

Hargrave Apartments
1-5 Hargrave St & 38-40 Orth St
KINGSWOOD
Residential Apartments

Stormwater Concept DA Submission Report





CONTENTS

Cor	itents	2
1.	Executive Summary	4
	Proposed Development and Drainage System Overview	
3.	Rainwater Reuse System	5
4.	Water Quality	7
5.	Appendix A – Piping Calculations	10
6.	Appendix A - Drawings	13

Document Control Sheet

Title	Hargrave Apartments Kingswood – Stormwater Concept DA Submission Report
Project	Hargrave Apartments
Description	Stormwater Concept DA Submission Report
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Revision History

Issued To		Revision and Date								
	REV	Α								
	DATE	12/09/2016								
	REV									
	DATE									
	REV									
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160913 Hargrave Apartments Stormwater Concept Report |

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Page 3 of 19

1. EXECUTIVE SUMMARY

JHA Consulting Engineers have been engaged to provide details of the proposed stormwater methodology for the development site.

The proposed residential development will connect to the existing Council stormwater main in Orth Street as outlined within this report. The stormwater runoff from the roof areas will discharge to a pollution control treatment plant via a 120 m³ rainwater reuse tank. Generally, rainfall runoff from all other paved and landscaped areas will be collected via a system of surface inlet pits and discharge to Council main via a treatment plant. Storms in excess of the piped drainage system will travel overland to both Hargraves and Orth Streets.

The system will drain to the existing Council stormwater drainage system in Orth Street approximately 70 m east of the development site.

The development site is not within a zone requiring On Site Detention for the stormwater system.

The proposed treatment plant will reduce the pollution loads from the site in accordance with Council's draft policy for Water Management.

2. PROPOSED DEVELOPMENT AND DRAINAGE SYSTEM OVERVIEW

The new development comprises of 3 basement levels of car parking and 7 levels of residential apartments with a total of approximately 121 apartments.

The drainage from the roof areas will be directed to a rainwater reuse storage tank of 120m³ effective capacity. This tank will overflow to the stormwater treatment plant. This system will incorporate a syphonic drainage system to minimise the depth of the reuse storage tank installation.

The runoff from all ground level hardstand and landscaped areas will generally be directed to the stormwater treatment plant located adjacent to the car park exit to Orth Street on the northern side of the development. The runoff from the Level 6 open communal area will also be directed to the treatment plant.

The roof system will be designed to have drainage capacity for the 5 minute duration 100 year ARI storm to discharge to the reuse storage tank. The reuse inlet system will have capacity for a first flush diversion system for 20 litre per 100 m² of roof area with the discharge draining to a pump out system located in basement 3 car park.

The overflow capacity for the reuse storage tank will be capable of the 5 minute duration 100 year ARI storm with the overflow directed to a junction pit prior to the treatment plant to enable excess rainfall to bypass the capacity of the treatment plant as necessary. The treatment plant will discharge via a new piping system extended from Council's existing piped stormwater system in Orth Street.

The ground level drainage system will have the capacity for a 5 minute duration 20 year ARI storm with the runoff from greater storms directed via overland flow paths to either Hargrave or Orth Streets.

The treatment plant will be of SPEL manufacture, as detailed in the Water Quality section, which will discharge from the site to a new 375 diameter stormwater drainage piping system connecting to the existing Council drainage system in Orth Street. The new drainage system will include a new inlet pit adjacent to the discharge point from the site and connect to the existing Council stormwater inlet pit approximately 70 m east of the site in Orth Street.

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160913 Hargrave Apartments Stormwater Concept Report | Executive Summary

3. RAINWATER REUSE SYSTEM

The Rainwater Reuse System has been designed to provide maximum reuse from the stormwater collected from the roof area.

The storage capacity has been determined from actual rainfall data from the Bureau of Meteorology's closest rainfall data site and the inflow averaged from the average monthly rainfalls from 1971 to 2015.

The calculations and assumption for the volume of reuse water required are set out in Tables 3.1 & 3.2 below.

It is noted that the estimated volume of usage exceeds the actual storage volume but given that the average rainfall is approximately 61 mm per month it is not considered good practice to oversize the storage volume for minimal return on the volume that can be utilised.

POPULATION ESTIMATION								
STYLE OF APARTMENT	Total Development	No. of Occupants	Total Population					
STUDIO	0	1	0					
1 BEDROOM	66	2	132					
2 BEDROOM	54	3	162					
3 BEDROOM	1	4	4					
TOTAL APARTMENTS	121							
COMMERCIAL & RETAIL	0	m^2						
TOTAL POPULATION			298					
IRRIGATION AREA		500	m2					

Table 3.1

WATER USAGE ESTIMATION								
ACTIVITY	USAGE WEEKL USAGE		MONTHLY USAGE					
TOILET FLUSH (AAA Rated dual flush)	18	37548	150192					
Irrigation	25mm / m ²	12500	50000					
TOTAL LITRES		50,048	200,192					
Average litres per								
Apartment		414	1654					

Assumptions: Only WC's and irrigation are fed from rainwater reuse

Table 3.2

The storage volume of 120,000 litres of reuse water will be able to provide approximately 40% of the expected usage demand annually. Our assessment is that even an increase of storage volume to approximately 500,000 litres, the usage return would only increase to approximately 43%.

