

SMEC Testing Services Pty Ltd

ACN 101 164 792 ABN 22 101 164 792 CONSULTING GEOTECHNICAL & ENVIRONMENTAL ENGINEERS

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June 13, 2013 Project No. 19161/0013 Report No. 13/0931 LWI/ms

SITE INVESTIGATION REPORT

Client: McDonald Jones Homes Address: Lot 2183 Adina Street, Jordan Springs Proposed Development: Residential dwelling

Site Description

Approx. area (m²): 510 Approx. fall: Near level, reasonable site drainage Vegetation: Nil Improvements: Vacant – new estate

Geology, Fieldwork Details and Subsurface Conditions

The Penrith geological series sheet at a scale of 1:100,000 show the site is underlain by Triassic Age Bringelly Shale of the Wianamatta Group. Rocks within this formation comprise shale, claystone and laminite.

Two boreholes were drilled and two Dynamic cone penetrometer (DCP) tests were carried out on June 4, 2013 at the locations shown on Drawing No. 13/0931. The subsurface conditions encountered are shown on the attached borehole logs. Explanation sheets and notes relating to geotechnical reports are also attached.

When making an assessment of the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. The actual condition at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies.

The subsurface conditions consist of fill overlying silty clays. The fill is 0.3 and 0.4 metres thick and appears to have been placed as engineered material. Firm to stiff becoming very stiff natural silty clays are present to the depth of drilling, 2.5 metres.

No groundwater was observed in the boreholes during the fieldwork.



Wind Classification

The classification has been carried out in accordance with the guidelines set out in AS4055-2006 "Wind loads for housing".

The site is classified N1.

Laboratory Testing

In order to assist with determining the site classification, a shrink/swell test was carried out on a representative sample retrieved from the site. The detailed test report is attached and summarised below:

Location	Depth	Material Description	Shrink/Swell Index
	(m)		(% per ΔpF)
BH1	0.6-0.8	Brown and grey silty clay	2.6

Site Classification

The classification has been prepared in accordance with the guidelines set out in the "Residential Slabs and Footings" Code, AS2870 - 2011.

Based on the subsurface conditions observed the site may be classified *moderately* reactive (M), provided the recommendations given below are adopted.

Foundation design and construction consistent with this classification shall be adopted as specified in the above referenced standard and in accordance with the following design details.

Foundation Design and Construction

Pad and/or strip footings founded in natural materials underlying the fill may be proportioned using an allowable bearing pressure of 100 kPa. The minimum depth of founding must comply with the requirements of AS2870.

Piers founded in the very stiff materials may be proportioned using an allowable end bearing pressure of 450 kPa provided their depth to diameter ratio exceeds a value of 4. An adhesion value of 20 kPa may be adopted below a depth of 0.75 metres.

In order to ensure the bearing values given can be achieved, care should be taken to ensure the base of the excavations is free of all loose material prior to concreting. To this end, it is recommended that all excavations be concreted as soon as possible, preferably immediately after excavating, cleaning, inspecting and approval. Pier excavations should not be left open overnight. The possibility of groundwater inflow needs to be considered when drilling the piers and pouring concrete.



The site is considered suitable for slab on ground construction provided due regard is given to the groundsurface slope.

During foundation construction, should the subsurface conditions vary to those inferred in this report, a suitably experienced geotechnical engineer should review the design and recommendations given above to determine if any alterations are required.

Soil Aggressiveness

The exposure classification for the concrete has been determined for the onsite soils. The exposure classification is obtained from Tables 5.1 and 5.2 of AS2870-2011. In regards to the electrical conductivity, the laboratory test results have been multiplied by the appropriate factor to convert the results to EC_e .

Detailed test reports are attached and summarised below, together with the exposure classification.

Sample No.			рН	Sulfate (ppm)	Exposure Classification
	EC _{1:5}	ECe			
S1/0013	0.196	1.8	5.6	120	A1

The minimum concrete strength and reinforcement cover required for the various exposure classifications are given in Tables 5.3 and 5.4 of AS2870-2011.

Reference to DLWC (2002) "Site Investigations for Urban Salinity" indicates that an EC_e value of 1.8 dS/m is consistent with non-saline soils.

Additional Comments

Attention is drawn to Appendix B of AS2870 - 2011 regarding the need to properly maintain the foundations. Surface drainage should be provided to avoid the possibility of water ponding near the building and the finished ground surface should fall at least 50 mm over a distance of one metre away from the building.

The above classification has been made assuming that the maximum depth to filling placed in any building platform will be 400 mm and that all footings will bear in either natural ground or in control filling. Prior to the placement of any filling the existing surface should be stripped of all vegetation and topsoil.



