

Proposed Café / Bakery 44 to 55 Tench Avenue, Jamistown

Rezoning Traffic and Parking Assessment Report

Prepared for: MKT Café Pty Ltd

December 2019

Report No: PT1906501_Final

Document Set ID: 8967005 Version: 1, Version Date: 17/12/2019

TABLE OF CONTENTS

1.	Intr	roduction	3
2.	Exi	sting Development / Conditions	4
2	2.1	Site Location	4
2	2.2	Existing Site Traffic Generation	4
2	2.3	Classification Criteria	5
2	2.4	Existing Road Network	5
2	2.5	Existing Traffic Flows	6
2	2.6	Existing Intersection Operating Conditions	7
2	2.7	Public Transport - Buses	8
3.	The	e Proposed Development	10
4.	Pot	tential Traffic Impacts	11
Z	1.1	Introduction	11
۷	4.2.	Development Traffic Generation	
2	1.3	Trip Distribution / Future Traffic Flows	12
Z	1.4	Future Intersection Operation	13
5.	Pai	rking and Access Review	14
Ę	5.1	Council DCP Parking Provision	14
Ę	5.2	Car Park Classification	14
5	5.3	Car Park Compliance Assessment	14
Ę	5.4	Site Servicing	14
Ę	5.5	Bicycle Parking	15
6.	Со	nclusions	16
7.	Ар	pendix A – Intersection Counts	17
8.	Ар	pendix B – Plans of Proposed Scheme	18
9.	Ар	pendix C - SIDRA Outputs	19
10		· Appendix D - Service Vehicle Area Turning Path Analysis	

List of Figures

- Figure 1 Site Location
- Figure 2 Intersection Count Locations
- Figure 3 Walking Distance to Existing Bus Stops
- Figure 4 Assumed Trip Distribution of Generated Trips

List of Tables

- Table 1 Existing Weekday Peak Period Volumes in vicinity of site (veh/hr)
- Table 2 Level of Service Criteria
- Table 3 Existing Weekday AM / PM Intersection Operating Conditions
- Table 4 Existing Bus Services within Walking Distance to Development Site
- Table 5 Future Weekday Peak Period Volumes in vicinity of site (veh/hr)
- Table 6 Future Weekday AM / PM Intersection Operating Conditions

1. Introduction

This report has been prepared on behalf of MKT Café Pty Ltd to present findings of a traffic and parking assessment of the proposed café development at the site known as 44 to 55 Tench Avenue, Jamistown,

The study has assessed existing traffic conditions, parking demands, access arrangements, future traffic conditions and design compliance.

The remainder of the report is set out as follows:

- Section 2 describes the existing traffic and parking conditions;
- Section 3 summarises the proposed development;
- Section 4 reviews the potential traffic impacts of the proposal;
- Section 5 reviews the design for compliance with relevant standards; and
- Section 6 presents the conclusions

2. Existing Development / Conditions

The following presents a summary of existing site and traffic conditions.

2.1 Site Location

The development site includes a single dwelling residence with driveway access to / from Tench Avenue. The location of the development site is shown in Figure 1.

Figure 1 - Site Location



Source: Google maps

The site is located directly opposite the Nepean River with mainly rural residential dwellings surrounding the site. The Nepean Shores cabin park is located to the north of the development site also with direct access to Tench Avenue.

2.2 **Existing Site Traffic Generation**

As stated above, the existing includes a single dwelling house within a rural residential setting. Applying the RMS rate for a single dwelling house, it is expected the existing site would generate one (1) peak hour trip in each the AM and PM peak periods.

2.3 Classification Criteria

It is usual to classify roads according to a road hierarchy in order to determine their functional role within the road network. Changes to traffic flows on the roads can then be assessed within the context of the road hierarchy. Roads are classified according to the role they fulfil and the volume of traffic they should appropriately carry. The RTA has set down the following guidelines for the functional classification of roads.

- Arterial Road typically a main road carrying over 15,000 vehicles per day and fulfilling a role as a major inter-regional link (over 1,500 vehicles per hour)
- Sub-arterial Road defined as secondary inter-regional links, typically carrying volumes between 5,000 and 20,000 vehicles per day (500 to 2,000 vehicles per hour)
- Collector Road provides a link between local roads and regional roads, typically carrying between 2,000 and 10,000 vehicles per day (250 to 1,000 vehicles per hour). At volumes greater than 5,000 vehicles per day, residential amenity begins to decline noticeably.
- Local Road provides access to individual allotments, carrying low volumes, typically less than 2,000 vehicles per day (250 vehicles per hour).

2.4 Existing Road Network

<u>Tench Avenue</u>— is a local road forming a loop connection between Mulgoa Road (via an underpass under the M4 Motorway) including Factory Road, Bellevue Road and Jamison Road. The street across the frontage includes 3.25m wide travel lanes in each direction, 1.2m wide formed shoulders and 90-degree parking spaces along the western side to provide parking access to the Tench Reserve Park. The street includes a posted speed limit of 50km/hr with raised thresholds installed along the street between Jamison Road and Bellevue Road.

<u>Jamison Road</u> – is a local east-west road linking Mulgoa Road in the east with Tench Avenue in the west. It also provides vehicular access to Cables Aqua Park. The intersection of Mulgoa Road / Jamison Road is controlled by traffic signals. Near Tench Avenue, Jamison Road includes 3.25m wide travel lanes in each direction, 1.2m wide formed shoulders and a posted speed limit of 50km/hr.

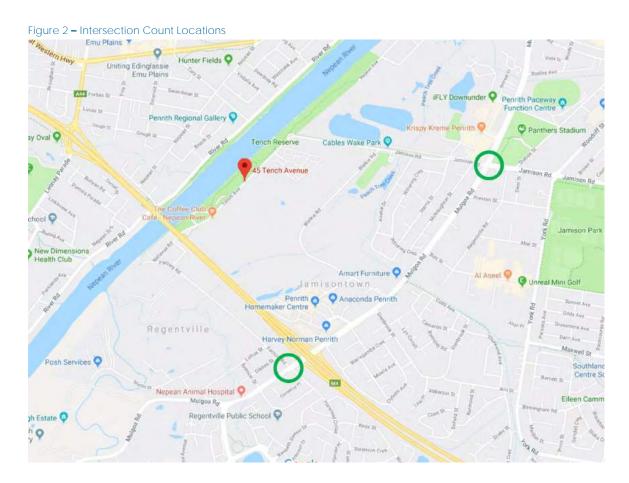
<u>Factory Road</u> – is a local east-west road linking Mulgoa Road in the east with Bellevue Road in the west. The intersection of Mulgoa Road / Factory Road is a priority-controlled intersection limited to left in / left out movements only. Near Tench Avenue, Factory Road includes a 7.0m wide carriageway and unformed shoulders and a posted speed limit of 50km/hr.

<u>Mulgoa Road</u> – Mulgoa Road is a State arterial road and a main access route from Penrith to the M4 Motorway. It generally includes 2 – 3 travel lanes in each direction, a posted speed limit of 60-70km/hr and carries large volumes of traffic during most periods of the day.

2.5 **Existing Traffic Flows**

To gauge existing traffic flows on the surrounding road network, intersection counts were undertaken at either end of the loop formed by Jamison Road / Tench Avenue, Bellevue Road and Factory Road with Mulgoa Road. As the proposal includes a café which can operate during the evening periods and on a weekend, counts were undertaken between the hours of 4:00pm -7:00pm on a weekday and 10:00am - 2:00pm on a Saturday.

The locations where AM and PM intersection counts were undertaken are shown in Figure 2.



Copies of the intersection count can be found in Appendix A of this report. Of note, Jamison Road provides access to the Cables Ski Aqua Park which in itself is a reasonable traffic generator. The peak flows by direction in each street at each intersection are summarised below.

Table 1 - Existing Weekday Peak Period Volumes in vicinity of site (veh/hr)

		Saturd	ay AM	Week	day PM
Road	Location	NB/EB	SB/WB	NB/EB	SB/WB
Mulgoa Road	North of Jamison Road	1,419	1,612	1,439	1,438
	South of Jamison Road	1,397	1,575	1,395	1,381
Jamison Road	West of Mulgoa Road	520	404	253	501
Mulgoa Road	North of Factory Road	1,240	1,277	1,291	1,852
	South of Factory Road	1,167	1,260	1,101	1,800
Factory Road	West of Mulgoa Road	114	41	234	45

From Table 1 it can be seen that existing flows on surrounding roads are in generally in line with their classification. It is noted that westbound flows on Jamison Road on a weekday were higher than expected for a local street. This is reflective of its role to provide access to Cable Ski Aqua Park, Nepean Shores Cabin Park and the Nepean Waterfront along with underpass access of the M4 Motorway.

2.6 Existing Intersection Operating Conditions

All intersections surveyed have been analysed using the Sidra Intersection analysis program. Sidra Intersection determines the average delay that vehicles encounter, the degree of saturation of the intersection, and the level of service. The degree of saturation is the ratio of the arrival rate of vehicles to the capacity of the approach. Sidra Intersection provides analysis of the operating conditions which can be compared to the performance criteria set out in Table 2.

Table 2 - Level of Service Criteria

Level of Service	Average Delay per Vehicle (secs/veh)	Signals & Roundabouts	Give Way & Stop Signs
Α	less than 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & Spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays Roundabouts require other control mode	At capacity, requires other control mode
F	> 70	Extra capacity required	Extreme delay, traffic signa or other major treatment required

Adapted from RTA Guide to Traffic Generating Developments, 2002.

For roundabouts and priority intersections, the reported average delay is for the individual movement with the highest average delay per vehicle. At signalised intersections, the reported average delay is over all movements.

The existing weekday and weekend day intersection operating conditions are presented in Table 3. Average delay is expressed in seconds per vehicle.

Table 3 – Existing Weekday AM / PM Intersection Operating Conditions

		Saturday A	M Peak	Weekday P	M Peak
Intersection	Control	Av Delay	LOS	Av Delay	LOS
Jamison Rd / Mulgoa Rd	Signals	62.4	Е	49.1	D
Factory Rd / Mulgoa Rd	Priority	13.2	Α	12.6	Α

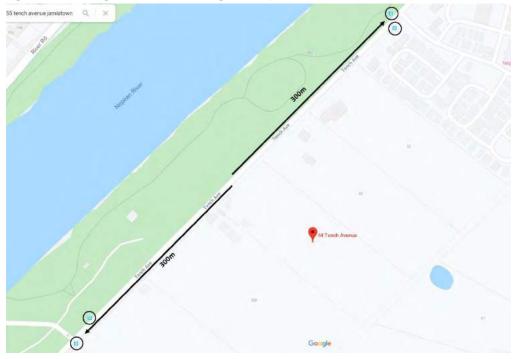
Avg Delay (sec/veh) is over all movements at signals, and for worst movement at priority and roundabouts

From Table 3 it is noted that all intersections in the vicinity of the development site currently operate within acceptable levels of service during both the weekend AM and weekday PM peak periods. However, the intersection of Mulgoa Road / Jamison Road is operating near capacity during the weekend AM peak period but functions with spare capacity during the weekday PM peak.

2.7 Public Transport - Buses

The development site is located some 300m walking distance from four (4) existing bus stops in Tench Avenue. The existing bus stops and walking distances to each is shown below in Figure 3.

Figure 3 - Walking Distance to Existing Bus Stops



The existing bus stops provide access to the following existing bus routes as summarised below in Table 4.

Table 4 – Existing Bus Services within Walking Distance to Development Site

Bus Route	Origin	Destination	Via
795	Penrith	Warragamba	Glenmore Park

From Table 4 it can be seen that the site is located within a convenient walking distance to an existing bus service to the regional centre of Penrith and the local centre of Mulgoa.

3. The Proposed Development

The key components of the proposed development are summarised below

- Alterations and additions to the existing dwelling to convert the building to a cafe with indoor and outdoor dining areas, bakery, take away coffee and amenities.
- Outdoor dining pavilion, wood fired pizza hut and farmers co-op pavilion.
- Water tanks.
- Demolish tin sheds.
- Seating pods and playground.
- Livestock enclosures.
- Car parking at the rear of the site behind orchard and disabled car parking near Tench Avenue.
- OSD basin and channel on southern boundary.
- · Loading dock in south west corner behind cafe.
- Landscaping including vegetable gardens and windmill.

The front building would achieve a café area of 35sqm with a seating area of 105sqm. A further 40sqm is provided in a decked area outside the building.

The rear building extension would achieve a total of 258sqm of kitchen / café / bakery area with an 80sqm courtyard central to each component of this building. A further 190sqm within a pavilion space for seating is provided adjacent to the rear building.

The development achieve a total of 109 on site parking spaces including two (2) accessible parking spaces. The main car park includes aisle widths no less than 7.44m serving adjacent parking spaces with a minimum width of 2.5m parking spaces provided.

The proposed driveway access to Tench Avenue would include a minimum width of 8.16m at the boundary of the site.

Servicing of the small café / bakery development is proposed via a separate loading dock which permits up to a Small Rigid Truck to enter / leave the site in a forward direction.

Waste servicing of the site would occur at the kerbside via Council's own waste vehicles for 240 litre bins.

Plans of a potential development scheme for the development can be found in Appendix B of this report.

4. Potential Traffic Impacts

4.1 Introduction

The following presents an assessment of the potential traffic impacts of the proposal using the Roads and Traffic Authority Guide to Traffic Generating Developments standard approach.

4.2 Development Traffic Generation

4.2.1 Retail Component

The proposed development includes uses which would be considered a café / restaurant type of use. In terms of floorspace which could be considered as gross floor area which would be attributed to traffic generation, the following areas would be considered in the gross floor area estimate:

- 35sqm café
- 105sqm bakery
- 110sqm kitchen
- 190sqm pavilion seating area
- 40sqm decking seating area
- 105sqm internal seating area
- Total: 630sqm GFA

The RTA Guide to Traffic Generating Developments defines a 'restaurant' as the following:

5.8.2 Restaurants.

Definition.