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The capacity for the storage volume for the first flush diversion is noted in Table 3.4 below. It has been assumed that the pollution load in regards to the roof water runoff would be medium to heavy thus 20 litre per $100m^2$ of roof area has been used. The first flush system will incorporate a system of diverters and 300 diameter piping to store the required volume. The first flush system will drain to a pump out pit located in the basement 3 car park which discharges to the stormwater system for the external ground level runoff.

The proposed volume of first flush will assist in maintaining the cleanest reuse water and also eliminate constant cleansing of the reuse storage tanks even for small intensity and duration of minor storm events.

First Flush		
Capacity		
Light Pollution	5	litres per 100 m2
Heavy Pollution	20	litres per 100 m2
Roof Area	1590	m2
Proposed First Flush	20	litres per 100 m2
Storage Required	318	litres
Proposed 300mm pipe to		
store first flush		
Capacity =	70.686	litres per meter
Length Required	4.498769205	m
Length to be provided	5	m min

Table 3.4

The stored reuse rainwater, will be pumped via a set of dual pumps and filters to distribute the reuse rainwater to all water closet cisterns and the landscaping irrigation system. The water supply to the cisterns will be supplemented with a potable water supply in accordance with Sydney Water requirements.

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160913 Hargrave Apartments Stormwater Concept Report | Rainwater Reuse System

4. WATER QUALITY

Generally, the discharge from the site roof and ground level will be directed through a SPEL treatment system as defined and detailed below.

Council's policy requirements are as noted below together with the MUSIC Model results for the reductions to be achieved with the proposed treatment system.

Pollutant	Description	Council Retention Criteria	Reduction Achieved
Litter	All anthropogenic material (cans, bottles, wrappings etc.)	70% of material >= 5 mm diameter	100%
Coarse Sediment	Coarse Sand (>=0.5mm)	80% of the load for particles <+0.5mm diameter	80%
Nutrients	Total Phosphorus & Total Nitrogen	45% retention of the load for each	Phosphorus – 64% Nitrogen – 53.8%
Fine Particles	Fine Sand (>=0.05mm)	50% of the load for particles <=0.1um diameter	50%
Free Oil and Grease	Free floating viscous liquids >= 150um that do not emulsify in aqueous solutions	90% of the load with no visible discharge	90%

Table 4.1 – Penrith Council Pollution Retention Criteria – Table 2 of Section C3 Water Management with Reduction Achieved as modelled noted

We have undertaken a MUSIC model to determine the reduction in pollutants utilising the nominated SPEL treatment plant. The results of this model are as per the below.

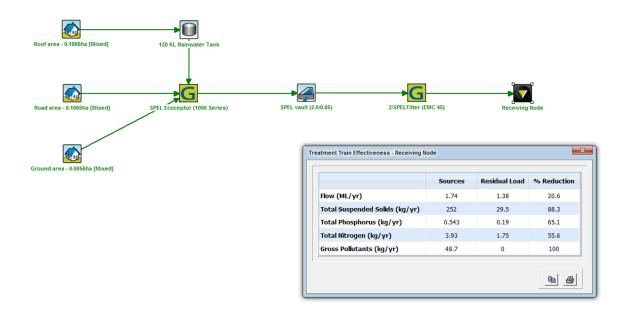
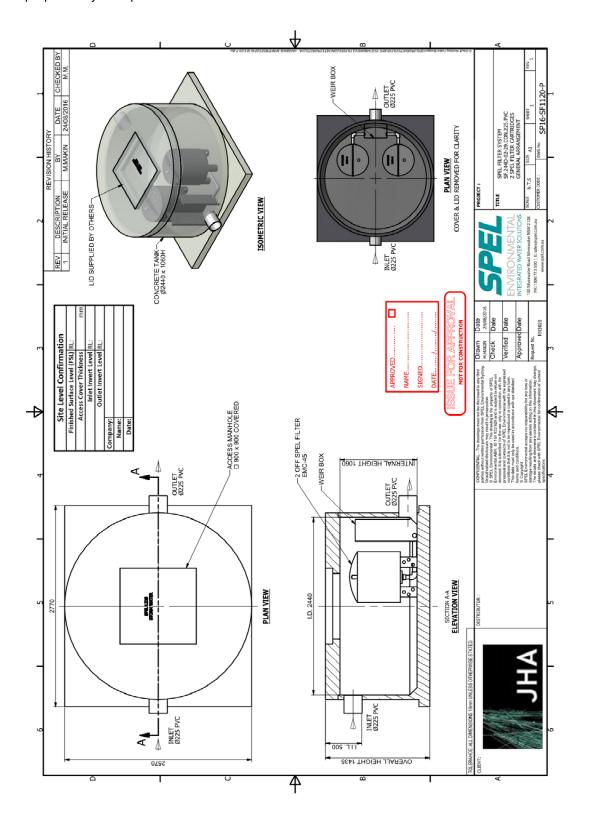
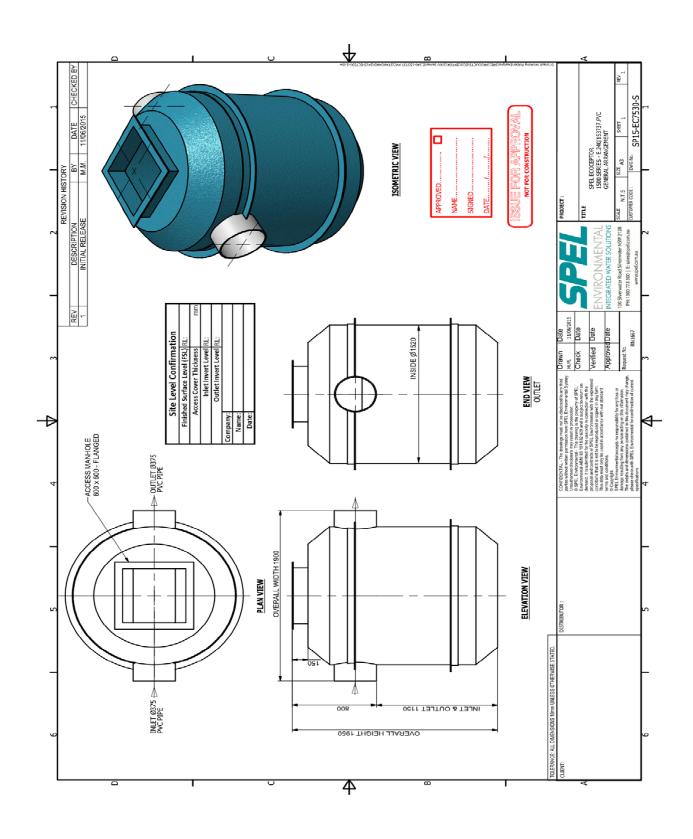