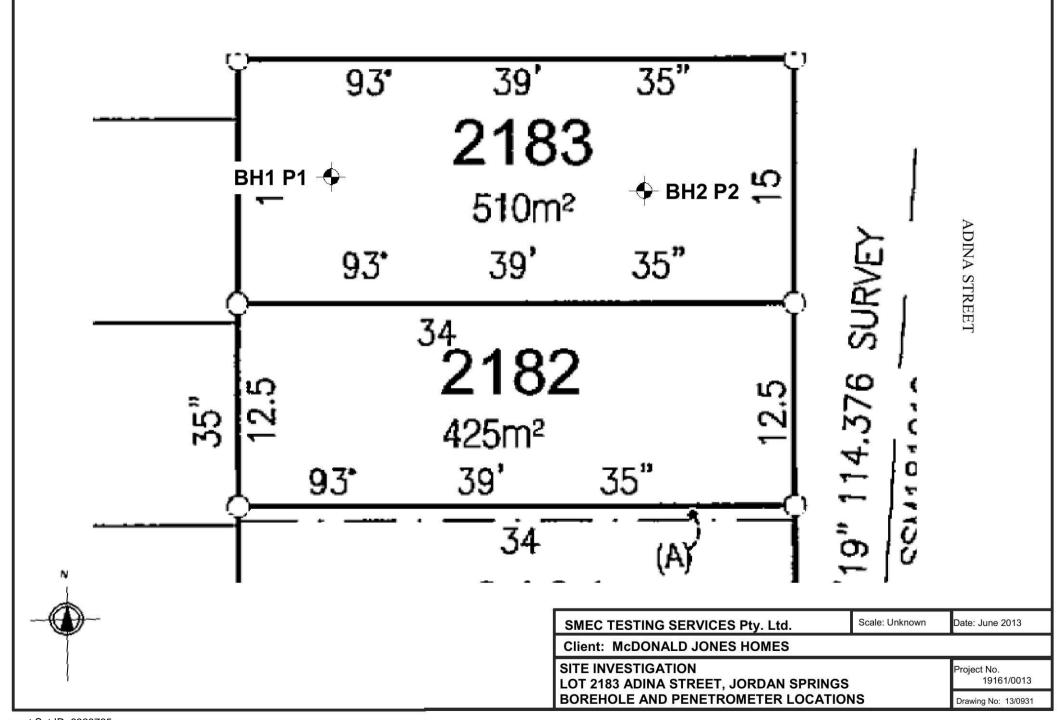
The above classification is based on the soil profiles observed at the time of testing. If site works are undertaken, the classification of the actual building platform may vary across the site depending upon the extent of the cut and/or fill and the degree of compaction of any fill. The designer of the footing system must take the above factors into account.

If excavations for rainwater or detention tanks are to be made within 6 metres of the building foundations, advice should be sought regarding their effect on the foundations.

Placing absorption trenches on the high side of the property may create abnormal moisture conditions for the foundations (Refer to Section 1.3.3 of AS2870). This could have a negative effect on the foundation performance and more than likely alter the site classification provided above.

This report has been prepared assuming the site development will be limited to one or two storey residential buildings. The information and interpretation may not be relevant if the design proposal changes (e.g. to a five-storey building involving major cuts during the site preparation). If changes occur, we would be pleased to review the report and advise on the adequacy of the investigation.

Laurie Ihnativ, BE, MEngSc, MBA, FIE Aust. Manager, SMEC Testing Services Pty Limited



Document Set ID: 6828765 Version: 1, Version Date: 08/09/2015

Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by SMEC Testing Services Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, SMEC Testing Services Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, SMEC Testing Services Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows reinterpretation and assessment of the implications for future work.

Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

Client: M Project:	fcDonald Jon Lot 2183 Adi	ina Street, Jorda	n Springs Date : June 4, 2013		REHOLE NO.:	BH 1
Location:	Refer to Dra	wing No. 13/09	31 Logged: MB		Sheet 1 of 1	
W AT TA EB RL E	S A P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			SILTY CLAY: brown, trace of organics, low plasticity, trace of gravel	CL	VERY STIFF	D-M
	S1 0.1-0.2 m		FILL FILL	CL	FIRM TO STIFF	D-M
		0.5				DA
	U50	1.0			STIFF	-
					VERY STIFF	-
					VERT SHIFT	
		2.0	SILTY CLAY: brown with grey, medium plasticity	CL	VERY STIFF	D-M
		2.5				
			BOREHOLE DISCONTINUED AT 2.5 M			
NOTES:	D - disturbed WT - level o	d sample of water table or			: Armstrong MKII	<u>I</u>
			See explanation sheets for meaning of all descriptive terms and symbols		eter (mm): 100 n Vertical (°) 0	

