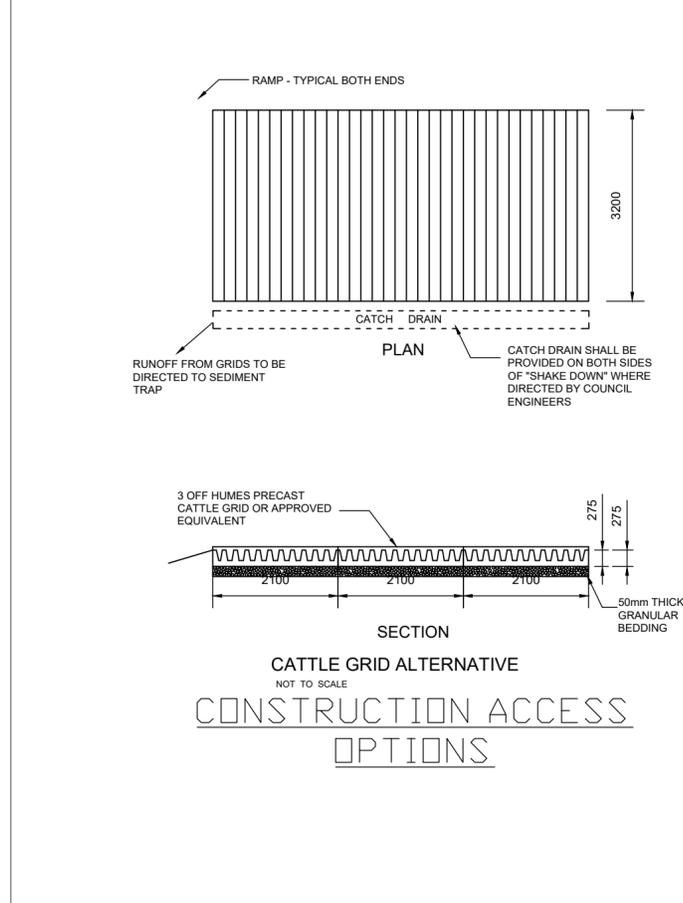
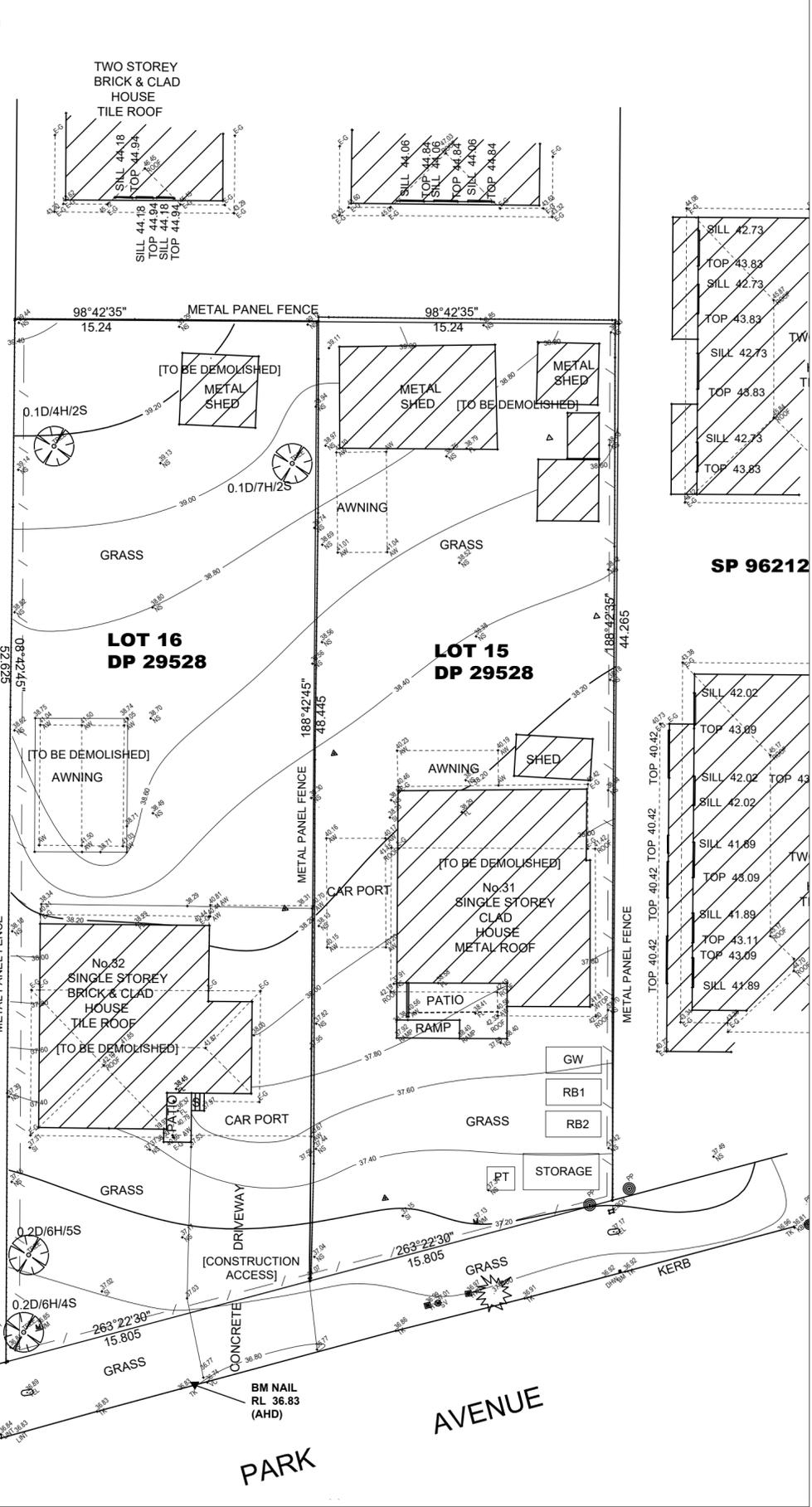
Area = 89.6 sq.m

Duration	1 Yr	Q1	V1	2 Yr	Q2	V2	5 Yr	Q5	V5	10 Yr	Q10	V10	20 Yr	Q20	V20	50 Yr	Q50	V50	100 Yr	Q100	V100
6 min	78	1.9	0.7	100	2.5	0.9	127	3.2	1.1	142	3.5	1.3	163	4.0	1.5	189	4.7	1.7	209	5.2	1.9
7 min	74	1.8	0.8	94	2.4	1.0	120	3.0	1.3	134	3.3	1.4	154	3.8	1.6	179	4.4	1.9	197	4.9	2.1
8 min	70	1.7	0.8	90	2.2	1.1	114	2.8	1.4	127	3.2	1.5	146	3.6	1.7	170	4.2	2.0	188	4.7	2.2
9 min	67	1.7	0.9	86	2.1	1.1	108	2.7	1.5	122	3.0	1.6	139	3.5	1.9	162	4.0	2.2	179	4.5	2.4
10 min	64	1.6	1.0	82	2.0	1.2	104	2.6	1.6	116	2.9	1.7	133	3.3	2.0	155	3.9	2.3	171	4.3	2.6
11 min	61	1.5	1.0	79	2.0	1.3	100	2.5	1.6	112	2.8	1.8	128	3.2	2.1	149	3.7	2.4	164	4.1	2.7
12 min	59	1.5	1.1	76	1.9	1.4	96	2.4	1.7	108	2.7	1.9	123	3.1	2.2	143	3.6	2.6	158	3.9	2.8
13 min	57	1.4	1.1	73	1.8	1.4	93	2.3	1.8	104	2.6	2.0	119	3.0	2.3	138	3.4	2.7	153	3.8	3.0
14 min	55	1.4	1.2	71	1.8	1.5	90	2.2	1.9	100	2.5	2.1	115	2.9	2.4	134	3.3	2.8	148	3.7	3.1
15 min	53	1.3	1.2	68	1.7	1.5	87	2.2	1.9	97	2.4	2.2	111	2.8	2.5	129	3.2	2.9	143	3.6	3.2
16 min	52	1.3	1.2	66	1.7	1.6	84	2.1	2.0	94	2.3	2.3	108	2.7	2.6	126	3.1	3.0	139	3.5	3.3
17 min	50	1.3	1.3	65	1.6	1.6	82	2.0	2.1	92	2.3	2.3	105	2.6	2.7	120	3.0	3.1	135	3.4	3.4
18 min	49	1.2	1.3	63	1.6	1.7	80	2.0	2.1	89	2.2	2.4	102	2.5	2.7	119	3.0	3.2	131	3.3	3.5