Figure 1 – MUSIC Results Screen Snapshot

The SPEL treat System will have all surface runoff directed to a SPEL Ecoceptor (E.240/103737). This includes the overflow from the rainwater reuse storage tank which is fitted with a first flush system which will remove gross pollutants and organic load from the reuse storage tank.

The Ecoceptor then drains to two SPEL Filter (EMC 45) units installed in series vial a SPEL Vault. This will ensure that all runoff is treated prior to any discharge from the site. The details of the proposed system parts are included below.





5. APPENDIX A – PIPING CALCULATIONS

The piping calculations presented below are preliminary but indicate that the concept is able to be achieved. The design basis is for a 20 yr ARI 5 minute storm with the pipe capacity determined as a pipe flowing full.

160913 Hargrave Apartments Stormwater Concept Report | Appendix A – Piping Calculations

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PIPE FLOWING FULL CALCULATIONS



Level 23, 101 Miller Street, North Sydney, NSW 2060

PO Box 3, North Sydney, NSW 2059

D. d. d.		1000	RANGE III	I-I-NI		D	1.	D		20
Project:		Job No:	400404	Designed:			Frequenc	20 year		
HARGRAVES AF	AKIMEN			Data	160181				Intensity:	213 PVC
Address:	0.41.04		owoon.	Date:	0.0 40	Sheet:	1	Materia	l:	PVC
1-5 Hargrave Street and 38-40	Orth Stre	et KING	SWOOD	l	8-Sep-16	of:	2	Mannin	ac "n":	0.013
	c	UB-ARE	Λ	TOTAL	Max Time	Intensity	Flow	PIPE	GRADE	PIPE
SECTION	3			TOTAL	Con.	intensity	Q	DIAM.	GRADE	CAPACITY
SECTION	Α	С	AxC	AxC	min	mm/hr	l/s	mm	1 in X	l/s
Reuse Water Drainage					111111	111111/111	I/S	111111	11117	1/5
FIRST FLUSH-STORAGE	1648.0	0.9	1483.2	1483.2	5.0	272.00	112.06	Dir Cumi	nonic Syst	am.
Storage Overflow to Junction Pit	1040.0	0.9	1403.2	1483.2	5.0	272.00				182.98
Ground Level Drainage				1403.2		272.00	112.00	3/5	100	102.90
SIP2	54.0	0.8	43.2	43.2		213.00	2.56	100	200	3.70
SIP2-SIP3	34.0	0.0	43.2	62.4	5.0		3.69	100	200	3.70
SIP3	70.0	0.8	56.0	56.0	5.0	213.00	3.31	100	200	3.70
SIP3-SIP4	70.0	0.0	30.0	118.4	5.0	213.00	7.01	150	200	11.66
SIP4	60.0	0.8	48.0	48.0	3.0	213.00	2.84	100	200	3.70
SIP4-SIP5	00.0	0.0	40.0	166.4	5.0	213.00	9.85	150	200	11.66
SIP5	19.0	0.8	15.2	15.2	5.0	213.00	0.90	100	200	3.70
SIP5-SIP6	10.0	0.0	10.2	181.6	5.0	213.00	10.74	150	200	11.66
SIP6 & DP1 & DP3	88.5	0.8	70.8	70.8	0.0	213.00	4.19	100	200	3.70
SIP6-SIP7	00.0	0.0	10.0	252.4	5.0		14.93	150	200	11.66
SIP7	104.0	0.8	83.2	83.2	0.0	213.00	4.92	100	200	3.70
SIP7-SIP8	101.0	0.0	00.2	335.6	5.0	213.00	19.86	225	45	71.83
SIP8 & DP2	51.3	0.8	41.0	41.0	0.0	213.00	2.43	100	200	3.70
SIP8-SIP36	0110	0.0		376.6	5.0		22.28	225		71.83
SIP36	57.0	0.8	45.6	45.6	0.0	213.00	2.70	100		3.70
SIP36-SIP9	0110	0.0		422.2	5.0	213.00	24.98	225	45	71.83
SIP9	17.0	0.8	13.6	13.6		213.00	0.80	100	200	3.70
SIP9-SIP10				435.8	5.0		25.78	225	200	34.07
SIP10	49.0	0.8	39.2	39.2		213.00	2.32	100		3.70
SIP10-SIP11				475.0	5.0	213.00	28.10	225	200	34.07
SIP11	10.5	0.8	8.4	8.4		213.00	0.50	100	200	3.70
SIP11-SIP12				483.4	5.0	213.00	28.60	225	200	34.07
SIP12	9.0	0.8	7.2	7.2		213.00	0.43	100	200	3.70
SIP12-SIP13				490.6	5.0	213.00	29.03	225	200	34.07
SIP13	70.0	0.8	56.0	56.0		213.00	3.31	100	200	3.70
SIP13-SIP14				546.6	5.0		32.34	225	200	34.07
SIP14	28.0	0.8	22.4	22.4		213.00	1.33	100	200	3.70
SIP14-SIP15				569.0	5.0		33.67	225		69.55
SIP15	14.0	0.8	11.2	11.2		213.00	0.66			3.70
SIP15-SIP16				580.2	5.0		34.33	225	48	69.55
SIP16	12.0	0.8	9.6	9.6		213.00	0.57	100		3.70
SIP16-SIP17				589.8	5.0					69.55
SIP17	16.0	0.8	12.8			213.00				3.70
SIP17-GTD1				602.6			35.65			69.55
GTD1	20.5	0.8	16.4	16.4		213.00		100		3.70
GTD1-SIP18				619.0	5.0			225		69.55
SIP18	9.0	0.8	7.2	7.2		213.00	0.43	100		3.70
SIP18-SIP19				626.2	5.0		37.05			69.55
SIP19	6.5	0.8	5.2	5.2		213.00				3.70
SIP19-Junction Pit				631.4	5.0	213.00	37.36	225	48	69.55
								<u> </u>		

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160913 Hargrave Apartments Stormwater Concept Report | Appendix A – Piping Calculations