A *restaurant* is a refreshment room where food is served to customers. It can either be licensed or unlicensed. This definition includes cafes, tea rooms, eating houses, etc.

On the above basis the following rates would apply:

- Evening peak hour vehicle trips = 5 per 100 m² gross floor area.
- Daily vehicle trips = 60 per 100m² gross floor area.

Thus, a total of 630sqm of café / restaurant space (excluding the central courtyard area of 80sqm), it is estimated that the proposed development would generate in the order of 32 PM peak hour trips two – way.

For the AM weekday peak, it is expected that trips generated would generally be confined to staff trips. However, as a conservative estimate the PM peak hour traffic generation has also been applied to the weekday AM peak and the weekend AM peak.

Further, it is assumed trips generated would be 50/50 split between inbound and outbound movements.

4.3 Trip Distribution / Future Traffic Flows

Having regard to the traffic flows recorded on each potential route to / from the development site via Jamison Road and Factory Road, it has been assumed 75% of the traffic would travel to / from the site via Jamison Road with the remaining 25% travelling to the site via Factory Road but from the site via Jamison Road as right turn movements out of Factory Road at Mulgoa Road are not available. The assumed trip distribution is shown below.

Figure 4 - Assumed Trip Distribution of Generated Trips

Using the trip distribution shown above in Figure 4, the additional traffic generated by the proposed development has been added to the existing traffic flows on the surrounding road network. The resultant future two-way traffic flows are shown in Table 5.

Table 5 - Future Weekday Peak Period Volumes in vicinity of site (veh/hr)

		Saturda	ay AM	Week	day PM
Road	Location	NB/EB	SB/WB	NB/EB	SB/WB
Mulgoa Road	North of Jamison Road	1,436	1,629	1,456	1,455
	South of Jamison Road	1,397	1,579	1,395	1,383
Jamison Road	West of Mulgoa Road	539	415	274	512
Mulgoa Road	North of Factory Road	1,240	1,281	1,291	1,856
	South of Factory Road	1,171	1,264	1,105	1,804
Factory Road	West of Mulgoa Road	114	45	234	49

From Table 5 it can be seen that future traffic flows would be similar to that which currently occur and would remain generally within their current expected environmental maximum for each classification of surrounding streets.

4.4 Future Intersection Operation

The future traffic flows on the surrounding road network have been assessed in SIDRA. The resulting future intersection operation is presented below.

Table 6 – Future Weekday AM / PM Intersection Operating Conditions

		Saturday A	M Peak	Weekday P	M Peak
Intersection	Control	Av Delay	LOS	Av Delay	LOS
Jamison Rd / Mulgoa Rd	Signals	64.2	Е	50.6	D
Factory Rd / Mulgoa Rd	Priority	13.3	А	12.7	А

Avg Delay (sec/veh) is over all movements at signals, and for worst movement at priority and roundabouts

From Table 6 it is noted that all intersections surveyed in the vicinity of the development site would continue to operate at the same or with little change to existing level of service conditions in both peak periods at each location following the additional traffic generated by full development of the subject site. This assumes a conservative analysis of peak hour traffic generation on a Thursday evening applied to a Saturday AM peak period.

Having regard to the analysis presented above, it is concluded that the potential traffic impacts of the proposal are considered satisfactory.

Copies of the SIDRA outputs are provided in Appendix C of this report.

5. Parking and Access Review

5.1 Council DCP Parking Provision

The Penrith Development Control Plan 2014 (Penrith DCP 2014) does not provide any parking rates for a café. However, the DCP does include a rate of 1 space per 6sqm of seating area for restaurants.

Therefore, with a total seating area of 145sqm for the café and 190sqm for the rear building, the development would require a minimum of 56 on-site parking spaces to comply with the DCP.

As the proposed design for the car park includes a total of 109 parking spaces, the on-site parking provision exceeds the minimum requirements of the DCP and is considered satisfactory.

5.2 Car Park Classification

Given the nature of the development, it is not considered that this development would function as a shopping centre with many patrons spending some time at the development having meals or buying speciality food.

Thus, for the purpose of classification of the development in accordance with the Australian Standard for Off Street Car Parking Facilities, this development is considered a Class 2 development.

Thus, parking spaces should be provided at a minimum width of 2.5m with a minimum adjacent aisle width of 5.8m.

5.3 Car Park Compliance Assessment

All areas of the proposed car park have been reviewed for compliance with the Australian Standard for Off Street Car Parking Facilities – AS2890.1 and were found to be satisfactory.

All parking spaces and adjacent aisle width comply with or exceed the minimum requirements of a Class 2 development as detailed in AS2890.1.

Thus, overall the design of the car park is considered satisfactory.

5.4 Site Servicing

As stated above, the waste collection of the site would occur at the kerbside by Council waste vehicles. Further, the design of the site has included a separate entry / exit driveway to provide access to a service vehicle parking area.

The nature of the development is such that provision of goods for sale / food stuffs would occur via vans and up to a Small Rigid Truck.

A turning path assessment of the service vehicle area using the largest expected vehicle to access the area, a small rigid truck, has been undertaken using AutoTURN. These turn paths are provided in Appendix D of this report.

The turn path assessment provided in Appendix D of this report confirm the largest expected vehicle to access the service vehicle area can enter and exit the site in a forward direction. Thus, the provision for service vehicles is considered satisfactory.

5.5 Bicycle Parking

As stated above, the proposed development includes twenty (20) bicycle parking spaces.

On the matter of bicycle parking provision, the following is noted from Councils DCP:

Bicycle parking in accordance with the suggested bicycle parking provision rates for different land use types in the document 'Planning Guidelines for Walking and Cycling' (NSW Government 2004). Bicycle parking spaces should comply with AS2890.3:1993 Bicycle Parking Facilities.

Of note, AS2890.3 now includes a 2015 version which has replaced the 1993 version detailed in Council's DCP.

As noted in Table 1.1 of AS2890.3 – 2015, short term bicycle parking for retail developments require a 'Class 3' provision in the form of bicycle racks. The proposed development complies with this level of security.

Also of note, AS2890.3 -2015 does not include specific bicycle parking rates for a development.

As a comparison, Austroads Bicycle Parking Facilities: Updating the Austroads Guide to Traffic Management does include such rates. Table 5.1 of this guide suggests the following bicycle parking provision rates:

Land House	Box constant	Peak Population	10% Mo	ode Share Rate
Land Use	Description	Density	Short-Stay 1	Long-Stay 1
Retail	Bulky Goods Retail	27sqm NFA per person ⁴	0.30 spaces per 100sgm NFA	0.07 spaces per 100sq NFA

Therefore, the proposed development which is expected to include short term visiting of the site would require approximately two (2) short stay spaces. As the development includes a total of 20 bicycle parking spaces, the provision well exceeds the requirements of the Austroads Guide and is considered satisfactory.

6. Conclusions

This report has reviewed the potential traffic impacts of the proposed café / restaurant development at the site known as No.44 – 55 Tench Avenue, Jamistown. The findings of this assessment are presented below:

- 1. The potential traffic generation of the development would not impact on the surrounding road network to a point of detriment.
- 2. Intersections in the immediate vicinity of the development would operate at a satisfactory level of service in the future at full development.
- 3. The development is located within a short walking distance to a number of existing bus stops and services to surrounding regional centres.
- 4. The proposed on-site parking provision exceeds the minimum requirements of Council's DCP and is considered satisfactory.
- 5. The bicycle parking provision exceeds the minimum requirements of Austroads Bicycle Parking Facilities: Updating the Austroads Guide to Traffic Management.
- 6. The proposed service vehicle area enables the expected largest service vehicle to enter and exit the site in a forward direction and is considered satisfactory.

Overall the traffic impacts of the proposal are considered acceptable.

7. Appendix A – Intersection Counts

Client : Positive Traffic Pty Ltd

Suburb : Penrith Surveys

Location : 1. Jamison Rd / Mulgoa Rd

Day/Date : Thu, 19th Sep 2019

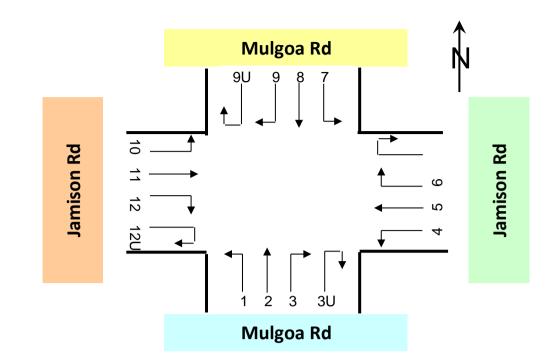
Weather : Fine

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2 Class 3

Classifications Cars Trucks Buses





Approach								Mulg	oa Rd															Jamis	on Rd							
Direction			tion 1 Turn)				tion 2 ough)			Direct (Right				Directi (U Tı				Direc (Left	tion 4 Turn)				ction 5 ough)			Direc (Right	tion 6 Turn)				ion 6U urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 16:15	5	0	0	5	203	12	0	215	76	1	2	79	0	0	0	0	96	5	1	102	61	1	0	62	59	1	1	61	0	0	0	0
16:15 to 16:30	6	0	0	6	194	10	0	204	89	2	2	93	0	0	0	0	105	1	2	108	59	0	0	59	48	6	1	55	0	0	0	0
16:30 to 16:45	4	0	0	4	260	15	5	280	80	1	2	83	0	0	0	0	70	1	2	73	50	0	0	50	57	6	3	66	0	0	0	0
16:45 to 17:00	6	0	0	6	234	9	3	246	89	1	0	90	0	0	0	0	95	2	2	99	55	0	0	55	65	3	1	69	0	0	0	0
17:00 to 17:15	6	0	0	6	242	8	1	251	88	1	2	91	0	0	0	0	92	3	0	95	74	0	0	74	74	6	0	80	0	0	0	0
17:15 to 17:30	3	0	0	3	236	9	0	245	88	1	1	90	0	0	0	0	94	0	1	95	75	1	0	76	89	4	2	95	0	0	0	0
17:30 to 17:45	5	0	0	5	215	6	0	221	76	0	2	78	0	0	0	0	107	1	1	109	88	0	0	88	62	3	1	66	0	0	0	0
17:45 to 18:00	5	2	0	7	273	8	0	281	85	0	1	86	0	0	0	0	87	0	1	88	52	0	0	52	85	2	1	88	0	0	0	0
18:00 to 18:15	7	0	0	7	215	7	0	222	82	0	1	83	0	0	0	0	99	1	2	102	37	1	0	38	91	2	1	94	0	0	0	0
18:15 to 18:30	3	0	0	3	240	5	0	245	87	1	2	90	0	0	0	0	95	1	1	97	39	1	0	40	62	2	0	64	0	0	0	0
18:30 to 18:45	2	0	0	2	210	1	0	211	89	0	0	89	0	0	0	0	91	0	1	92	34	0	0	34	52	1	1	54	0	0	0	0
18:45 to 19:00	4	0	0	4	199	2	0	201	73	1	2	76	0	0	0	0	97	0	1	98	25	0	0	25	56	3	0	59	0	0	0	0
Total	56	2	0	58	2,721	92	9	2,822	1,002	9	17	1,028	0	0	0	0	1,128	15	15	1,158	649	4	0	653	800	39	12	851	0	0	0	0

Approach								Mulg	oa Rd															Jamis	on Rd							
Direction		Direct (Left				Direc				Direc (Right	tion 9 :Turn)			Direct (U T				Direct (Left	ion 10 Turn)			Direct (Thro	ion 11 ough)			Directi (Right					ion 12U Turn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 16:15	69	2	0	71	207	11	0	218	37	1	0	38	0	0	0	0	24	1	0	25	25	0	0	25	6	0	0	6	0	0	0	0
16:15 to 16:30	89	5	0	94	160	1	0	161	82	1	0	83	0	0	0	0	23	0	0	23	26	1	0	27	5	2	0	7	0	0	0	0
16:30 to 16:45	62	1	0	63	263	9	0	272	63	2	0	65	0	0	0	0	27	2	0	29	29	1	0	30	4	0	0	4	0	0	0	0
16:45 to 17:00	61	2	0	63	235	7	0	242	62	1	0	63	0	0	0	0	24	0	0	24	24	1	0	25	10	3	0	13	0	0	0	0
17:00 to 17:15	45	1	0	46	240	4	0	244	38	1	0	39	0	0	0	0	24	0	0	24	29	0	0	29	10	0	0	10	0	0	0	0
17:15 to 17:30	61	1	0	62	216	3	0	219	60	0	0	60	0	0	0	0	29	1	0	30	20	0	0	20	14	1	0	15	0	0	0	0
17:30 to 17:45	60	1	0	61	240	7	0	247	28	0	0	28	0	0	0	0	25	0	0	25	32	0	0	32	6	1	0	7	0	0	0	0
17:45 to 18:00	46	3	0	49	251	4	0	255	39	0	0	39	0	0	0	0	24	0	0	24	16	0	0	16	5	0	0	5	0	0	0	0
18:00 to 18:15	52	1	0	53	230	5	0	235	42	0	0	42	0	0	0	0	26	0	0	26	27	0	0	27	7	0	0	7	0	0	0	0
18:15 to 18:30	71	0	0	71	255	5	0	260	41	0	0	41	0	0	0	0	8	0	0	8	20	1	0	21	8	0	0	8	0	0	0	0
18:30 to 18:45	41	1	0	42	260	7	0	267	19	0	0	19	0	0	0	0	18	0	0	18	15	0	0	15	11	0	0	11	0	0	0	0
18:45 to 19:00	46	1	0	47	243	5	0	248	24	0	0	24	0	0	0	0	13	0	0	13	18	0	0	18	7	0	0	7	0	0	0	0
Total	703	19	0	722	2,800	68	0	2,868	535	6	0	541	0	0	0	0	265	4	0	269	281	4	0	285	93	7	0	100	0	0	0	0