20 min	47	1.2	1.4	60	1.5	1.8	76	1.9	2.3	85	2.1	2.5	97	2.4	2.9	113	2.8	3.4	125	3.1	3.7
25 min	42	1.0	1.6	53	1.3	2.0	68	1.7	2.5	76	1.9	2.8	87	2.2	3.2	101	2.5	3.8	112	2.8	4.2
30 min	38	0.9	1.7	49	1.2	2.2	62	1.5	2.8	69	1.7	3.1	79	2.0	3.5	92	2.3	4.1	89	2.2	4.0
35 min	35	0.9	1.8	45	1.1	2.3	57	1.4	3.0	64	1.6	3.3	73	1.8	3.8	84	2.1	4.4	88	2.2	4.6
40 min	32	0.8	1.9	42	1.0	2.5	53	1.3	3.1	59	1.5	3.5	68	1.7	4.0	79	2.0	4.7	87	2.2	5.2
45 min	30	0.8	2.0	39	1.0	2.6	49	1.2	3.3	55	1.4	3.7	63	1.6	4.3	74	1.8	4.9	81	2.0	5.5
50 min	29	0.7	2.1	37	0.9	2.7	47	1.2	3.5	52	1.3	3.9	60	1.5	4.5	69	1.7	5.2	77	1.9	5.7
55 min	27	0.7	2.2	35	0.9	2.9	44	1.1	3.6	49	1.2	4.1	56	1.4	4.6	66	1.6	5.4	73	1.8	6.0
60 min	26	0.6	2.3	33	0.8	3.0	42	1.0	3.8	47	1.2	4.2	54	1.3	4.8	63	1.6	5.6	69	1.7	6.2
75 min	22	0.6	2.5	29	0.7	3.2	37	0.9	4.1	41	1.0	4.6	47	1.2	5.3	55	1.4	6.2	61	1.5	6.8
90 min	20	0.5	2.7	26	0.6	3.5	33	0.8	4.4	37	0.9	5.0	42	1.1	5.7	49	1.2	6.6	55	1.4	7.4
2 hr	17	0.4	3.0	21	0.5	3.8	27	0.7	4.9	31	0.8	5.5	36	0.9	6.4	42	1.0	7.5	46	1.2	8.3
3 hr	13	0.3	3.4	16	0.4	4.4	21	0.5	5.7	24	0.6	6.5	28	0.7	7.5	33	0.8	8.8	36	0.9	9.8
4 hr	11	0.3	3.8	14	0.3	4.9	18	0.4	6.4	20	0.5	7.2	23	0.6	8.4	27	0.7	9.8	31	0.8	11.0
5 hr	9	0.2	4.1	12	0.3	5.3	15	0.4	6.9	18	0.4	7.9	20	0.5	9.1	24	0.6	10.7	27	0.7	12.0