Document Set ID: 7343784 Version: 1, Version Date: 22/09/2016

PIPE FLOWING FULL CALCULATIONS



Level 23, 101 Miller Street, North Sydney, NSW 2060

PO Box 3, North Sydney, NSW 2059

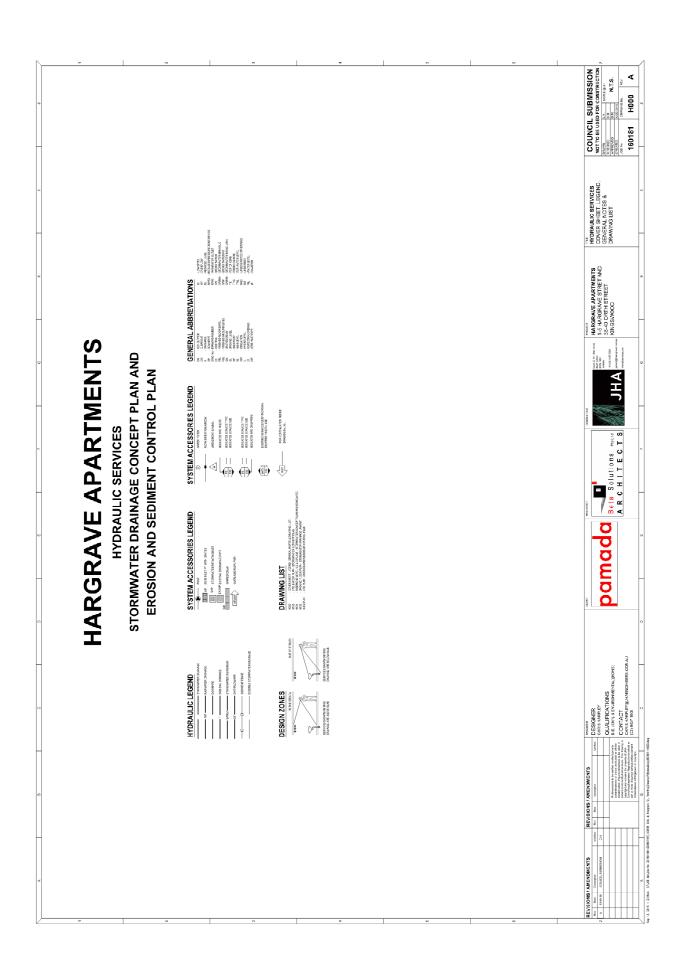
Project:		INN	BWW COLLIN	Joh Mar		Doniens	ı.	Decise	Eroginos	20
Project:		Job No:	160181	Designed			Frequenc Intensity:			
HARGRAVES Address:	AFARIMEN			Date:	100181	Sheet:	_	Materia		PVC
1-5 Hargrave Street and 38-4	40 Orth Stree	+ KING	SWOOD	Date.	8-Sep-16	l	2	wateria		FVC
1-5 margrave Street and 30-4	-0 01111 01100	i King	311000		0-3eh-10	of:	2	Manning	gs "n":	0.013
SUB-AREA					Max Time	Intensity	Flow	PIPE	GRADE	PIPE
SECTION		С	A v. C	A × C	Con.		Q	DIAM.		CAPACITY
	А	C	AxC	AxC	min	mm/hr	l/s	mm	1 in X	l/s
SIP1	24.0	0.8	19.2	19.2		213.00	1.14	100	200	3.70
SIP1-GTD2				19.2	5.0	213.00	1.14	100	200	3.70
GTD2	55.0	0.8	44.0	44.0		213.00	2.60	100	200	3.70
GTD2-SIP20				63.2	5.0	213.00	3.74	100	20	11.71
SIP20	23.0	0.8	18.4	18.4		213.00	1.09	100	200	3.70
SIP20-SIP21				81.6	5.0	213.00	4.83	100	20	11.71
SIP21	24.0	0.8	19.2	19.2		213.00	1.14	100	200	3.70
SIP21-SIP23	\bot			100.8	5.0	213.00	5.96	150	27	31.73
SIP22 & SIP23	53.0	0.8	42.4	42.4		213.00	2.51	100	200	3.70
SIP23-SIP24	+			143.2	5.0	213.00	8.47	150	40	26.07
SIP24	49.0	0.8	39.2	39.2		213.00	2.32	100	200	3.70
SIP24-SIP34				182.4	5.0	213.00	10.79	150	40	26.07
SIP34	17.5	0.8	14.0	14.0		213.00	0.83	100	200	3.70
SIP34-SIP25	+			196.4	5.0	213.00	11.62	150	40	26.07
SIP25	20.0	0.8	16.0	16.0		213.00	0.95	100	200	3.70
SIP25-SIP26				212.4	5.0	213.00	12.57	150	40	26.07
SIP26	127.0	0.8	101.6	101.6		213.00	6.01	150	200	11.66
SIP26-SIP27	+			314.0	5.0	213.00	18.58	225	200	34.07
SIP27	71.0	0.8	56.8	56.8		213.00	3.36	100	200	3.70
Ramps to Pumpout	112.0	0.8	89.6	89.6	5.0	213.00	5.30	150	200	11.66
SIP27-SIP28				460.4	5.0	213.00	27.24	225	200	34.07
SIP28	26.0	0.8	20.8	20.8		213.00	1.23	100	200	3.70
SIP28-SIP29	0.5.0		E0.0	481.2	5.0	213.00	28.47	225	200	34.07
SIP29 & DP4	65.3	0.8	52.2	52.2		213.00	3.09	100	200	3.70
SIP29-SIP30		0.0		533.4	5.0	213.00	31.56	225	200	34.07
SIP30	5.0	0.8	4.0	4.0		213.00	0.24	100	200	3.70
SIP30-SIP31	40.0		04.4	537.4	5.0	213.00	31.80	225	200	34.07
SIP31	43.0	0.8	34.4	34.4	F 0	213.00	2.04	100	200	3.70
SIP31-SIP32	7.0			571.8	5.0	213.00	33.83	225	200	34.07
SIP32 SIP32-SIP33	7.0	0.8	5.6	5.6	F 0	213.00	0.33	100 225	200 200	3.70
SIP32-SIP33 SIP33	50.0	0.0	40.0	577.4 40.0	5.0	213.00	34.16	100		34.07
	50.0	0.8	40.0		E 0		2.37		200 200	3.70
SIP33-Junction Pit Total to Treatment Plant				617.4 2732.0	5.0	213.00	36.53	225	200	34.07
Total to Treatment Plant	+ +			2/32.0						
TREATMENT PLANT -	+ +									
COUNCIL PIT				2732.0	5.0	213.00	161.64	375	100	182.98