GEOTECHNICAL LOG - NON CORE BOREHOLE

SMEC Testing Services Pty Ltd

Client: McDonald Jones Homes Project No.: 19161/0013 BOREHOLE NO .: BH 2 Project: Lot 2183 Adina Street, Jordan Springs Date : June 4, 2013 Location: Refer to Drawing No. 13/0931 Logged: MB Sheet 1 of 1 CONSISTENCY \mathbf{M} w s (cohesive soils) 0 A T A \mathbf{S} I or RELATIVE ТА \mathbf{M} Y s Е В Р DESCRIPTION OF DRILLED PRODUCT \mathbf{M} DENSITY Т \mathbf{L} в (sands and U RL Е DEPTH 0 R Е (Soil type, colour, grain size, plasticity, minor components, observations) gravels) S (m) L Е SILTY CLAY: brown, trace of gravel, low plasticity, trace of organics CLVERY STIFF D-M FILL CLVERY STIFF D-M SILTY CLAY: brown with orange, medium plasticity 0.5 STIFF 1.0VERY STIFF 1.5 VERY STIFF D-M SILTY CLAY: brown with grey and orange CL2.0 2.5 BOREHOLE DISCONTINUED AT 2.5 M NOTES: D - disturbed sample U - undisturbed tube sample B - bulk sample Contractor: STS WT - level of water table or free water N - Standard Penetration Test (SPT) Equipment: Armstrong MKII See explanation sheets for meaning of all descriptive terms and symbols Hole Diameter (mm): 100 Angle from Vertical (°) 0

GEOTECHNICAL LOG - NON CORE BOREHOLE

SMEC Testing Services Pty Ltd

SMEC Testing Services Pty Ltd

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Dynamic Cone Penetrometer Test Report

 Project: LOT 2183 ADINA STREET, JORDAN SPRINGS
 Project No.: 1

 Client: McDONALD JONES HOMES
 Report No.: 1

Chent, WEDONALD JONES HOWES

Address: PO Box 7994, Baulkham Hills

Test Method: AS 1289.6.3.2

Project No.: 19161/0013 Report No.: 13/0931 Report Date: June 12, 2013 Page: 1 of 1

Site No.	P1	P2				
Location	Refer to Drawing No. 13/0931	Refer to Drawing No. 13/0931				
Starting Level	Surface Level	Surface Level				
Depth (m)		Pe	netration Resi	stance (blows /	150mm)	
0.00 - 0.15	13	14				
0.15 - 0.30	7	7				
0.30 - 0.45	9	7				
0.45 - 0.60	5	8				
0.60 - 0.75	4	8				
0.75 - 0.90	3	2				
0.90 - 1.05	5	5				
1.05 - 1.20	6	6				
1.20 - 1.35	7	8				
1.35 - 1.50	10	9				
1.50 - 1.65	7	11				
1.65 - 1.80	6	12				
1.80 - 1.95	9	11				
1.95 - 2.10	8	12				
2.10 - 2.25	8	11				
2.25 - 2.40	7	12				
2.40 - 2.55	12	13				
2.55 - 2.70	Discontinued	Discontinued				
2.70 - 2.85						
2.85 - 3.00						
3.00 - 3.15						
3.15 - 3.30		0				
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						
and a second the second	drilled prior to tes	ting	L	1	1	^
Kemarks. • Fre	anned phot to tes	ung		Approved S	Signatory	the Guadi V.

Laurie Ihnativ - Manager

Shrink Swell Index Report Project: Lot 2183 Adina Street, Jordon Springs

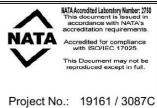
Phone: (02)9756 2166 Fax: (02)9756 1137 Email: enquiries@smectesting.com.au