6 hr	8	0.2	4.3	10	0.3	5.6	14	0.3	7.4	16	0.4	8.4	18	0.5	9.8	21	0.5	11.6	24	0.6	12.9
8 hr	7	0.2	4.8	9	0.2	6.2	11	0.3	8.2	13	0.3	9.4	15	0.4	10.9	18	0.4	13.0	20	0.5	14.5
10 hr	6	0.1	5.2	8	0.2	6.8	10	0.2	8.9	11	0.3	10.2	13	0.3	11.9	16	0.4	14.2	18	0.4	15.9
12 hr	5	0.1	5.5	7	0.2	7.2	9	0.2	9.6	10	0.3	11.0	12	0.3	12.8	14	0.4	15.2	16	0.4	17.1
14 hr	5	0.1	5.9	6	0.2	7.6	8	0.2	10.2	9	0.2	11.7	11	0.3	13.7	13	0.3	16.3	15	0.4	18.3
16 hr	4	0.1	6.2	6	0.1	8.0	7	0.2	10.7	9	0.2	12.3	10	0.3	14.4	12	0.3	17.2	13	0.3	19.3
18 hr	4	0.1	6.5	5	0.1	8.4	7	0.2	11.2	8	0.2	12.9	9	0.2	15.1	11	0.3	18.0	13	0.3	20.3
20 hr	4	0.1	6.7	5	0.1	8.8	7	0.2	11.7	8	0.2	13.5	9	0.2	15.8	11	0.3	18.8	12	0.3	21.2
22 hr	4	0.1	7.0	5	0.1	9.1	6	0.2	12.2	7	0.2	14.0	8	0.2	16.4	10	0.2	19.6	11	0.3	22.0
24 hr	3	0.1	7.2	4	0.1	9.4	6	0.1	12.6	7	0.2	14.5	8	0.2	17.0	9	0.2	20.3	11	0.3	22.8
36 hr	3	0.1	8.3	3	0.1	10.8	5	0.1	14.6	5	0.1	16.9	6	0.2	19.8	7	0.2	23.8	8	0.2	26.8
48 hr	2	0.1	9.1	3	0.1	11.9	4	0.1	16.1	4	0.1	18.7	5	0.1	22.0	6	0.2	26.4	7	0.2	29.8
60 hr	2	0.0	9.7	2	0.1	12.7	3	0.1	17.3	4	0.1	20.1	4	0.1	23.7	5	0.1	28.4	6	0.1	32.1
72 hr	2	0.0	10.1	2	0.1	13.4	3	0.1	18.2	3	0.1	21.2	4	0.1	24.9	5	0.1	30.0	5	0.1	33.9

Required Pump Discharge 5.2  
Volume of Storage in Tank 7.4  
Volume of Total Storage 17.1  
Aboveground ponding = 9.8



- LEGEND:**
- AHD AUSTRALIAN HEIGHT DATUM