160913 Hargrave Apartments Stormwater Concept Report | Appendix A – Piping Calculations JHA

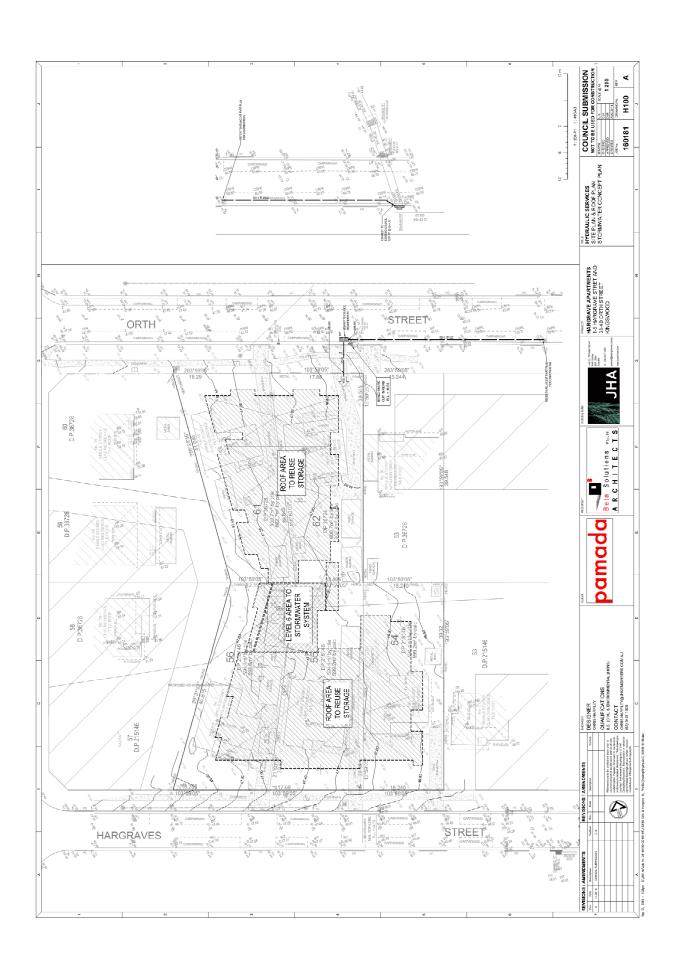
6. APPENDIX B - DRAWINGS

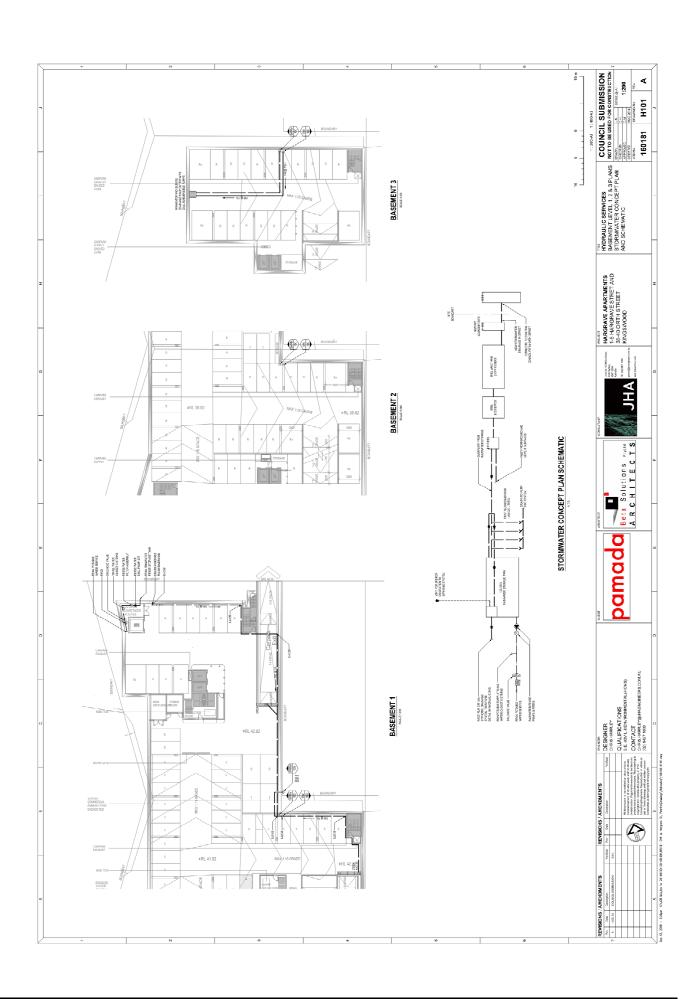
Project	Drawing No	Revision	Title
160181	H000	Α	Cover Sheet
160181	H100	Α	Site Plan & Roof Plan
160181	H101	Α	Basements 1, 2 & 3 and Stormwater
			Concept Plan Schematic
160181	H102	Α	Ground Level
160181	H-ESCP-01	Α	Erosion and Sediment Control Plan