14/1 Cowpasture Place, Wetherill Park NSW 2164

Client: McDONALD JONES HOMES

SMEC Testing Services Pty Ltd

	PO Box 7994, Baull od: AS1289.7.1.1	kham Hills NSW				Report Date: Page:	
Sampling I	Proceedure: AS 12	89.1.3.1 Clause 3.1.3.	2 - Thin Walled	Sampler			
STS	/ Sample No.	0013-1					
San	nple Location	Borehole 1 Refer to Drawing					
Mater	ial Description	Silty Clay, brown & grey, trace of gravel					
ſ	Depth (m)	0.7 - 0.9					
Sa	ample Date	4/06/2013					
	Moisture Content (%)	24.3					
ž	Soil Crumbling	Nil					
Shrink	Extent of Cracking	Open Cracks					
	Strain (%)	3.4					
	Moisture Content Initial (%)	23.7					
Swell	Moisture Content Final (%)	29.1					
	Strain (%)	2.7					
Inert	Inclusions (%)	1.0					
Shrink	Swell Index (%)	2.6					
Remarks:					Approved Signa	ntoryHo	lenh.
Techniciar	:: LC				Lincoln	Coleman - Seni	or Geotechnicia



Report No.: 13/0895





Environmental Division

	CERTIFICATE OF ANALYSIS								
Work Order	ES1312625	Page	: 1 of 4						
Client	: SMEC TESTING SERVICES PTY LTD	Laboratory	: Environmental Division Sydney						
Contact	: ALL REPORTS	Contact	: Client Services						
Address	: P O BOX 6989	Address	: 277-289 Woodpark Road Smithfield NSW Australia 2164						
	WETHERILL PARK NSW, AUSTRALIA 2164								
E-mail	: enquiries@smectesting.com.au	E-mail	: sydney@alsglobal.com						
elephone	;	Telephone	: +61-2-8784 8555						
acsimile	:	Facsimile	: +61-2-8784 8500						
Project	: 19161 3087C	QC Level	: NEPM 1999 Schedule B(3) and ALS QCS3 requirement						
Order number	: 10209								
C-O-C number	: 156986	Date Samples Received	: 04-JUN-2013						
Sampler	: MB	Issue Date	: 07-JUN-2013						
Site									
		No. of samples received	: 9						
Quote number	: EN/025/12	No. of samples analysed	: 9						

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. All pages of this report have been checked and approved for release.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

	NATA Accredited Laboratory 825	<i>Signatories</i> This document has been electronically	signed by the authorized signatories i	indicated below. Electronic signing has been
NATA	Accredited for compliance with	carried out in compliance with procedures sp	ecified in 21 CFR Part 11.	
INATA	ISO/IEC 17025.	Signatories	Position	Accreditation Category
		Hoa Nguyen	Senior Inorganic Chemist	Sydney Inorganics
WORLD RECOGNISED		Nanthini Coilparampil	Laboratory Manager - Inorganics	Sydney Inorganics
ACCREDITATION		Raymond Commodor	Instrument Chemist	Sydney Inorganics

Address 277-289 Woodpark Road Smithfield NSW Australia 2184 PHONE +61-2-8784 8555 Facsimile +61-2-8784 8500 Environmental Division Sydney ABN 84 009 936 029 Part of the ALS Group An ALS Limited Company

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General Comments

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

 Key :
 CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

 LOR = Limit of reporting

^ = This result is computed from individual analyte detections at or above the level of reporting



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)	Matrix: SOIL (Matrix: SOIL) Client sample ID				S1/0012	S1/0013	S1/0014	S1/0015
	Cli	ient sampli	ng date / time	[04-JUN-2013]	[04-JUN-2013]	[04-JUN-2013]	[04-JUN-2013]	[04-JUN-2013]
Compound	CAS Number	LOR	Unit	ES1312625-001	ES1312625-002	ES1312625-003	ES1312625-004	ES1312625-005
EA002 : pH (Soils)								
pH Value		0.1	pH Unit	5.8	7.0	5.6	7.8	6.9
EA010: Conductivity								
Electrical Conductivity @ 25°C		1	µS/cm	400	247	196	220	120
EA055: Moisture Content								
Moisture Content (dried @ 103°C)		1.0	%	17.2	13.7	13.8	17.7	16.4
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	280	150	120	30	100



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)	Client sample ID			S1/0016	S1/0017	S1/0018	S1/0019	
	Cli	ient sampli	ng date / time	[04-JUN-2013]	[04-JUN-2013]	[04-JUN-2013]	[04-JUN-2013]	
Compound	CAS Number	LOR	Unit	ES1312625-006	ES1312625-007	ES1312625-008	ES1312625-009	
EA002 : pH (Soils)								
pH Value		0.1	pH Unit	5.4	5.2	5.4	5.9	
EA010: Conductivity								
Electrical Conductivity @ 25°C		1	µS/cm	153	265	325	300	
EA055: Moisture Content								
Moisture Content (dried @ 103°C)		1.0	%	21.8	16.8	13.8	14.8	
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	80	260	170	290	