  - BM BENCH MARK
  - TEL COMMUNICATION PIT
  - E-G EAVE & GUTTER
  - FL FLOOR LEVEL
  - S STEP
  - AW AWNING
  - RL REDUCED LEVEL
  - TK TOP OF KERB
  - VC VEHICLE CROSSING
  - AW AWNING
  - WM WATER METER
  - NS NATURAL SURFACE
  - EC EDGE OF CONCRETE
  - KBINV KERB INVERT LEVEL
  - TOP WINDOW TOP
  - SILL WINDOW SILL
  - SHURB SHURB
  - TR TREE
  - PP POWER POLE
  - SV SEWER VENT
  - SMH SEWER MAN HOLE
  - FH FIRE HYDRANT



- NOTES**
1. EXCAVATE AREA APPROX. 3.3m WIDE BY 2.2m LENGTH. THE FLOOR OF THE EXCAVATION MUST BE FLAT, WITHOUT HIGH POINTS. AN EXCAVATED DEPTH OF 100mm ACCOMMODATES A BEDDING LAYER 50mm THICK AND GRID SET DOWN OF 50mm PER UNIT.
  2. BEDDING MATERIAL SHALL BE SAND OR OTHER SUITABLE APPROVED MATERIAL. BEDDING MATERIAL SHALL BE EVENLY RAKED OVER FLOOR OF EXCAVATION TO A DEPTH SLIGHTLY MORE THAN 50mm. ENSURE BEDDING IS LEVEL IN BOTH DIRECTIONS.
  3. LOWER CATTLE GRID ONTO THE PREPARED BASE. ENSURE THAT NO PART OF THE UNIT IS SITTING ON ANY HIGH POINT.
  4. BACKFILL AND COMPACT AROUND GRID. GRADE EXCAVATED MATERIAL UP TO GRID ON EACH SIDE TO FORM A RAMP. IF DEPRESSIONS OCCUR ON THESE RAMPS WITH USE, ADD ADDITIONAL MATERIAL.
  5. MAINTAIN SHAKER GRIDS IN CLEAN AND SERVICEABLE CONDITION DURING TOTAL TIME OF USAGE.
  6. MINIMUM LENGTH OF SHAKER PAD = 5 UNITS.