Mulgoa Rd



Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

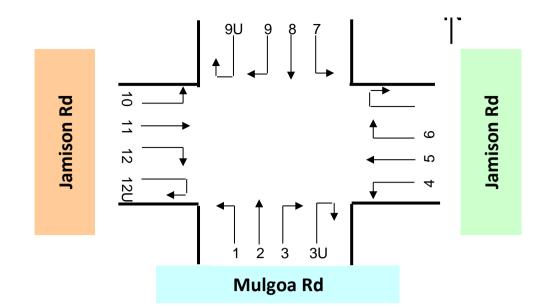
Location : 1. Jamison Rd / Mulgoa Rd

Day/Date : Thu, 19th Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Hourly Summary





Approach					Mulg	oa Rd															Jamis	on Rd										
Direction		Direct (Left ⁻				Direct (Thro					tion 3 Turn)			Direct (U T	ion 3U urn)				tion 4 Turn)			Directi (Thro				Direct (Right					ion 6U Turn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 17:00	21	0	0	21	891	46	8	945	334	5	6	345	0	0	0	0	366	9	7	382	225	1	0	226	229	16	6	251	0	0	0	0
16:15 to 17:15	22	0	0	22	930	42	9	981	346	5	6	357	0	0	0	0	362	7	6	375	238	0	0	238	244	21	5	270	0	0	0	0
16:30 to 17:30	19	0	0	19	972	41	9	1,022	345	4	5	354	0	0	0	0	351	6	5	362	254	1	0	255	285	19	6	310	0	0	0	0
16:45 to 17:45	20	0	0	20	927	32	4	963	341	3	5	349	0	0	0	0	388	6	4	398	292	1	0	293	290	16	4	310	0	0	0	0
17:00 to 18:00	19	2	0	21	966	31	1	998	337	2	6	345	0	0	0	0	380	4	3	387	289	1	0	290	310	15	4	329	0	0	0	0
17:15 to 18:15	20	2	0	22	939	30	0	969	331	1	5	337	0	0	0	0	387	2	5	394	252	2	0	254	327	11	5	343	0	0	0	О
17:30 to 18:30	20	2	0	22	943	26	0	969	330	1	6	337	0	0	0	0	388	3	5	396	216	2	0	218	300	9	3	312	0	0	0	0
17:45 to 18:45	17	2	0	19	938	21	0	959	343	1	4	348	0	0	0	0	372	2	5	379	162	2	0	164	290	7	3	300	0	0	0	0
18:00 to 19:00	16	0	0	16	864	15	0	879	331	2	5	338	0	0	0	0	382	2	5	389	135	2	0	137	261	8	2	271	0	0	0	О
Total	56	2	0	58	2,721	92	9	2,822	1,002	9	17	1,028	0	0	0	0	1,128	15	15	1,158	649	4	0	653	800	39	12	851	0	0	0	0

Approach								Mulg	oa Rd															Jamis	on Rd							
Direction		Direct (Left ⁻				Direct (Thro					tion 9 :Turn)				ion 9U urn)				ion 10 Turn)			Direction				Direct (Right					ion 12U Turn)	
Time Period	S L				Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 17:00	281	10	0	291	865	28	0	893	244	5	0	249	0	0	0	0	98	3	0	101	104	3	0	107	25	5	0	30	0	0	0	0
16:15 to 17:15	257	9	0	266	898	21	0	919	245	5	0	250	0	0	0	0	98	2	0	100	108	3	0	111	29	5	0	34	0	0	0	0
16:30 to 17:30	229	5	0	234	954	23	0	977	223	4	0	227	0	0	0	0	104	3	0	107	102	2	0	104	38	4	0	42	0	0	0	0
16:45 to 17:45	227	5	0	232	931	21	0	952	188	2	0	190	0	0	0	0	102	1	0	103	105	1	0	106	40	5	0	45	0	0	0	0
17:00 to 18:00	212	6	0	218	947	18	0	965	165	1	0	166	0	0	0	0	102	1	0	103	97	0	0	97	35	2	0	37	0	0	0	0
17:15 to 18:15	219	6	0	225	937	19	0	956	169	0	0	169	0	0	0	0	104	1	0	105	95	0	0	95	32	2	0	34	0	0	0	0
17:30 to 18:30	229	5	0	234	976	21	0	997	150	0	0	150	0	0	0	0	83	0	0	83	95	1	0	96	26	1	0	27	0	0	0	0
17:45 to 18:45	210	5	0	215	996	21	0	1,017	141	0	0	141	0	0	0	0	76	0	0	76	78	1	0	79	31	0	0	31	0	0	0	0
18:00 to 19:00	210	3	0	213	988	22	0	1,010	126	0	0	126	0	0	0	0	65	0	0	65	80	1	0	81	33	0	0	33	0	0	0	0
Total	703	19	0	722	2,800	68	0	2,868	535	6	0	541	0	0	0	0	265	4	0	269	281	4	0	285	93	7	0	100	0	0	0	0

Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

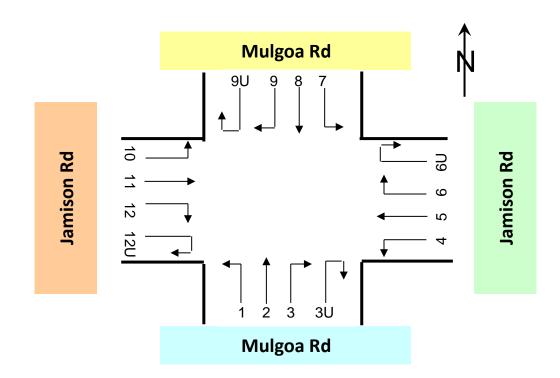
Location : 1. Jamison Rd / Mulgoa Rd

Day/Date : Thu, 19th Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Peak Hour Summary





Approach		Mulg	oa Rd			Jamis	on Rd			Mulg	oa Rd			Jamis	on Rd		otal
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand 1
16:30 to 17:30	1,336	45	14	1,395	890	26	11	927	1,406	32	0	1,438	244	9	0	253	4,013

Ар	proa	ach		Mulg	oa Rd			Jamis	on Rd			Mulg	oa Rd			Jamis	on Rd		otal
Tim	ie Pe	riod	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand Total
16:00	to	17:00	1,246	51	14	1,311	820	26	13	859	1,390	43	0	1,433	227	11	0	238	3,841
16:15	to	17:15	1,298	47	15	1,360	844	28	11	883	1,400	35	0	1,435	235	10	0	245	3,923
16:30	to	17:30	1,336	45	14	1,395	890	26	11	927	1,406	32	0	1,438	244	9	0	253	4,013
16:45	to	17:45	1,288	35	9	1,332	970	23	8	1,001	1,346	28	0	1,374	247	7	0	254	3,961
17:00	to	18:00	1,322	35	7	1,364	979	20	7	1,006	1,324	25	0	1,349	234	3	0	237	3,956
17:15	to	18:15	1,290	33	5	1,328	966	15	10	991	1,325	25	0	1,350	231	3	0	234	3,903
17:30	to	18:30	1,293	29	6	1,328	904	14	8	926	1,355	26	0	1,381	204	2	0	206	3,841
17:45	to	18:45	1,298	24	4	1,326	824	11	8	843	1,347	26	0	1,373	185	1	0	186	3,728
18:00	to	19:00	1,211	17	5	1,233	778	12	7	797	1,324	25	0	1,349	178	1	0	179	3,558
	Tota	I	3,779	103	26	3,908	2,577	58	27	2,662	4,038	93	0	4,131	639	15	0	654	11,355

Client : Positive Traffic Pty Ltd

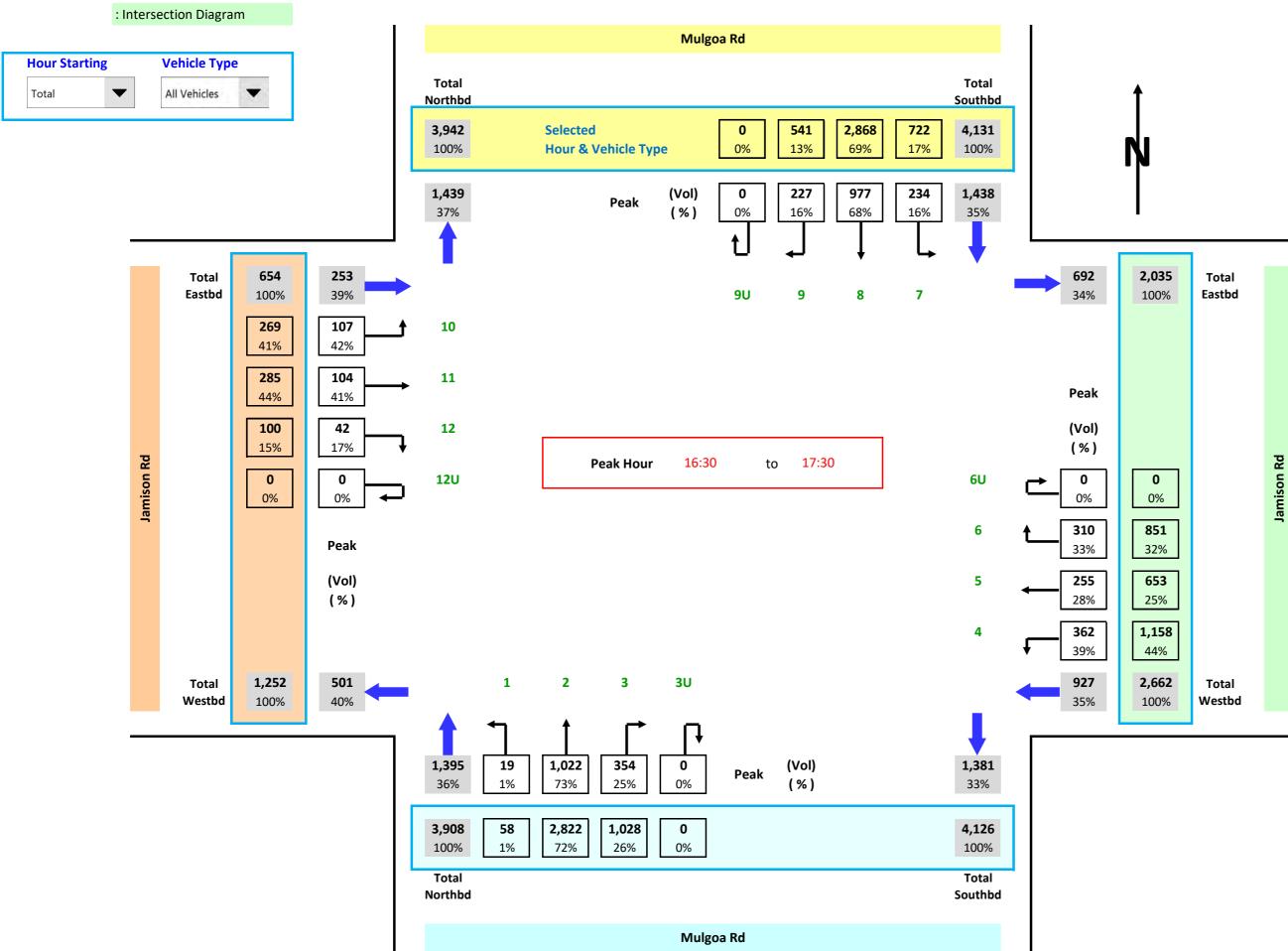
Suburb : Penrith Surveys

Location : 1. Jamison Rd / Mulgoa Rd

Day/Date: Thu, 19th Sep 2019

Weather : Fine

Description : Classified Intersection Count





Client : Positive Traffic Pty Ltd

Suburb : Penrith Surveys

Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date

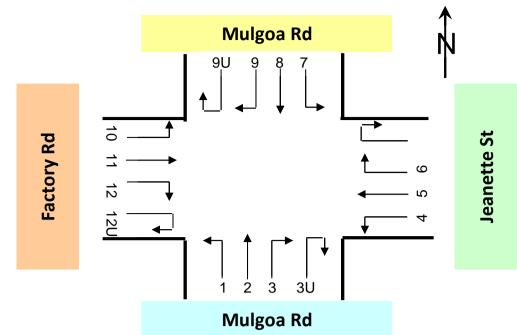
: Thu, 19th Sep 2019

Weather : Fi

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2 Class 3
Classifications Cars Trucks Buses





Approach								Mulg	oa Rd															Jean	ette St							
Direction		Direct (Left 1					tion 2 ough)			Direc (Right					ion 3U urn)			Direc (Left				Direc (Thro	tion 5 ough)				tion 6 Turn)			Directi (U T		
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 16:15	23	0	0	23	238	7	4	249	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:15 to 16:30	9	0	0	9	239	5	7	251	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
16:30 to 16:45	11	0	0	11	217	9	3	229	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0
16:45 to 17:00	5	0	0	5	261	11	4	276	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0
17:00 to 17:15	10	0	0	10	246	8	0	254	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0
17:15 to 17:30	10	0	0	10	272	3	4	279	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
17:30 to 17:45	15	0	0	15	250	9	1	260	0	0	0	0	0	0	0	0	11	0	0	11	0	0	0	0	0	0	0	0	0	0	0	0
17:45 to 18:00	9	0	0	9	260	3	1	264	0	0	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0
18:00 to 18:15	5	0	0	5	221	4	3	228	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
18:15 to 18:30	9	0	0	9	209	2	1	212	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0
18:30 to 18:45	2	0	0	2	189	2	2	193	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:45 to 19:00	9	0	0	9	198	3	1	202	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
Total	117	0	0	117	2,800	66	31	2,897	0	0	0	0	0	0	0	0	31	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0