Environmental Division

QUALITY CONTROL REPORT

Work Order	: ES1312625	Page	ः 1 of 4
Client	: SMEC TESTING SERVICES PTY LTD	Laboratory	: Environmental Division Sydney
Contact	: ALL REPORTS	Contact	: Client Services
Address	: P O BOX 6989	Address	: 277-289 Woodpark Road Smithfield NSW Australia 2164
	WETHERILL PARK NSW, AUSTRALIA 2164		
E-mail	: enquiries@smectesting.com.au	E-mail	: sydney@alsglobal.com
Telephone	:	Telephone	: +61-2-8784 8555
Facsimile	1	Facsimile	: +61-2-8784 8500
Project	: 19161 3087C	QC Level	: NEPM 1999 Schedule B(3) and ALS QCS3 requirement
Site	ī		
C-O-C number	: 156986	Date Samples Received	: 04-JUN-2013
Sampler	: MB	Issue Date	: 07-JUN-2013
Order number	: 10209		
		No. of samples received	: 9
Quote number	: EN/025/12	No. of samples analysed	:: 9

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. All pages of this report have been checked and approved for release.

This Quality Control Report contains the following information:

- Laboratory Duplicate (DUP) Report; Relative Percentage Difference (RPD) and Acceptance Limits
- Method Blank (MB) and Laboratory Control Spike (LCS) Report; Recovery and Acceptance Limits
- Matrix Spike (MS) Report; Recovery and Acceptance Limits

NATA Accredited Laboratory 825

Accredited for compliance with

ISO/IEC 17025.



Signatories

This document has been electronically signed by the authorized signatories indicated below. Electronic signing has been carried out in compliance with procedures specified in 21 CFR Part 11.

Signatories	Position	Accreditation Category
Hoa Nguyen	Senior Inorganic Chemist	Sydney Inorganics
Nanthini Coilparampil	Laboratory Manager - Inorganics	Sydney Inorganics
Raymond Commodor	Instrument Chemist	Sydney Inorganics

Address 277-289 Woodpark Road Smithfield NSW Australia 2164 PHONE +61-2-8784 8555 FacsImile +61-2-8784 8500 Environmental Division Sydney ABN 84 009 936 029 Part of the ALS Group An ALS Limited Company

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General Comments

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

 Key :
 Anonymous = Refers to samples which are not specifically part of this work order but formed part of the QC process lot

 CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

 LOR = Limit of reporting

 RPD = Relative Percentage Difference

= Indicates failed QC



Laboratory Duplicate (DUP) Report

The quality control term Laboratory Duplicate refers to a randomly selected intralaboratory split. Laboratory duplicates provide information regarding method precision and sample heterogeneity. The permitted ranges for the Relative Percent Deviation (RPD) of Laboratory Duplicates are specified in ALS Method QWI-EN/38 and are dependent on the magnitude of results in comparison to the level of reporting: Result < 10 times LOR:-No Limit; Result between 10 and 20 times LOR:-0% - 50%; Result > 20 times LOR:-0% - 20%.

Sub-Matrix: SOIL		Laboratory Duplicate (DUP) Report							
Laboratory sample ID	Client sample ID	Method: Compound	CAS Number	LOR	Unit	Original Result	Duplicate Result	RPD (%)	Recovery Limits (%)
EA002 : pH (Soils)	(QC Lot: 2902759)								
EB1313187-001	Anonymous	EA002: pH Value		0.1	pH Unit	7.9	7.9	0.0	0% - 20%
ES1312625-001	S1/0023	EA002: pH Value		0.1	pH Unit	5.8	5.7	0.0	0% - 20%
EA010: Conductivi	ty (QC Lot: 2902760)								
EB1313187-001	Anonymous	EA010: Electrical Conductivity @ 25°C	- <u></u>	1	μS/cm	100	98	2.0	0% - 20%
ES1312625-008	S1/0018	EA010: Electrical Conductivity @ 25°C		1	μS/cm	325	301	7.7	0% - 20%
EA055: Moisture C	ontent (QC Lot: 2901987	()							
ES1312624-001	Anonymous	EA055-103: Moisture Content (dried @ 103°C)		1.0	%	22.4	21.6	3.9	0% - 20%
ED040S: Soluble N	ajor Anions (QC Lot: 29	02762)							
ES1312625-002	S1/0012	ED040S: Sulfate as SO4 2-	14808-79-8	10	mg/kg	150	160	0.0	0% - 50%



Method Blank (MB) and Laboratory Control Spike (LCS) Report

The quality control term Method / Laboratory Blank refers to an analyte free matrix to which all reagents are added in the same volumes or proportions as used in standard sample preparation. The purpose of this QC parameter is to monitor potential laboratory contamination. The quality control term Laboratory Control Sample (LCS) refers to a certified reference material, or a known interference free matrix spiked with target analytes. The purpose of this QC parameter is to monitor method precision and accuracy independent of sample matrix. Dynamic Recovery Limits are based on statistical evaluation of processed LCS.