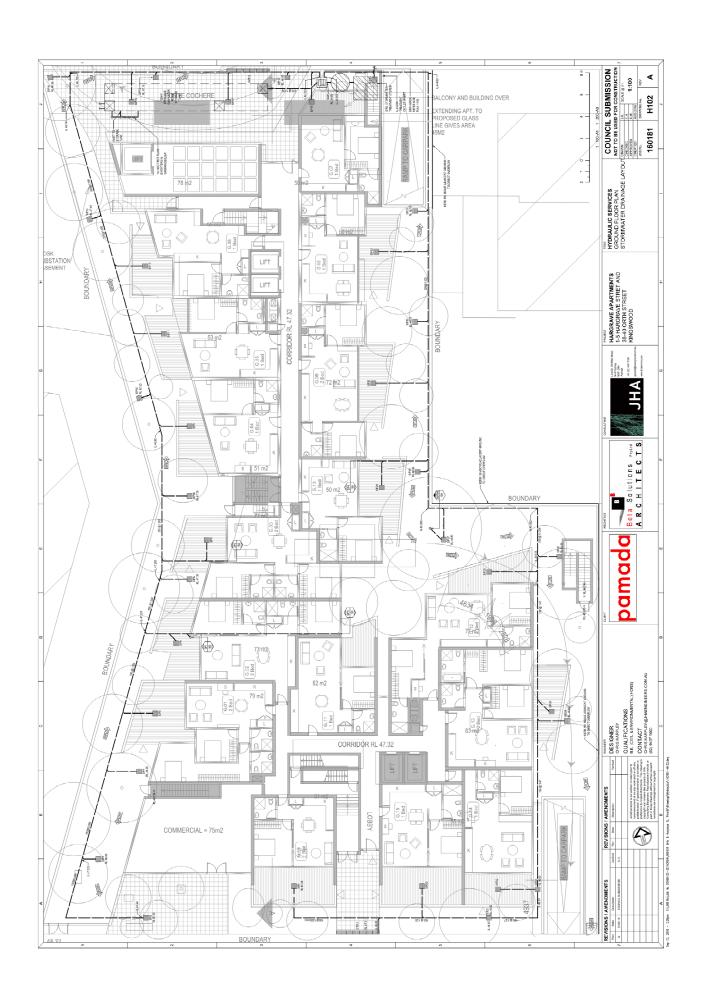
JHA 160913 Hargrave Apartments Stormwater Concept Report | Appendix B - Drawings