Approach								Mulg	oa Rd															Facto	ory Rd							
Direction			tion 7 Turn)				tion 8 ough)				ction 9 t Turn)				ion 9U Turn)				tion 10 t Turn)				tion 11 ough)				ion 12 Turn)				ion 12U Γurn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 16:15	5	1	1	7	420	18	0	438	2	0	0	2	0	0	0	0	38	0	0	38	0	0	0	0	0	0	0	0	0	0	0	0
16:15 to 16:30	14	0	0	14	477	6	0	483	0	0	0	0	0	0	0	0	31	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0
16:30 to 16:45	10	1	2	13	373	10	2	385	0	0	0	0	0	0	0	0	25	0	0	25	0	0	0	0	0	0	0	0	0	0	0	0
16:45 to 17:00	20	1	1	22	419	18	1	438	1	0	0	1	0	0	0	0	53	1	0	54	0	0	0	0	0	0	0	0	0	0	0	0
17:00 to 17:15	15	0	1	16	431	3	0	434	0	0	0	0	0	0	0	0	48	0	0	48	0	0	0	0	0	0	0	0	0	0	0	0
17:15 to 17:30	26	2	0	28	471	4	0	475	0	0	0	0	0	0	0	0	71	2	0	73	0	0	0	0	0	0	0	0	0	0	0	0
17:30 to 17:45	8	1	0	9	412	3	1	416	1	0	0	1	0	0	0	0	62	0	0	62	0	0	0	0	0	0	0	0	0	0	0	0
17:45 to 18:00	15	0	2	17	448	6	2	456	0	0	0	0	0	0	0	0	51	0	0	51	0	0	0	0	0	0	0	0	0	0	0	0
18:00 to 18:15	6	0	1	7	416	9	1	426	0	0	0	0	0	0	0	0	21	0	0	21	0	0	0	0	0	0	0	0	0	0	0	0
18:15 to 18:30	10	0	0	10	409	10	1	420	0	0	0	0	0	0	0	0	30	0	0	30	0	0	0	0	0	0	0	0	0	0	0	0
18:30 to 18:45	14	0	1	15	349	6	0	355	0	0	0	0	0	0	0	0	18	0	0	18	0	0	0	0	0	0	0	0	0	0	0	0
18:45 to 19:00	23	1	0	24	287	4	1	292	0	0	0	0	0	0	0	0	15	0	0	15	0	0	0	0	0	0	0	0	0	0	0	0
Total	166	7	9	182	4,912	97	9	5,018	4	0	0	4	0	0	0	0	463	3	0	466	0	0	0	0	0	0	0	0	0	0	0	0

Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

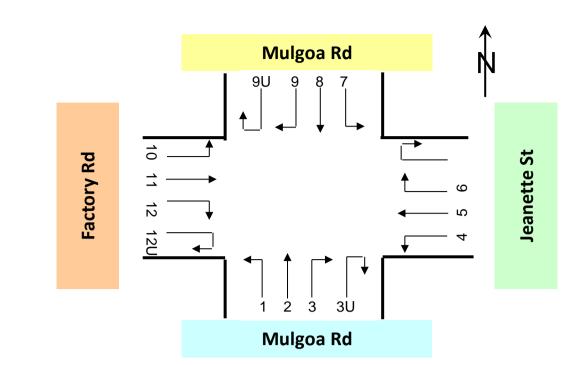
Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date : Thu, 19th Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Hourly Summary





Approach								Mulg	oa Rd															Jeane	ette St							
Direction		Direc (Left	tion 1 Turn)				ction 2 rough)				tion 3 Turn)				ion 3U urn)			Direc (Left	tion 4 Turn)			Direc (Thro	tion 5 ough)			Direc (Right	tion 6 Turn)			Direct (U T	ion 6U urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 17:00	48	0	0	48	955	32	18	1,005	0	0	0	0	0	0	0	0	7	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0
16:15 to 17:15	35	0	0	35	963	33	14	1,010	0	0	0	0	0	0	0	0	9	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0
16:30 to 17:30	36	0	0	36	996	31	11	1,038	0	0	0	0	0	0	0	0	9	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0
16:45 to 17:45	40	0	0	40	1,029	31	9	1,069	0	0	0	0	0	0	0	0	17	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0
17:00 to 18:00	44	0	0	44	1,028	23	6	1,057	0	0	0	0	0	0	0	0	19	0	0	19	0	0	0	0	0	0	0	0	0	0	0	0
17:15 to 18:15	39	0	0	39	1,003	19	9	1,031	0	0	0	0	0	0	0	0	18	0	0	18	0	0	0	0	0	0	0	0	0	0	0	0
17:30 to 18:30	38	0	0	38	940	18	6	964	0	0	0	0	0	0	0	0	20	0	0	20	0	0	0	0	0	0	0	0	0	0	0	0
17:45 to 18:45	25	0	0	25	879	11	7	897	0	0	0	0	0	0	0	0	9	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0
18:00 to 19:00	25	0	0	25	817	11	7	835	0	0	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0
Total	117	0	0	117	2,800	66	31	2,897	0	0	0	0	0	0	0	0	31	0	0	31	0	0	0	0	0	0	0	0	0	0	0	0

Approach								Mulg	oa Rd															Facto	ry Rd							
Direction			tion 7 Turn)			Direc	tion 8 ough)			Direc (Right	tion 9 Turn)				ion 9U urn)			Direct (Left	ion 10 Turn)			Direct (Thro				Direct (Right	ion 12 Turn)			Directi (U T	ion 12U Turn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
16:00 to 17:00	49	3	4	56	1,689	52	3	1,744	3	0	0	3	0	0	0	0	147	1	0	148	0	0	0	0	0	0	0	0	0	0	0	0
16:15 to 17:15	59	2	4	65	1,700	37	3	1,740	1	0	0	1	0	0	0	0	157	1	0	158	0	0	0	0	0	0	0	0	0	0	0	0
16:30 to 17:30	71	4	4	79	1,694	35	3	1,732	1	0	0	1	0	0	0	0	197	3	0	200	0	0	0	0	0	0	0	0	0	0	0	0
16:45 to 17:45	69	4	2	75	1,733	28	2	1,763	2	0	0	2	0	0	0	0	234	3	0	237	0	0	0	0	0	0	0	0	0	0	0	0
17:00 to 18:00	64	3	3	70	1,762	16	3	1,781	1	0	0	1	0	0	0	0	232	2	0	234	0	0	0	0	0	0	0	0	0	0	0	0
17:15 to 18:15	55	3	3	61	1,747	22	4	1,773	1	0	0	1	0	0	0	0	205	2	0	207	0	0	0	0	0	0	0	0	0	0	0	0
17:30 to 18:30	39	1	3	43	1,685	28	5	1,718	1	0	0	1	0	0	0	0	164	0	0	164	0	0	0	0	0	0	0	0	0	0	0	0
17:45 to 18:45	45	0	4	49	1,622	31	4	1,657	0	0	0	0	0	0	0	0	120	0	0	120	0	0	0	0	0	0	0	0	0	0	0	0
18:00 to 19:00	53	1	2	56	1,461	29	3	1,493	0	0	0	0	0	0	0	0	84	0	0	84	0	0	0	0	0	0	0	0	0	0	0	0
Total	166	7	9	182	4,912	97	9	5,018	4	0	0	4	0	0	0	0	463	3	0	466	0	0	0	0	0	0	0	0	0	0	0	0

Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

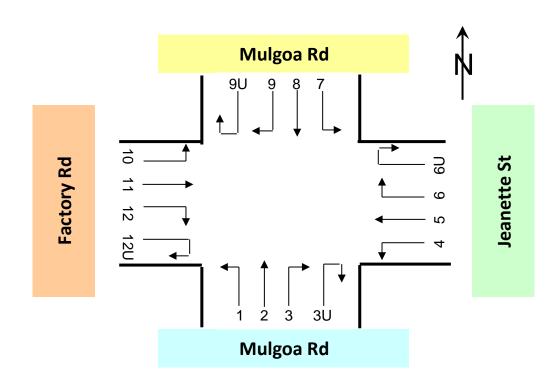
Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date : Thu, 19th Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Peak Hour Summary





Approach		Mulg	oa Rd			Jeane	tte St			Mulg	oa Rd			Facto	ry Rd		otal
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand 1
17:00 to 18:00	1,072	23	6	1,101	19	0	0	19	1,827	19	6	1,852	232	2	0	234	3,206

Ар	proa	ich		Mulg	oa Rd			Jeane	ette St			Mulg	oa Rd			Facto	ry Rd		otal
Tim	ie Pei	riod	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand Total
16:00	to	17:00	1,003	32	18	1,053	7	0	0	7	1,741	55	7	1,803	147	1	0	148	3,011
16:15	to	17:15	998	33	14	1,045	9	0	0	9	1,760	39	7	1,806	157	1	0	158	3,018
16:30	to	17:30	1,032	31	11	1,074	9	0	0	9	1,766	39	7	1,812	197	3	0	200	3,095
16:45	to	17:45	1,069	31	9	1,109	17	0	0	17	1,804	32	4	1,840	234	3	0	237	3,203
17:00	to	18:00	1,072	23	6	1,101	19	0	0	19	1,827	19	6	1,852	232	2	0	234	3,206
17:15	to	18:15	1,042	19	9	1,070	18	0	0	18	1,803	25	7	1,835	205	2	0	207	3,130
17:30	to	18:30	978	18	6	1,002	20	0	0	20	1,725	29	8	1,762	164	0	0	164	2,948
17:45	to	18:45	904	11	7	922	9	0	0	9	1,667	31	8	1,706	120	0	0	120	2,757
18:00	to	19:00	842	11	7	860	5	0	0	5	1,514	30	5	1,549	84	0	0	84	2,498
	Tota	I	2,917	66	31	3,014	31	0	0	31	5,082	104	18	5,204	463	3	0	466	8,715

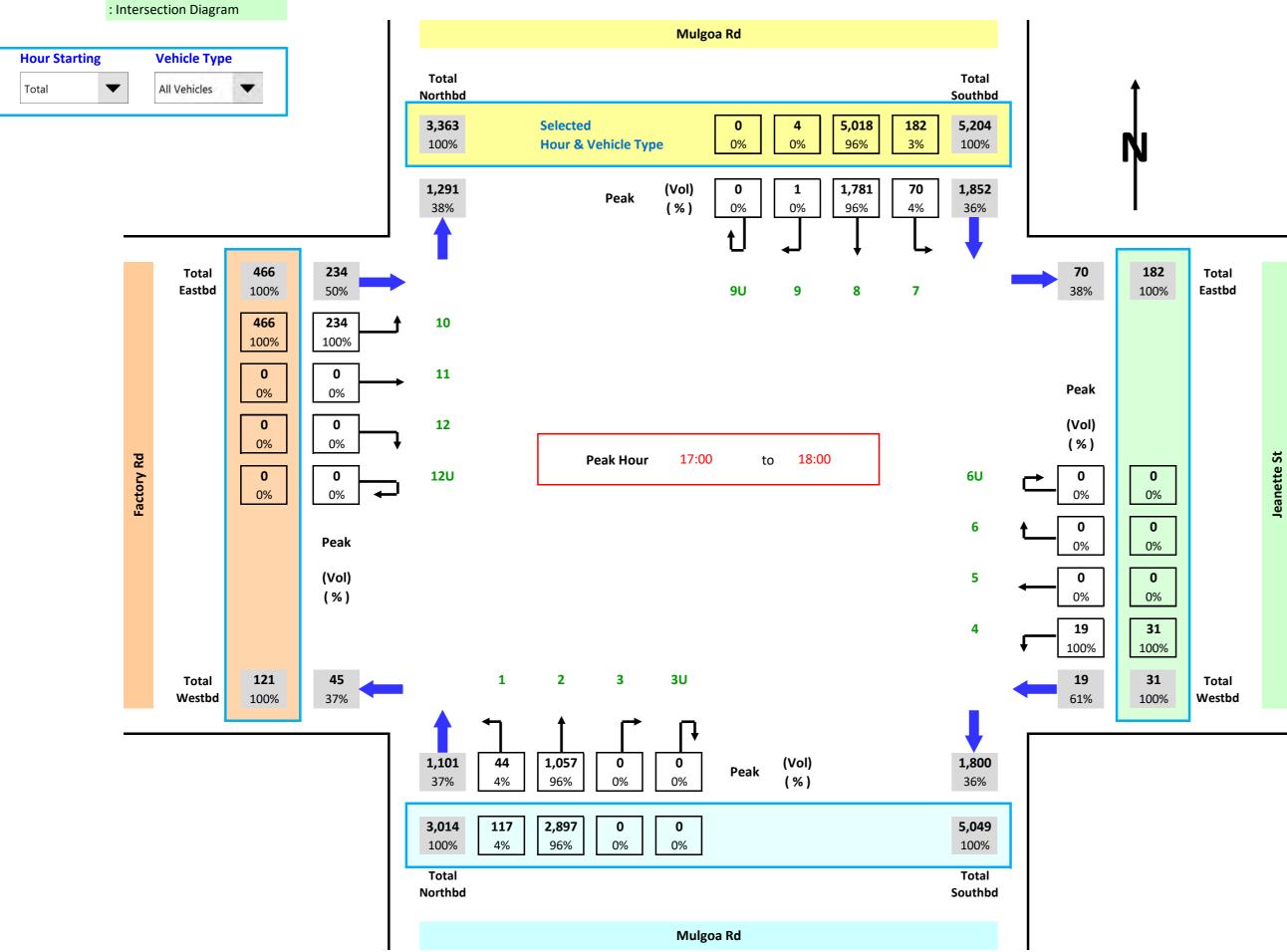
Client : Positive Traffic Pty Ltd Suburb : Penrith Surveys