Sub-Matrix: SOIL	Method Blank (MB)	Laboratory Control Spike (LCS) Report						
						Spike Recovery (%)	Recovery Limits (%)	
Method: Compound	CAS Number	LOR	Unit	Result	Concentration	LCS	Low	High
EA010: Conductivity (QCLot: 2902760)								
EA010: Electrical Conductivity @ 25°C		1	µS/cm	<1	1412 µS/cm	108	70	130
ED040S: Soluble Major Anions (QCLot: 2902762)								
ED040S: Sulfate as SO4 2-	14808-79-8	10	mg/kg	<10	750 mg/kg	98.0	85	109

Matrix Spike (MS) Report

The quality control term Matrix Spike (MS) refers to an intralaboratory split sample spiked with a representative set of target analytes. The purpose of this QC parameter is to monitor potential matrix effects on analyte recoveries. Static Recovery Limits as per laboratory Data Quality Objectives (DQOs). Ideal recovery ranges stated may be waived in the event of sample matrix interference.

• No Matrix Spike (MS) Results are required to be reported.

Matrix Spike (MS) and Matrix Spike Duplicate (MSD) Report

The quality control term Matrix Spike (MS) and Matrix Spike Duplicate (MSD) refers to intralaboratory split samples spiked with a representative set of target analytes. The purpose of these QC parameters are to monitor potential matrix effects on analyte recoveries. Static Recovery Limits as per laboratory Data Quality Objectives (DQOs). Ideal recovery ranges stated may be waived in the event of sample matrix interference.

• No Matrix Spike (MS) or Matrix Spike Duplicate (MSD) Results are required to be reported.





Environmental Division

INTERPRETIVE QUALITY CONTROL REPORT

Work Order	: ES1312625	Page	: 1 of 5
Client	: SMEC TESTING SERVICES PTY LTD	Laboratory	: Environmental Division Sydney
Contact	: ALL REPORTS	Contact	: Client Services
Address	: P O BOX 6989	Address	: 277-289 Woodpark Road Smithfield NSW Australia 2164
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Project	: 19161 3087C	QC Level	: NEPM 1999 Schedule B(3) and ALS QCS3 requirement
Site	i		
C-O-C number	: 156986	Date Samples Received	: 04-JUN-2013
Sampler	: MB	Issue Date	: 07-JUN-2013
Order number	: 10209		
		No. of samples received	: 9
Quote number	: EN/025/12	No. of samples analysed	: 9

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. All pages of this report have been checked and approved for release.

This Interpretive Quality Control Report contains the following information:

- Analysis Holding Time Compliance
- Quality Control Parameter Frequency Compliance
- Brief Method Summaries
- Summary of Outliers

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Analysis Holding Time Compliance

The following report summarises extraction / preparation and analysis times and compares with recommended holding times. Dates reported represent first date of extraction or analysis and precludes subsequent dilutions and reruns. Information is also provided re the sample container (preservative) from which the analysis aliquot was taken. Elapsed period to analysis represents number of days from sampling where no extraction / digestion is involved or period from extraction / digestion where this is present. For composite samples, sampling date is assumed to be that of the oldest sample contributing to the composite. Sample date for laboratory produced leachates is assumed as the completion date of the leaching process. Outliers for holding time are based on USEPA SW 846, APHA, AS and NEPM (1999). A listing of breaches is provided in the Summary of Outliers.

Holding times for leachate methods (excluding elutriates) vary according to the analytes being determined on the resulting solution. For non-volatile analytes, the holding time compliance assessment compares the leach date with the shortest analyte holding time for the equivalent soil method. These soil holding times are: Organics (14 days); Mercury (28 days) & other metals (180 days). A recorded breach therefore does not guarantee a breach for all non-volatile parameters.