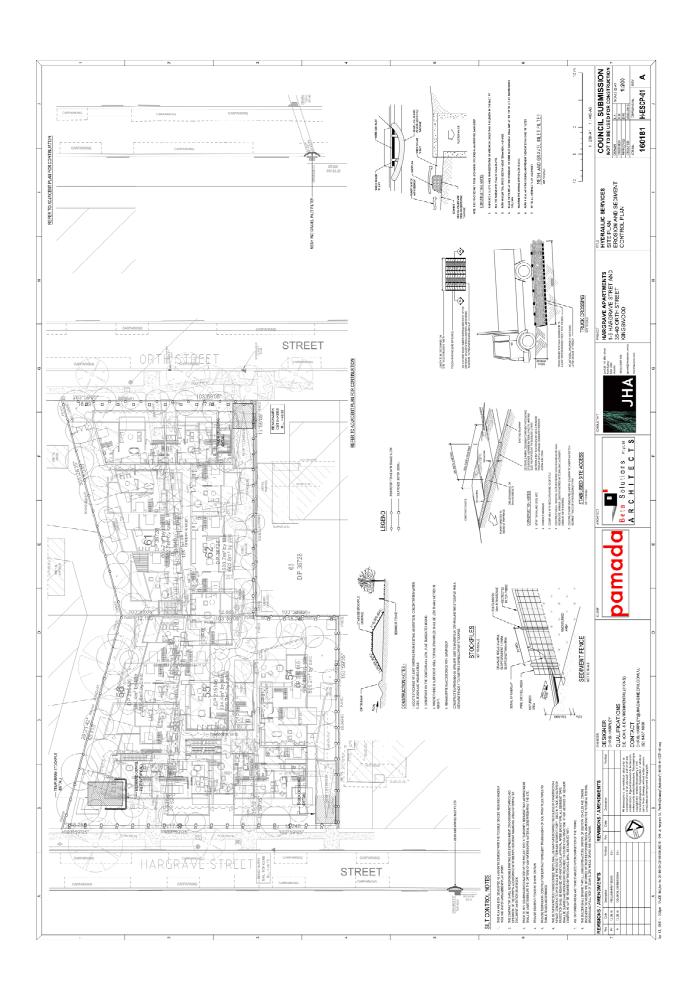


JHA 160913 Hargrave Apartments Stormwater Concept Report | Appendix B - Drawings









7. APPENDIX C - CHECKLIST

APPENDICES

Appendix A

Checklist for Stormwater Concept Plan

Surve	y Information	Yes	No	NA
1.	Site boundaries are clearly indicated	x		
2.	North point shown	X		
3.	Services within the public footway			x
4.	Site features including tree, structures, depressions	х		
5.	Contours at 0.1m for flat sites ranging to 0.5m for steep sites and extending 10m into adjoining properties	х		
6.	Top of kerb levels	Х		
7.	Boundary levels	Х		
8.	Levels to AHD where site is affected by overland flow, flooding or where works on Council's drainage network are required.	x		
9.	Benchmarks indicated	×		
Gener		Yes	No	NA
Gener	al	res	NO	NA
1.	Plans to scale of 1:100 or 1:200	х		
2.	Designers name, qualifications and contact details are included on the plans	x		
3.	Design report submitted including details of any variations	Х		
4.	Plans are consistent with architectural and landscape plans	х		
5.	100 year ARI overland flow extents provided	X		
6.	Development layout and proposed driveway locations are clearly indicated	X		
7.	Drainage layout with preliminary pipe sizing and levels	x		
8.	Preliminary calculations to indicate that the proposed design is achievable, as required	x		
9.	Proposed finished floor, garage and ground surface levels	Х		
10	Freeboard to finished levels has been achieved	х		
11	Location and level of any proposed retaining walls			x
12	Proposed connection point to Council's stormwater system	х		
13	. Appropriate tail water selected			X
14	. The proposal will not have adverse impact on other properties or the stormwater network	х		

Penrith City Council Page 28 of 53

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Stormwater Drainage for Building Developments

HARGRAVE APARTMENTS

HYDRAULIC SERVICES STORMWATER DRAINAGE CONCEPT PLAN AND **EROSION AND SEDIMENT CONTROL PLAN**

HYDRAULIC LEGEND

VRM ———— STORMWATER RISING MAIN SEDIMENT FENCE

EXISTING STORMWATER DRAINAGE

SYSTEM ACCESSORIES LEGEND