: 2. Mulgoa Rd & Factory Rd & Jeanette St Location

Day/Date : Thu, 19th Sep 2019

Weather : Fine

Description : Classified Intersection Count





Client : Positive Traffic Pty Ltd

Suburb : Penrith Surveys

Location : 1. Jamison Rd / Mulgoa Rd

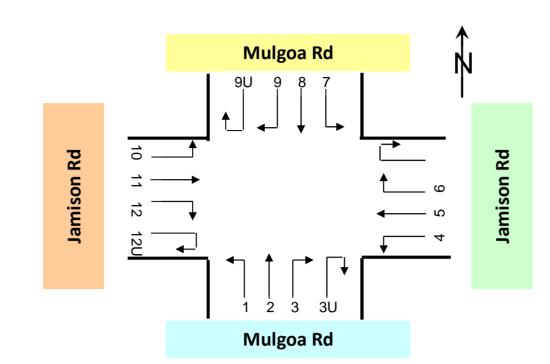
Day/Date : Sat, 21st Sep 2019

Weather : Fir

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2 Class 3
Classifications Cars Trucks Buses





Approach								Mulg	oa Rd															Jamis	on Rd							
Direction	(Left Turn)						ction 2 ough)			Direct (Right				Direct (U T				Direc (Left				Direc (Thre	tion 5 ough)			Direct (Right				Directi (U T		
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 10:15	16	0	1	17	229	7	1	237	86	0	1	87	0	0	0	0	102	3	0	105	50	1	0	51	67	5	2	74	0	0	0	0
10:15 to 10:30	4	0	0	4	239	6	1	246	80	0	1	81	0	0	0	0	115	1	1	117	44	2	0	46	84	4	0	88	0	0	0	0
10:30 to 10:45	13	0	0	13	247	6	0	253	96	0	0	96	0	0	0	0	90	0	1	91	39	0	0	39	46	4	2	52	0	0	0	0
10:45 to 11:00	11	0	0	11	208	7	0	215	101	0	0	101	0	0	0	0	88	1	0	89	56	0	0	56	66	2	1	69	0	0	0	0
11:00 to 11:15	14	0	0	14	255	7	0	262	97	1	1	99	0	0	0	0	100	3	0	103	80	0	0	80	51	0	1	52	0	0	0	0
11:15 to 11:30	13	0	0	13	217	10	0	227	105	1	1	107	0	0	0	0	119	4	0	123	83	0	1	84	66	3	0	69	0	0	0	0
11:30 to 11:45	23	0	0	23	280	2	0	282	114	1	0	115	0	0	0	0	104	1	1	106	57	1	0	58	63	1	2	66	0	0	0	0
11:45 to 12:00	9	0	0	9	220	5	0	225	85	1	0	86	0	0	0	0	108	3	1	112	61	1	0	62	83	1	0	84	0	0	0	0
12:00 to 12:15	22	0	0	22	252	8	1	261	97	0	1	98	0	0	0	0	110	3	0	113	66	3	0	69	77	0	0	77	0	0	0	0
12:15 to 12:30	21	0	1	22	254	4	0	258	84	1	2	87	0	0	0	0	105	4	1	110	62	2	0	64	75	3	1	79	0	0	0	0
12:30 to 12:45	10	0	0	10	223	5	0	228	111	0	0	111	0	0	0	0	108	2	2	112	57	2	0	59	78	2	1	81	0	0	0	0
12:45 to 13:00	23	1	0	24	246	8	0	254	88	1	0	89	0	0	0	0	103	2	0	105	55	0	0	55	101	1	2	104	0	0	0	0
13:00 to 13:15	15	1	0	16	235	6	0	241	96	0	1	97	0	0	0	0	91	0	0	91	65	0	0	65	66	5	1	72	0	0	0	0
13:15 to 13:30	20	0	0	20	188	6	1	195	110	1	1	112	0	0	0	0	110	4	1	115	53	1	0	54	53	5	0	58	0	0	0	0
13:30 to 13:45	17	0	0	17	248	7	0	255	71	1	0	72	0	0	0	0	114	1	0	115	51	1	0	52	51	2	1	54	0	0	0	0
13:45 to 14:00	9	0	1	10	226	2	4	232	84	0	0	84	0	0	0	0	106	1	1	108	35	1	1	37	53	3	0	56	0	0	0	0
Total	240	2	3	245	3,767	96	8	3.871	1,505	8	9	1,522	0	0	0	0	1.673	33	9	1.715	914	15	2	931	1,080	41	14	1.135	0	0	0	0

Approach	Direction 7							Mulg	oa Rd															Jamis	on Rd							
Direction		Direct (Left					ction 8 ough)			Direc (Right	tion 9 : Turn)				ion 9U urn)				ion 10 Turn)			Direct (Thro				Directi (Right				Directi (U T	on 12U urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 10:15	35	0	0	35	219	3	0	222	30	2	0	32	0	0	0	0	39	0	0	39	33	0	0	33	18	0	0	18	0	0	0	0
10:15 to 10:30	41	1	0	42	203	7	0	210	36	0	0	36	0	0	0	0	43	1	0	44	32	0	0	32	28	1	0	29	0	0	0	0
10:30 to 10:45	41	0	0	41	245	5	1	251	45	0	0	45	0	0	0	0	39	0	0	39	35	0	0	35	14	0	1	15	0	0	0	0
10:45 to 11:00	38	1	0	39	218	5	0	223	52	0	0	52	0	0	0	0	55	0	0	55	48	0	0	48	14	0	0	14	0	0	0	0
11:00 to 11:15	51	0	0	51	263	6	0	269	36	1	0	37	0	0	0	0	45	0	0	45	38	0	1	39	18	0	1	19	0	0	0	0
11:15 to 11:30	54	0	0	54	193	8	0	201	48	1	0	49	0	0	0	0	51	0	0	51	32	0	0	32	20	0	2	22	0	0	0	0
11:30 to 11:45	56	0	0	56	243	3	0	246	51	1	0	52	0	0	0	0	38	0	0	38	30	0	0	30	22	2	0	24	0	0	0	0
11:45 to 12:00	61	0	0	61	248	4	0	252	51	0	0	51	0	0	0	0	59	0	0	59	44	0	0	44	22	1	0	23	0	0	0	0
12:00 to 12:15	68	0	0	68	237	2	0	239	45	1	0	46	0	0	0	0	37	0	0	37	41	0	0	41	17	2	0	19	0	0	0	0
12:15 to 12:30	54	1	0	55	240	4	0	244	48	0	0	48	0	0	0	0	41	0	0	41	29	0	0	29	24	1	0	25	0	0	0	0
12:30 to 12:45	63	1	0	64	246	5	1	252	54	2	0	56	0	0	0	0	46	0	0	46	37	0	0	37	19	0	0	19	0	0	0	0
12:45 to 13:00	84	0	0	84	289	4	1	294	42	0	0	42	0	0	0	0	52	0	0	52	37	0	0	37	10	0	1	11	0	0	0	0
13:00 to 13:15	75	2	0	77	243	3	0	246	55	0	0	55	0	0	0	0	49	0	0	49	48	1	0	49	16	0	0	16	0	0	0	0
13:15 to 13:30	78	1	0	79	290	9	0	299	64	0	0	64	0	0	0	0	39	0	0	39	34	0	0	34	15	0	0	15	0	0	0	0
13:30 to 13:45	57	1	0	58	247	5	0	252	40	0	0	40	0	0	0	0	45	0	0	45	54	0	0	54	31	0	0	31	0	0	0	0
13:45 to 14:00	56	1	0	57	220	1	0	221	42	0	0	42	0	0	0	0	45	0	0	45	44	0	0	44	12	0	0	12	0	0	0	0
Total	912	9	0	921	3,844	74	3	3,921	739	8	0	747	0	0	0	0	723	1	0	724	616	1	1	618	300	7	5	312	0	0	0	0

Client : Positive Traffic Pty Ltd

Suburb : Penrith Surveys

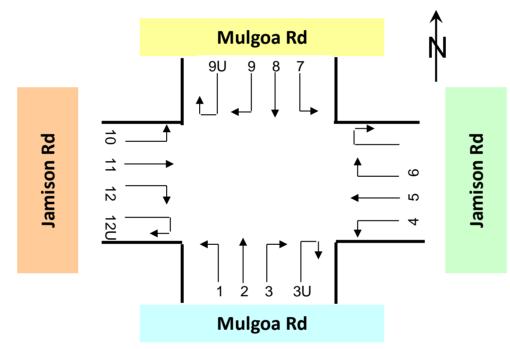
Location : 1. Jamison Rd / Mulgoa Rd

Day/Date : Sat, 21st Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Hourly Summary





Approach								Mulg	oa Rd															Jamis	on Rd							
Direction		Direc				Direct				Direc					ion 3U			Direc				Direct				Direct					tion 6U	
		<u>, </u>	Turn)			(Thro	ougn)	Ι			Turn)			(U T	urn)				Turn)	1	-	(Thro	ugnj			(Right	Turnj	I			Γurn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 11:00	44	0	1	45	923	26	2	951	363	0	2	365	0	0	0	0	395	5	2	402	189	3	0	192	263	15	5	283	0	0	0	0
10:15 to 11:15	42	0	0	42	949	26	1	976	374	1	2	377	0	0	0	0	393	5	2	400	219	2	0	221	247	10	4	261	0	0	0	0
10:30 to 11:30	51	0	0	51	927	30	0	957	399	2	2	403	0	0	0	0	397	8	1	406	258	0	1	259	229	9	4	242	0	0	0	0
10:45 to 11:45	61	0	0	61	960	26	0	986	417	3	2	422	0	0	0	0	411	9	1	421	276	1	1	278	246	6	4	256	0	0	0	0
11:00 to 12:00	59	0	0	59	972	24	0	996	401	4	2	407	0	0	0	0	431	11	2	444	281	2	1	284	263	5	3	271	0	0	0	0
11:15 to 12:15	67	0	0	67	969	25	1	995	401	3	2	406	0	0	0	0	441	11	2	454	267	5	1	273	289	5	2	296	0	0	0	0
11:30 to 12:30	75	0	1	76	1,006	19	1	1,026	380	3	3	386	0	0	0	0	427	11	3	441	246	7	0	253	298	5	3	306	0	0	0	0
11:45 to 12:45	62	0	1	63	949	22	1	972	377	2	3	382	0	0	0	0	431	12	4	447	246	8	0	254	313	6	2	321	0	0	0	0
12:00 to 13:00	76	1	1	78	975	25	1	1,001	380	2	3	385	0	0	0	0	426	11	3	440	240	7	0	247	331	6	4	341	0	0	0	0
12:15 to 13:15	69	2	1	72	958	23	0	981	379	2	3	384	0	0	0	0	407	8	3	418	239	4	0	243	320	11	5	336	0	0	0	0
12:30 to 13:30	68	2	0	70	892	25	1	918	405	2	2	409	0	0	0	0	412	8	3	423	230	3	0	233	298	13	4	315	0	0	0	0
12:45 to 13:45	75	2	0	77	917	27	1	945	365	3	2	370	0	0	0	0	418	7	1	426	224	2	0	226	271	13	4	288	0	0	0	0
13:00 to 14:00	61	1	1	63	897	21	5	923	361	2	2	365	0	0	0	0	421	6	2	429	204	3	1	208	223	15	2	240	0	0	0	0
Total	240	2	3	245	3,767	96	8	3,871	1,505	8	9	1,522	0	0	0	0	1,673	33	9	1,715	914	15	2	931	1,080	41	14	1,135	0	0	0	0

Approach								Mulg	oa Rd															Jamis	on Rd							
Direction		Direct (Left 1				Direct (Thro				Direct (Right	tion 9 Turn)				ion 9U urn)				ion 10 Turn)			Direct (Thre	ion 11 ough)			Direct (Right	ion 12 :Turn)			Directi (U T	on 12U urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 11:00	155	2	0	157	885	20	1	906	163	2	0	165	0	0	0	0	176	1	0	177	148	0	0	148	74	1	1	76	0	0	0	0
10:15 to 11:15	171	2	0	173	929	23	1	953	169	1	0	170	0	0	0	0	182	1	0	183	153	0	1	154	74	1	2	77	0	0	0	0
10:30 to 11:30	184	1	0	185	919	24	1	944	181	2	0	183	0	0	0	0	190	0	0	190	153	0	1	154	66	0	4	70	0	0	0	0
10:45 to 11:45	199	1	0	200	917	22	0	939	187	3	0	190	0	0	0	0	189	0	0	189	148	0	1	149	74	2	3	79	0	0	0	0
11:00 to 12:00	222	0	0	222	947	21	0	968	186	3	0	189	0	0	0	0	193	0	0	193	144	0	1	145	82	3	3	88	0	0	0	0
11:15 to 12:15	239	0	0	239	921	17	0	938	195	3	0	198	0	0	0	0	185	0	0	185	147	0	0	147	81	5	2	88	0	0	0	0
11:30 to 12:30	239	1	0	240	968	13	0	981	195	2	0	197	0	0	0	0	175	0	0	175	144	0	0	144	85	6	0	91	0	0	0	0
11:45 to 12:45	246	2	0	248	971	15	1	987	198	3	0	201	0	0	0	0	183	0	0	183	151	0	0	151	82	4	0	86	0	0	0	0
12:00 to 13:00	269	2	0	271	1,012	15	2	1,029	189	3	0	192	0	0	0	0	176	0	0	176	144	0	0	144	70	3	1	74	0	0	0	0
12:15 to 13:15	276	4	0	280	1,018	16	2	1,036	199	2	0	201	0	0	0	0	188	0	0	188	151	1	0	152	69	1	1	71	0	0	0	0
12:30 to 13:30	300	4	0	304	1,068	21	2	1,091	215	2	0	217	0	0	0	0	186	0	0	186	156	1	0	157	60	0	1	61	0	0	0	0
12:45 to 13:45	294	4	0	298	1,069	21	1	1,091	201	0	0	201	0	0	0	0	185	0	0	185	173	1	0	174	72	0	1	73	0	0	0	0
13:00 to 14:00	266	5	0	271	1,000	18	0	1,018	201	0	0	201	0	0	0	0	178	0	0	178	180	1	0	181	74	0	0	74	0	0	0	0
Total	912	9	0	921	3,844	74	3	3,921	739	8	0	747	0	0	0	0	723	1	0	724	616	1	1	618	300	7	5	312	0	0	0	0

Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

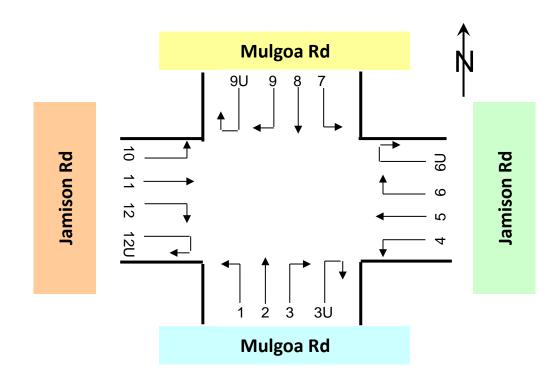
Location : 1. Jamison Rd / Mulgoa Rd

Day/Date : Sat, 21st Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Peak Hour Summary





Approach		Mulg	oa Rd			Jamis	on Rd			Mulg	oa Rd			Jamis	on Rd		otal
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand 1
12:30 to 13:30	1,365	29	3	1,397	940	24	7	971	1,583	27	2	1,612	402	1	1	404	4,384

Ар	proa	ch		Mulg	oa Rd			Jamis	on Rd			Mulg	oa Rd			Jamis	on Rd		otal
Tim	e Pei	riod	Cars	Trucks	Buses	Total	Grand Total												
10:00	to	11:00	1,330	26	5	1,361	847	23	7	877	1,203	24	1	1,228	398	2	1	401	3,867
10:15	to	11:15	1,365	27	3	1,395	859	17	6	882	1,269	26	1	1,296	409	2	3	414	3,987
10:30	to	11:30	1,377	32	2	1,411	884	17	6	907	1,284	27	1	1,312	409	0	5	414	4,044
10:45	to	11:45	1,438	29	2	1,469	933	16	6	955	1,303	26	0	1,329	411	2	4	417	4,170
11:00	to	12:00	1,432	28	2	1,462	975	18	6	999	1,355	24	0	1,379	419	3	4	426	4,266
11:15	to	12:15	1,437	28	3	1,468	997	21	5	1,023	1,355	20	0	1,375	413	5	2	420	4,286
11:30	to	12:30	1,461	22	5	1,488	971	23	6	1,000	1,402	16	0	1,418	404	6	0	410	4,316
11:45	to	12:45	1,388	24	5	1,417	990	26	6	1,022	1,415	20	1	1,436	416	4	0	420	4,295
12:00	to	13:00	1,431	28	5	1,464	997	24	7	1,028	1,470	20	2	1,492	390	3	1	394	4,378
12:15	to	13:15	1,406	27	4	1,437	966	23	8	997	1,493	22	2	1,517	408	2	1	411	4,362
12:30	to	13:30	1,365	29	3	1,397	940	24	7	971	1,583	27	2	1,612	402	1	1	404	4,384
12:45	to	13:45	1,357	32	3	1,392	913	22	5	940	1,564	25	1	1,590	430	1	1	432	4,354
13:00	to	14:00	1,319	24	8	1,351	848	24	5	877	1,467	23	0	1,490	432	1	0	433	4,151
	Total		5,512	106	20	5,638	3,667	89	25	3,781	5,495	91	3	5,589	1,639	9	6	1,654	16,662

: N5321 Job No.

Client : Positive Traffic Pty Ltd Suburb : Penrith Surveys

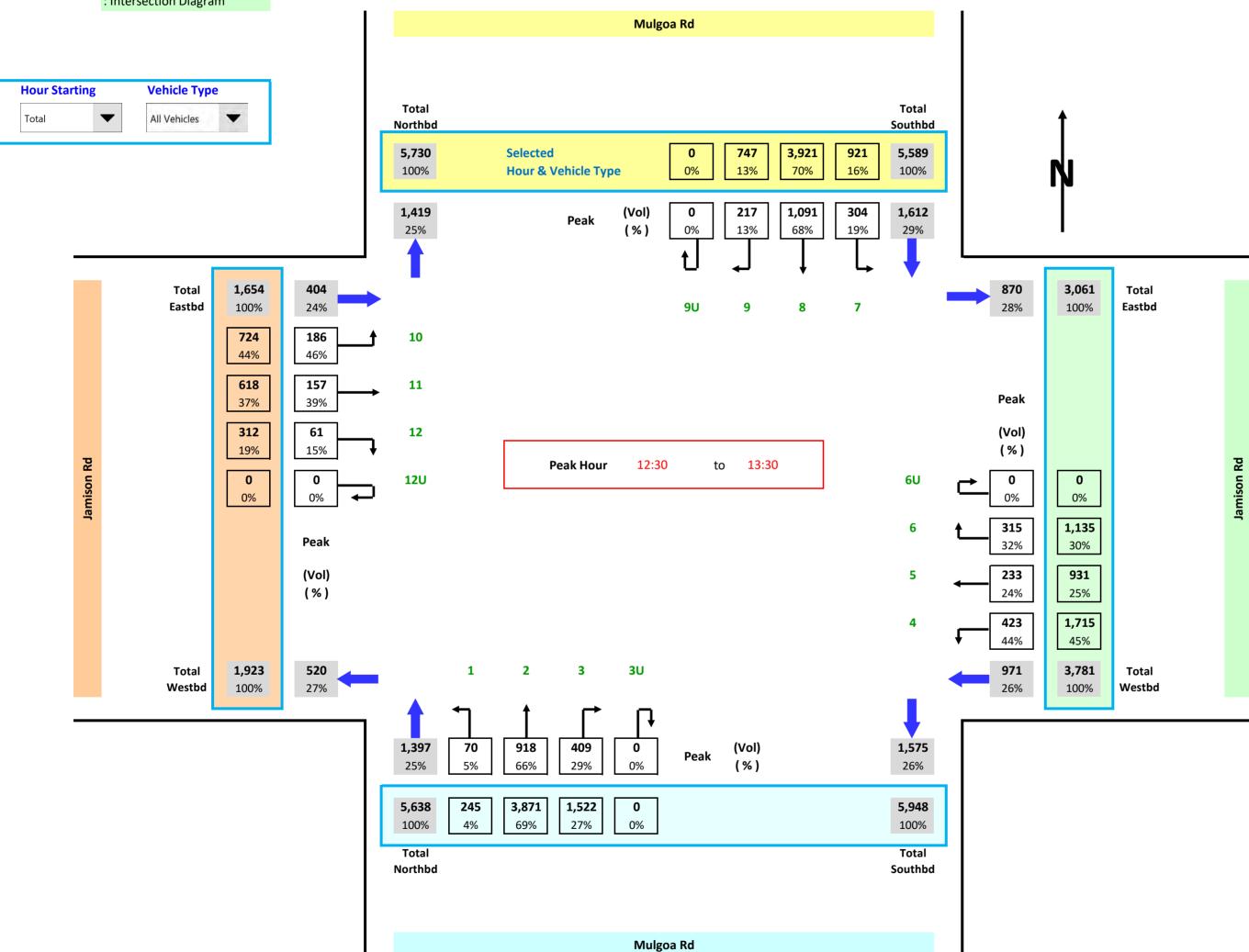
Location : 1. Jamison Rd / Mulgoa Rd

: Sat, 21st Sep 2019 Day/Date

Weather : Fine

Description : Classified Intersection Count

: Intersection Diagram





Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date : Sat, 21st Sep 2019

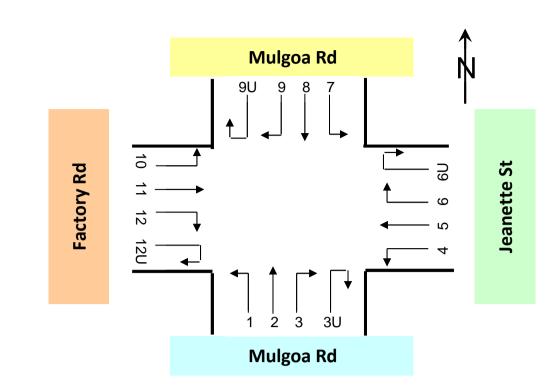
Weather : Fine

Description : Classified Intersection Count

: 15 mins Data

Class 1 Class 2 Class 3

Classifications Cars Trucks Buses





Approach								Mulg	oa Rd															Jeane	tte St							
Direction		Direc (Left				Direc (Thre				Direc (Right				Direct (U T					tion 4 Turn)				tion 5 ough)				tion 6 Turn)				ion 6U urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 10:15	5	0	0	5	305	7	1	313	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 to 10:30	11	0	0	11	286	4	2	292	0	0	0	0	0	0	0	0	1	0	О	1	О	0	0	0	0	0	0	0	0	0	0	0
10:30 to 10:45	10	1	0	11	264	6	0	270	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 to 11:00	10	0	1	11	293	6	1	300	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 to 11:15	15	0	0	15	290	5	0	295	0	0	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0
11:15 to 11:30	10	0	0	10	258	5	1	264	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
11:30 to 11:45	9	0	0	9	255	2	0	257	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	2	0	2	0	0	0	0
11:45 to 12:00	12	0	0	12	271	7	0	278	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 to 12:15	18	0	0	18	278	3	1	282	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
12:15 to 12:30	10	0	0	10	286	1	2	289	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 to 12:45	4	1	0	5	288	1	0	289	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 to 13:00	8	0	0	8	259	6	1	266	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	1	0	1	0	0	0	0
13:00 to 13:15	3	0	0	3	262	3	0	265	0	0	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0
13:15 to 13:30	8	0	1	9	223	4	1	228	0	0	0	0	0	0	0	0	0	0	o	0	0	0	0	0	0	0	0	0	0	0	0	0
13:30 to 13:45	6	0	0	6	224	6	0	230	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:45 to 14:00	6	0	0	6	209	1	1	211	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
Total	145	2	2	149	4,251	67	11	4,329	0	0	0	0	0	0	0	0	14	0	0	14	0	0	0	0	0	3	0	3	0	0	0	0

Approach								Mulg	oa Rd															Facto	ory Rd							
Direction			tion 7 Turn)				tion 8 ough)			Direc (Right					ion 9U urn)				ion 10 Turn)				ion 11 ough)				ion 12 Turn)			Direction (U T		
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 10:15	3	0	0	3	246	8	0	254	0	0	0	0	0	0	0	0	21	1	0	22	0	0	0	0	0	0	0	0	0	0	0	0
10:15 to 10:30	4	0	1	5	260	5	0	265	0	0	0	0	0	0	0	0	13	0	0	13	0	0	0	0	0	0	0	0	0	0	0	0
10:30 to 10:45	3	0	0	3	310	5	0	315	0	0	0	0	0	0	0	0	28	0	0	28	0	0	0	0	1	0	0	1	0	0	0	0
10:45 to 11:00	7	0	0	7	257	11	1	269	0	0	0	0	0	0	0	0	25	1	0	26	0	0	0	0	0	0	0	0	0	0	0	0
11:00 to 11:15	4	0	0	4	273	3	0	276	0	0	0	0	0	0	0	0	19	0	0	19	0	0	0	0	0	0	0	0	0	0	0	0
11:15 to 11:30	6	0	0	6	327	6	0	333	0	0	0	0	0	О	0	0	17	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0
11:30 to 11:45	3	0	1	4	277	9	0	286	0	0	0	0	0	0	0	0	20	0	0	20	0	0	0	0	0	0	0	0	0	0	0	0
11:45 to 12:00	3	0	0	3	296	2	1	299	1	0	0	1	0	0	0	0	26	0	О	26	0	0	0	0	1	0	0	1	0	0	0	0
12:00 to 12:15	5	1	0	6	318	7	0	325	0	0	0	0	0	0	0	0	25	0	0	25	0	0	0	0	0	0	0	0	0	0	0	0
12:15 to 12:30	2	1	1	4	310	11	0	321	0	0	0	0	0	0	0	0	28	2	0	30	0	0	0	0	0	0	0	0	0	0	0	0
12:30 to 12:45	4	0	0	4	298	3	0	301	0	0	0	0	0	0	0	0	27	1	o	28	0	0	0	0	0	0	0	0	0	0	0	0
12:45 to 13:00	5	1	0	6	305	4	1	310	0	0	0	0	0	0	0	0	30	0	0	30	1	0	0	1	0	0	0	0	0	0	0	0
13:00 to 13:15	12	1	0	13	302	13	0	315	0	0	0	0	0	0	0	0	17	1	0	18	0	0	0	0	0	0	0	0	0	0	0	0
13:15 to 13:30	8	1	1	10	333	6	1	340	0	0	0	0	0	0	0	0	28	0	0	28	0	0	0	0	0	0	0	0	0	0	0	0
13:30 to 13:45	4	0	0	4	303	12	0	315	0	0	0	0	0	0	0	0	30	1	0	31	0	0	0	0	0	0	0	0	0	0	0	0
13:45 to 14:00	4	1	0	5	320	4	1	325	0	0	0	0	1	0	0	1	27	0	0	27	0	0	0	0	0	0	0	0	0	0	0	0
Total	77	6	4	87	4,735	109	5	4,849	1	0	0	1	1	0	0	1	381	7	0	388	1	0	0	1	2	0	0	2	0	0	0	0

Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

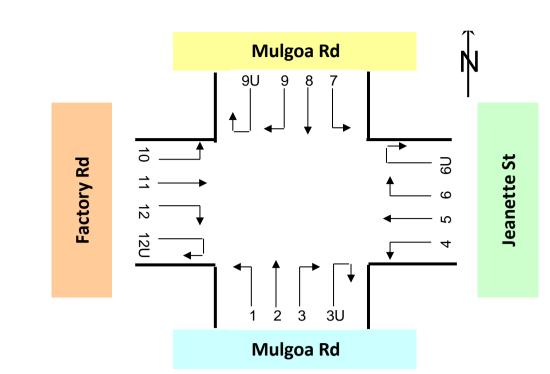
Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date : Sat, 21st Sep 2019

Weather : Fine

Description: Classified Intersection Count

: Hourly Summary





Approach								Mulg	oa Rd															Jeane	ette St							
Direction		Direc (Left				Direc (Thro				Direc				Direct (U T	ion 3U urn)				tion 4 Turn)			Direc					tion 6 t Turn)			Directi (U T		
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 11:00	36	1	1	38	1,148	23	4	1,175	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
10:15 to 11:15	46	1	1	48	1,133	21	3	1,157	0	0	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0
10:30 to 11:30	45	1	1	47	1,105	22	2	1,129	0	0	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0
10:45 to 11:45	44	0	1	45	1,096	18	2	1,116	0	0	0	0	0	0	0	0	7	0	0	7	0	0	0	0	0	2	0	2	0	0	0	0
11:00 to 12:00	46	0	0	46	1,074	19	1	1,094	0	0	0	0	0	0	0	0	7	0	0	7	0	0	0	0	0	2	0	2	0	0	0	0
11:15 to 12:15	49	0	0	49	1,062	17	2	1,081	0	0	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	2	0	2	0	0	0	0
11:30 to 12:30	49	0	0	49	1,090	13	3	1,106	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	2	0	2	0	0	0	0
11:45 to 12:45	44	1	0	45	1,123	12	3	1,138	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0
12:00 to 13:00	40	1	0	41	1,111	11	4	1,126	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	1	0	1	0	0	0	0
12:15 to 13:15	25	1	0	26	1,095	11	3	1,109	0	0	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	1	0	1	0	0	0	0
12:30 to 13:30	23	1	1	25	1,032	14	2	1,048	0	0	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	1	0	1	0	0	0	0
12:45 to 13:45	25	0	1	26	968	19	2	989	0	0	0	0	0	0	0	0	4	0	0	4	0	0	0	0	0	1	0	1	0	0	0	0
13:00 to 14:00	23	0	1	24	918	14	2	934	0	0	0	0	0	0	0	0	3	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0
Total	145	2	2	149	4,251	67	11	4,329	0	0	0	0	0	0	0	0	14	0	0	14	0	0	0	0	0	3	0	3	0	0	0	0

Approach								Mulg	oa Rd															Facto	ory Rd							
Direction		Direct (Left				Direc (Thro				Direc (Right	tion 9 Turn)				ion 9U urn)				tion 10 Turn)			Direct (Thro					tion 12 t Turn)			Direction (U To	on 12U urn)	
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total
10:00 to 11:00	17	0	1	18	1,073	29	1	1,103	0	0	0	0	0	0	0	0	87	2	0	89	0	0	0	0	1	0	0	1	0	0	0	0
10:15 to 11:15	18	0	1	19	1,100	24	1	1,125	0	0	0	0	0	0	0	0	85	1	0	86	0	0	0	0	1	0	0	1	0	0	0	0
10:30 to 11:30	20	0	0	20	1,167	25	1	1,193	0	0	0	0	0	0	0	0	89	1	0	90	0	0	0	0	1	0	0	1	0	0	0	0
10:45 to 11:45	20	0	1	21	1,134	29	1	1,164	0	0	0	0	0	0	0	0	81	1	0	82	0	0	0	0	0	0	0	0	0	0	0	0
11:00 to 12:00	16	0	1	17	1,173	20	1	1,194	1	0	0	1	0	0	0	0	82	0	0	82	0	0	0	0	1	0	0	1	0	0	0	0
11:15 to 12:15	17	1	1	19	1,218	24	1	1,243	1	0	0	1	0	0	0	0	88	0	0	88	0	0	0	0	1	0	0	1	0	0	0	0
11:30 to 12:30	13	2	2	17	1,201	29	1	1,231	1	0	0	1	0	0	0	0	99	2	0	101	0	0	0	0	1	0	0	1	0	0	0	0
11:45 to 12:45	14	2	1	17	1,222	23	1	1,246	1	0	0	1	0	0	0	0	106	3	0	109	0	0	0	0	1	0	0	1	0	0	0	0
12:00 to 13:00	16	3	1	20	1,231	25	1	1,257	0	0	0	0	0	0	0	0	110	3	0	113	1	0	0	1	0	0	0	0	0	0	0	0
12:15 to 13:15	23	3	1	27	1,215	31	1	1,247	0	0	0	0	0	0	0	0	102	4	0	106	1	0	0	1	0	0	0	0	0	0	0	0
12:30 to 13:30	29	3	1	33	1,238	26	2	1,266	0	0	0	0	0	0	0	0	102	2	0	104	1	0	0	1	0	0	0	0	0	0	0	0
12:45 to 13:45	29	3	1	33	1,243	35	2	1,280	0	0	0	0	0	0	0	0	105	2	0	107	1	0	0	1	0	0	0	0	0	0	0	0
13:00 to 14:00	28	3	1	32	1,258	35	2	1,295	0	0	0	0	1	0	0	1	102	2	0	104	0	0	0	0	0	0	0	0	0	0	0	0
Total	77	6	4	87	4,735	109	5	4,849	1	0	0	1	1	0	0	1	381	7	0	388	1	0	0	1	2	0	0	2	0	0	0	0

Client : Positive Traffic Pty Ltd
Suburb : Penrith Surveys

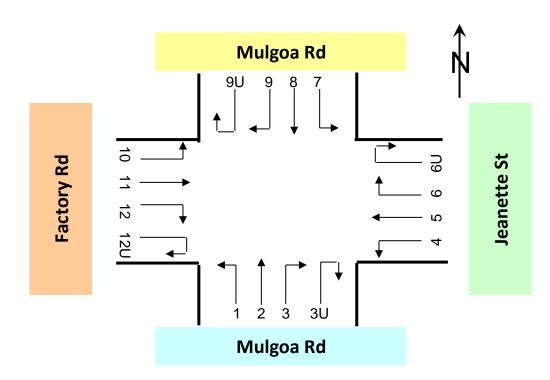
Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date : Sat, 21st Sep 2019

Weather : Fine

Description : Classified Intersection Count

: Peak Hour Summary





Approach		Mulg	oa Rd			Jeane	ette St			Mulg	oa Rd			Facto	ry Rd		otal
Time Period	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand 1
12:00 to 13:00	1,151	12	4	1,167	3	1	0	4	1,247	28	2	1,277	111	3	0	114	2,562

Ар	proa	ich		Mulg	oa Rd			Jeane	ette St			Mulg	oa Rd			Facto	ry Rd		otal
Tim	e Pe	riod	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Cars	Trucks	Buses	Total	Grand Total
10:00	to	11:00	1,184	24	5	1,213	1	0	0	1	1,090	29	2	1,121	88	2	0	90	2,425
10:15	to	11:15	1,179	22	4	1,205	5	0	0	5	1,118	24	2	1,144	86	1	0	87	2,441
10:30	to	11:30	1,150	23	3	1,176	5	0	0	5	1,187	25	1	1,213	90	1	0	91	2,485
10:45	to	11:45	1,140	18	3	1,161	7	2	0	9	1,154	29	2	1,185	81	1	0	82	2,437
11:00	to	12:00	1,120	19	1	1,140	7	2	0	9	1,190	20	2	1,212	83	0	0	83	2,444
11:15	to	12:15	1,111	17	2	1,130	4	2	0	6	1,236	25	2	1,263	89	0	0	89	2,488
11:30	to	12:30	1,139	13	3	1,155	3	2	0	5	1,215	31	3	1,249	100	2	0	102	2,511
11:45	to	12:45	1,167	13	3	1,183	1	0	0	1	1,237	25	2	1,264	107	3	0	110	2,558
12:00	to	13:00	1,151	12	4	1,167	3	1	0	4	1,247	28	2	1,277	111	3	0	114	2,562
12:15	to	13:15	1,120	12	3	1,135	4	1	0	5	1,238	34	2	1,274	103	4	0	107	2,521
12:30	to	13:30	1,055	15	3	1,073	4	1	0	5	1,267	29	3	1,299	103	2	0	105	2,482
12:45	to	13:45	993	19	3	1,015	4	1	0	5	1,272	38	3	1,313	106	2	0	108	2,441
13:00	to	14:00	941	14	3	958	3	0	0	3	1,287	38	3	1,328	102	2	0	104	2,393
	Tota	I	4,396	69	13	4,478	14	3	0	17	4,814	115	9	4,938	384	7	0	391	9,824

: N5321 Job No.

Client : Positive Traffic Pty Ltd : Penrith Surveys Suburb

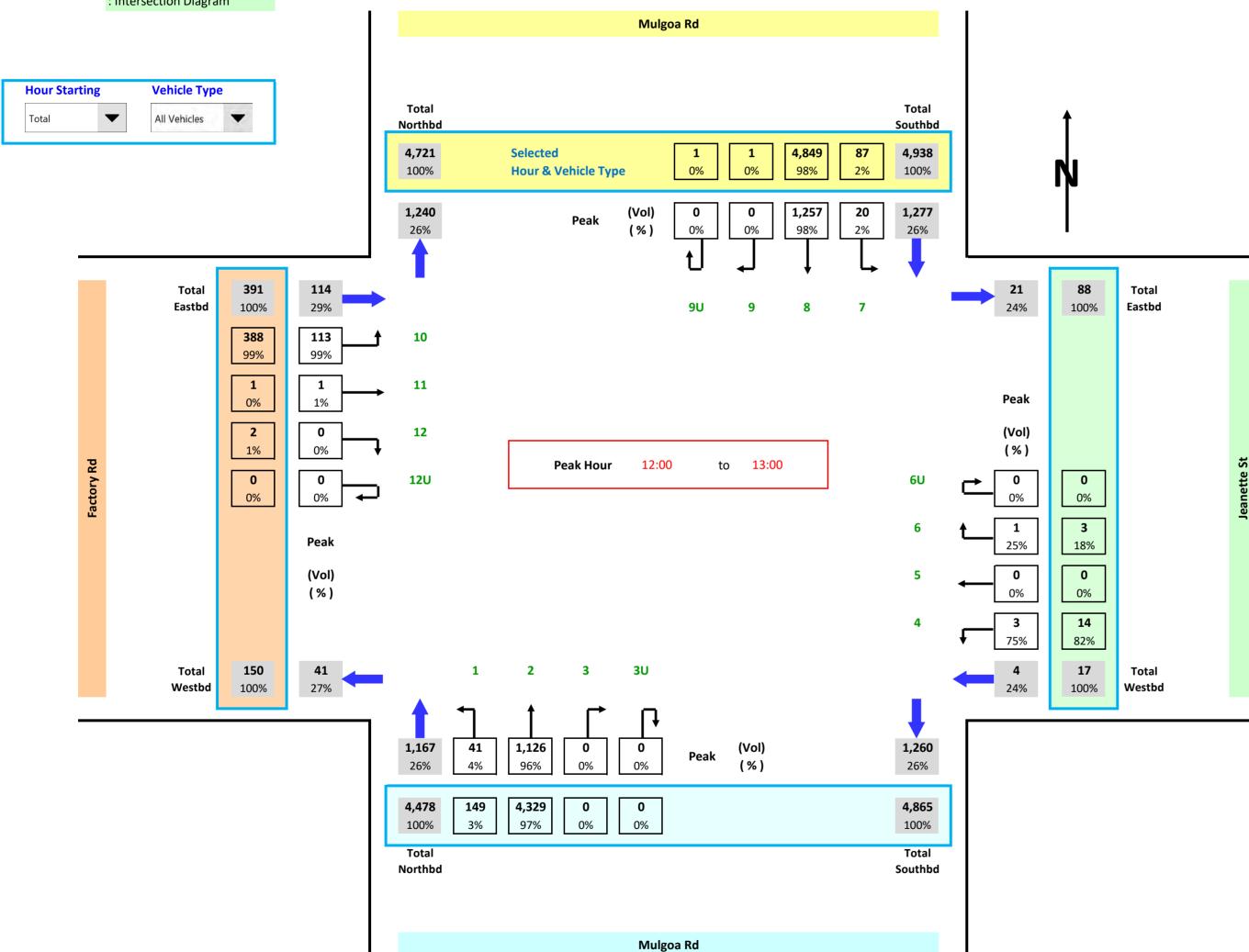
Location : 2. Mulgoa Rd & Factory Rd & Jeanette St

Day/Date : Sat, 21st Sep 2019

Weather : Fine

: Classified Intersection Count Description

: Intersection Diagram





8. Appendix B - Plans of Proposed Scheme

44 - 50 TENCH AVE **JAMISONTOWN**

DEVELOPMENT APPLICATION

ORANGE FIELDS - CAFE

- 00 COVER PAGE
- 01 EXISTING SITE PLAN
- 02 MASTER SITE PLAN
- 03 PROPOSED HOUSE DINING
- 04 PROPOSED PAVILIONS & KITCHEN
- 05 PROPOSED CO-OP PAVILION & SEATING PODS
- 06 PROPOSED PLAYGROUND
- 07 PROPOSED PARKING
- 08 PROPOSED LIVESTOCK ENCLOSURE
- **ROOF PLAN**
- 21 ELEVATIONS PAGE 1
- 22 ELEVATIONS PAGE 2
- 23 ELEVATIONS PAGE 3
- 24 ELEVATIONS PAGE 4
- 25 ELEVATIONS PAGE 5
- 26 ELEVATIONS PAGE 6
- 27 ELEVATIONS PAGE 7
- 28 ELEVATIONS PAGE 8
- 29 ELEVATIONS PAGE 9
- 81 ARCHITECTURAL VISUALISATIONS
- 91 PROPOSED ARCHITECTURAL FINISHES