Matrix: SOIL					Evaluation	: × = Holding time	breach ; ✓ = Withir	holding time.
Method		Sample Date	Ex	traction / Preparation			Analysis	
Container / Client Sample ID(s)			Date extracted	Due for extraction	Evaluation	Date analysed	Due for analysis	Evaluation
EA002 : pH (Soils)								
Soil Glass Jar - Unpreserved (EA002)				es es annañons insents ensuar			Appropriate approximation to be approximated	
S1/0023,	S1/0012,	04-JUN-2013	05-JUN-2013	11-JUN-2013	\checkmark	05-JUN-2013	06-JUN-2013	\checkmark
S1/0013,	S1/0014,							
S1/0015,	S1/0016,							
S1/0017,	S1/0018,							
S1/0019								
EA010: Conductivity								
Soil Glass Jar - Unpreserved (EA010)								
S1/0023,	S1/0012,	04-JUN-2013	05-JUN-2013	11-JUN-2013	1	05-JUN-2013	03-JUL-2013	\checkmark
S1/0013,	S1/0014,							
S1/0015,	S1/0016,							
S1/0017,	S1/0018,							
S1/0019								
EA055: Moisture Content								
Soil Glass Jar - Unpreserved (EA055-103)								
S1/0023,	S1/0012,	04-JUN-2013				05-JUN-2013	18-JUN-2013	\checkmark
S1/0013,	S1/0014,							
S1/0015,	S1/0016,							
S1/0017,	S1/0018,							
S1/0019								
ED040S : Soluble Sulfate by ICPAES								
Soil Glass Jar - Unpreserved (ED040S)								
S1/0023,	S1/0012,	04-JUN-2013	05-JUN-2013	11-JUN-2013	\checkmark	06-JUN-2013	03-JUL-2013	\checkmark
S1/0013,	S1/0014,							
S1/0015,	S1/0016,							
S1/0017,	S1/0018,							
S1/0019								



Quality Control Parameter Frequency Compliance

The following report summarises the frequency of laboratory QC samples analysed within the analytical lot(s) in which the submitted sample(s) was(where) processed. Actual rate should be greater than or equal to the expected rate. A listing of breaches is provided in the Summary of Outliers.

Matrix: SOIL	Evaluation: × = Quality Control frequency not within specification ; ✓ = Quality Control frequency within specification ;							
Quality Control Sample Type		Co	ount		Rate (%)		Quality Control Specification	
Analytical Methods	Method	QC	Reaular	Actual	Expected	Evaluation		
Laboratory Duplicates (DUP)								
Electrical Conductivity (1:5)	EA010	2	12	16.7	10.0	~	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
Major Anions - Soluble	ED040S	1	11	9.1	10.0	x	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
Moisture Content	EA055-103	1	13	7.7	10.0	x	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
pH (1:5)	EA002	2	19	10.5	10.0	~	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
Laboratory Control Samples (LCS)								
Electrical Conductivity (1:5)	EA010	1	12	8.3	5.0	1	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
Major Anions - Soluble	ED040S	1	11	9.1	5.0	~	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
Method Blanks (MB)								
Electrical Conductivity (1:5)	EA010	1	12	8.3	5.0	\checkmark	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	
Major Anions - Soluble	ED040S	1	11	9.1	5.0	1	NEPM 1999 Schedule B(3) and ALS QCS3 requirement	



Brief Method Summaries

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the US EPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request. The following report provides brief descriptions of the analytical procedures employed for results reported in the Certificate of Analysis. Sources from which ALS methods have been developed are provided within the Method Descriptions.

Analytical Methods	Method	Matrix	Method Descriptions
pH (1:5)	EA002	SOIL	(APHA 21st ed., 4500H+) pH is determined on soil samples after a 1:5 soil/water leach. This method is compliant with NEPM (1999) Schedule B(3) (Method 103)
Electrical Conductivity (1:5)	EA010	SOIL	(APHA 21st ed., 2510) Conductivity is determined on soil samples using a 1:5 soil/water leach. This method is compliant with NEPM (1999) Schedule B(3) (Method 104)
Moisture Content	EA055-103	SOIL	A gravimetric procedure based on weight loss over a 12 hour drying period at 103-105 degrees C. This method is compliant with NEPM (2010 Draft) Schedule B(3) Section 7.1 and Table 1 (14 day holding time).
Major Anions - Soluble	ED040S	SOIL	In-house. Soluble Anions are determined off a 1:5 soil / water extract by ICPAES.
Preparation Methods	Method	Matrix	Method Descriptions
1:5 solid / water leach for soluble analytes	EN34	SOIL	10 g of soil is mixed with 50 mL of distilled water and tumbled end over end for 1 hour. Water soluble salts are leached from the soil by the continuous suspension. Samples are settled and the water filtered off for analysis.



Summary of Outliers

Outliers : Quality Control Samples

The following report highlights outliers flagged in the Quality Control (QC) Report. Surrogate recovery limits are static and based on USEPA SW846 or ALS-QWI/EN/38 (in the absence of specific USEPA limits). This report displays QC Outliers (breaches) only.