SYSTEM ACCESSORIES LEGEND WATER FILTER FLOW DIRECTION ARROW AMENDMENT SYMBOL INDICATES PIPE RISERS INDICATES SERVICE TYPE

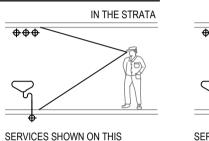
INDICATES SERVICE SIZE INDICATES SERVICE TYPE INDICATES SERVICE SIZE INDICATES PIPE DROPPERS

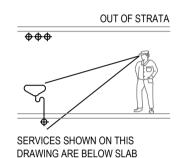
EXISTING SERVICES IDENTIFICATION. DROPPER / RISER / SIZE

FOR CONTINUATION REFER DRAWING No. H-..

DESIGN ZONES

DRAWING ARE ABOVE SLAB





DRAWING LIST

GRATED DRAIN

KIP KERB INLET PIT WITH GRATES

SWP STORMWATER PIT WITH GRATE

EX SWP EXISTING STORMWATER PIT

D

- COVER SHEET, LEGEND, GENERAL NOTES & DRAWING LIST. - SITE & ROOF PLAN - STORMWATER CONCEPT PLAN - BASEMENT LEVEL 1, 2 & 3 PLANS - STORMWATER CONCEPT PLAN AND SCHEMATIC - GROUND FLOOR PLAN - STORMWATER DRAINAGE LAYOUT H-ESCP-01 - SITE PLAN - EROSION AND SEDIMENT CONTROL PLAN

GENERAL ABBREVIATIONS

- BOX GUTTER - CLEAROUT - DRAINAGE - EXISTING - FINISHED FLOOR LEVEL - FINISHED GROUND LEVEL - GROUND LEVEL - HIGH POINT - HIGH LEVEL - IRRIGATION - INVERT LEVEL - INSPECTION OPENING - KERB INLET PIPE

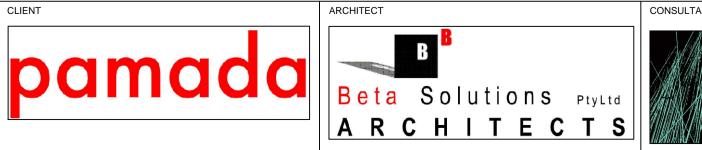
 OVERFLOW - REDUCED LEVEL RPZD - REDUCED PRESSURE ZONE DEVICE RWO - RAINWATER OUTLET SW - STORM WATER SWMH - STORMWATER MANHOLI SWP - STORM WATER PIT SWRM - STORMWATER RISING MAIN TK - TOP OF KERB TTG - TRENCH GRATE - TOP WATER LEVEL - UNLESS NOTED OTHERWISE - UNDERSIDE WL - WATER LEVEL Ø - DIAMETER

- LOW LEVEL

REVISIONS / AMENDMENTS REVISIONS / AMENDMENTS Date Description Verified Rev Date Description A 13.09.16 COUNCIL SUBMISSION All dimensions to be verified on site/s prior to commencement of on-site work and/ or off-site prefabrication. Figured dimension to be taken in preference to scaled dimensions. This drawing is CONTACT copyright and remains the property of JHA Consulting Engineers. Reproduction in whole or part of these drawings without written consent constitutes an infringement of copyright.

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HARGRAVE APARTMENTS 1-5 HARGRAVE STRET AND 38-40 ORTH STREET KINGSWOOD

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N.T.S. D.M. APPROVED CREATED AUG 2016 DRAWING No. 160181 H000