Duplicates, Method Blanks, Laboratory Control Samples and Matrix Spikes

- For all matrices, no Method Blank value outliers occur.
- For all matrices, no Duplicate outliers occur.
- For all matrices, no Laboratory Control outliers occur.
- For all matrices, no Matrix Spike outliers occur.

Regular Sample Surrogates

• For all regular sample matrices, no surrogate recovery outliers occur.

Outliers : Analysis Holding Time Compliance

This report displays Holding Time breaches only. Only the respective Extraction / Preparation and/or Analysis component is/are displayed.

• No Analysis Holding Time Outliers exist.

Outliers : Frequency of Quality Control Samples

The following report highlights breaches in the Frequency of Quality Control Samples.

Matrix: SOIL

Quality Control Sample Type	Count		Rate	∋ (%)	Quality Control Specification
Method	QC	Regular	Actual	Expected	
Laboratory Duplicates (DUP)					
Major Anions - Soluble	1	11	9.1	10.0	NEPM 1999 Schedule B(3) and ALS QCS3 requirement
Moisture Content	1	13	7.7	10.0	NEPM 1999 Schedule B(3) and ALS QCS3 requirement

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E1. CLASSIFICATION OF SOILS

E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by SMEC in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-1981, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour

Soil condition

- moisture condition
- consistency or density index

Soil structure

• structure (zoning, defects, cementing)

Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

E1.2 Soil Composition

(a) Soil Name and Classification Symbol

The USC system is summarized in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils more than 50% of the material less than 60 mm is larger than 0.06 mm (60 μm).
- Fine grained soils more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 μm).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 µm
Silt (2)		2 μm to 60 μm
Sand	Fine Medium Coarse	60 μm to 200 μm 200 μm to 600 μm 600 μm to 2 mm
Gravel (3)	Fine Medium Coarse	2 mm to 6 mm 6 mm to 20 mm 20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12%) in increasing order of significance, eg sandy silty clay.

Minor components of a soil are included in the description by means of the terms "some" and "trace" as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	М
Clay	С
Organic	0
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4

TABLE E1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	Р
Silty	M
Clayey	С
Liquid Limit <50% - low to medium plasticity	L
Liquid Limit >50% - low to medium plasticity	Н

(b) Grading

"Well graded"	Good representation of all particle sizes from the largest to the smallest.
"Poorly graded"	One or more intermediate sizes poorly represented
"Gap graded"	One or more intermediate sizes absent
"Uniformly graded"	Essentially single size material.

(c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

Angularity may be expressed as "rounded", "sub-rounded", "sub-angular" or "angular".

Particle form can be "equidimensional", "flat" or elongate".

Surface texture can be "glassy", "smooth", "rough", pitted" or striated".

(d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

Black	White	Grey	Red
Brown	Orange	Yellow	Green
Blue			

These may be modified as necessary by "light" or "dark". Borderline colours may be described as a combination of two colours, eg. red-brown.

For soils that contain more than one colour terms such as:

- Speckled Very small (<10 mm dia) patches
- Mottled Irregular
- Blotched Large irregular (>75 mm dia)
- Streaked Randomly oriented streaks

(e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

E1.3 Soil Condition

(a) Moisture

Soil moisture condition is described as "dry", "moist" or "wet".

The moisture categories are defined as:

Dry (D) - Little or no moisture evident. Soils are running. Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit. (b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E1.3.1.

TABLE	E1.3.1	-	CONSISTENCY	OF	FINE-GRAINED
		S	OILS		

TERM	UNCONFINED STRENGTH	FIELD IDENTIFICATION
Very Soft	(kPa) <25	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.
Firm	50 – 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength $(q_u = 2 c_u)$.

(c) Density Index

The insitu density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SPT N	STATIC	DENSITY
	VALUE	CONE	INDEX
		VALUE	(%)
		q _c (MPa)	
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8-25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

E1.4 Soil Structure

(a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample

Lens $\,$ - discontinuous with lenticular shape $\,$

Pocket - irregular inclusion

Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

(b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (infilled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded

E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

"Residual Soil" - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

"Colluvium" - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

"Landslide Debris" - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure. "Alluvium" - Material which has been transported essentially by water. Usually associated with former stream activity.

"Fill" - Material which has been transported and placed by man. This can range from natural soils which have been placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clays.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy an increase in volume due to shearing is indicted by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- feels gritty to the teeth

E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes "O" or "H" depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an "organic material" by classification.

Coal and lignite should be described as such and not simply as organic matter.