PLANNER HOPKINS CONSULTANTS MELISSAH OSLAND 02 6583 6722 Melissah.Osland@hopcon.com.au



Version: 1, Version Date: 17/12/2019

ACCESS CONSULTANT PHILIP CHUN ACCESSIBILITY LUCY ALDERSON - 02 9412 2322 Lucy Alderson@philipchun.com Document Set ID: 8967005



TRAFFIC CONSULTANT POSITIVE TRAFFIC DEAN BRODIE 0414 462247 dean@positivetraffic.com.au



KITCHEN CONSULTANT ROB LECHOWICZ 0423 015 067 rob_lichowicz@live.com



AIR QUALITY SLR CONSULTING VARUN MARWAHA 02 9427 8100 vmarwaha@slrconsulting.com



EFFLUENT SLR CONSULTING PETER CUPITT 02 4037 3200 pcupitt@slrconsulting.com



CONTAMINATION SLR CONSULTING LACHLAN MCWHA 02 9427 8100 lmcwha@slrconsulting.com



STORM WATER CONSULTANT MBR CONSULTANTS MICHAEL BOU RADA 0459 117 674 michael@mbrconsulting.com.au

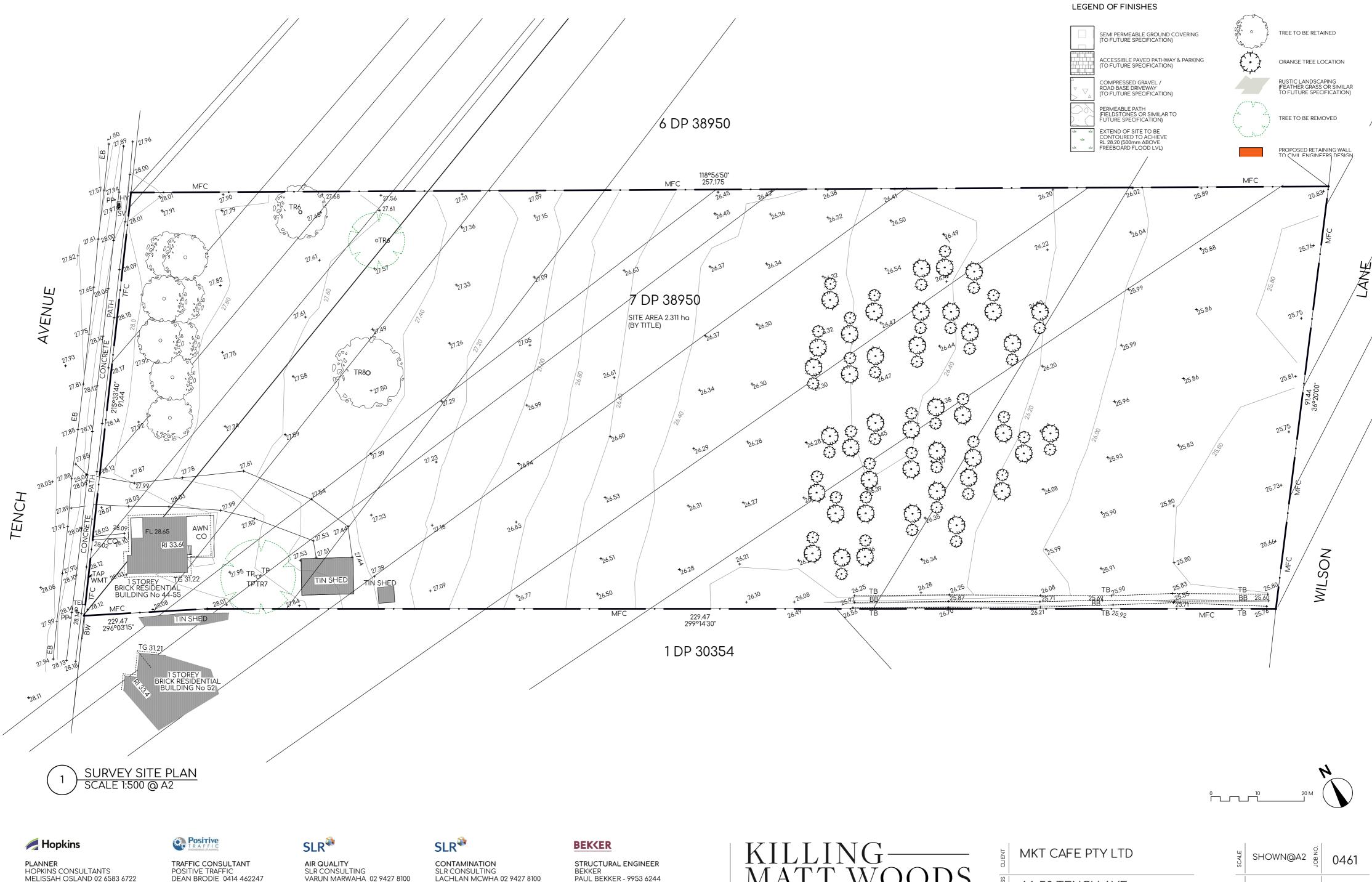


STRUCTURAL ENGINEER BEKKER PAUL BEKKER - 9953 6244 paulbekker@spin.net.au



CLIENT	MKT CAFE PTY LTD
ADDRESS	44-50 TENCH AVE JAMISONTOWN NSW
AWING NAME	COVER PAGE

SCALE	SHOWN@A2	JOB NO.	0461
DATE	27.09.19	DRAWN	MW
DRAWING NO.	00	ISSUE	D



HOPKINS CONSULTANTS
MELISSAH OSLAND 02 6583 6722
Melissah.Osland@hopcon.com.au

CHUN CHUN

Version: 1, Version Date: 17/12/2019

ACCESS CONSULTANT
PHILIP CHUN ACCESSIBILITY
LUCY ALDERSON - 02 9412 2322 Lucy Alderson@philipchun.com Document Set ID: 8967005 TRAFFIC CONSULTANT POSITIVE TRAFFIC DEAN BRODIE 0414 462247 dean@positivetraffic.com.au



KITCHEN CONSULTANT ROB LECHOWICZ 0423 015 067 rob_lichowicz@live.com

AIR QUALITY SLR CONSULTING VARUN MARWAHA 02 9427 8100 vmarwaha@slrconsulting.com

SLR

EFFLUENT SLR CONSULTING PETER CUPITT 02 4037 3200 pcupitt@slrconsulting.com SLR CONSULTING LACHLAN MCWHA 02 9427 8100 lmcwha@slrconsulting.com

MR

STORM WATER CONSULTANT MBR CONSULTANTS MICHAEL BOU RADA 0459 117 674 michael@mbrconsulting.com.au BEKKER PAUL BEKKER - 9953 6244 paulbekker@spin.net.au

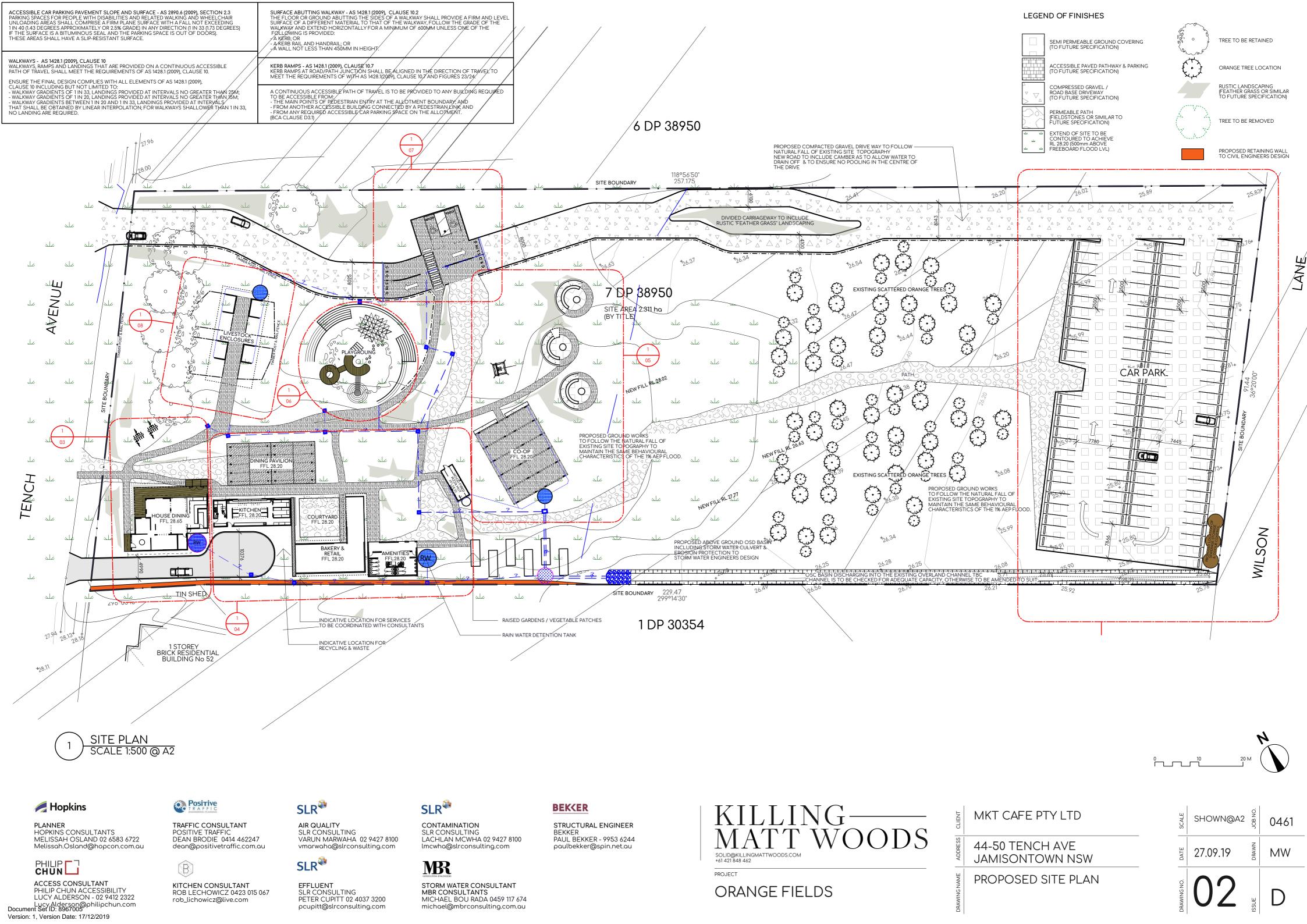


PROJECT

ORANGE FIELDS

44-50 TENCH AVE JAMISONTOWN NSW EXISTING SITE PLAN

27.09.19 MW









1 TOWARDS DINING PAVILIONS
SCALE 1:100 @ A2



PLANNER HOPKINS CONSULTANTS MELISSAH OSLAND 02 6583 6722 Melissah.Osland@hopcon.com.au

PHILIP CT

Version: 1, Version Date: 17/12/2019

ACCESS CONSULTANT
PHILIP CHUN ACCESSIBILITY
LUCY ALDERSON - 02 9412 2322
Lucy Alderson@philipchun.com
Document Set ID: 8967005



TRAFFIC CONSULTANT POSITIVE TRAFFIC DEAN BRODIE 0414 462247 dean@positivetraffic.com.au



KITCHEN CONSULTANT ROB LECHOWICZ 0423 015 067 rob_lichowicz@live.com



AIR QUALITY SLR CONSULTING VARUN MARWAHA 02 9427 8100 vmarwaha@strconsulting.com



EFFLUENT SLR CONSULTING PETER CUPITT 02 4037 3200 pcupitt@slrconsulting.com



CONTAMINATION SLR CONSULTING LACHLAN MCWHA 02 9427 8100 lmcwha@slrconsulting.com



STORM WATER CONSULTANT MBR CONSULTANTS MICHAEL BOU RADA 0459 117 674 michael@mbrconsulting.com.au



STRUCTURAL ENGINEER BEKKER PAUL BEKKER - 9953 6244 paulbekker@spin.net.au



4 TOWARDS WC'S & RETAIL
NTS



TOWARDS ANIMAL ENCLOSURES
NTS



PROJECT

ORANGE FIELDS

MKT CAFE PTY LTD

44-50 TENCH AVE
JAMISONTOWN NSW

VISUALISATIONS

31 sane

9. Appendix C – SIDRA Outputs

Site: 101 [Mulgoa_Factory_Wday_PM_Ex]

Giveway / Yield (Two-Way)

Intersection Performance - Hourly Values		
Performance Measure	Vehicles	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total)	57.8 km/h 1428.2 veh-km/h 24.7 veh-h/h	57.8 km/h 1713.9 pers-km/h 29.6 pers-h/h
Demand Flows (Total) Percent Heavy Vehicles (Demand) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	1405 veh/h 2.6 % 0.303 223.0 % 4632 veh/h	1686 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane) Control Delay (Worst Movement) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	0.74 veh-h/h 1.9 sec 12.6 sec 9.5 sec 1.2 sec 0.7 sec 0.3 sec NA	0.88 pers-h/h 1.9 sec 9.5 sec
95% Back of Queue - Vehicles (Worst Lane) 95% Back of Queue - Distance (Worst Lane) Queue Storage Ratio (Worst Lane) Total Effective Stops Effective Stop Rate Proportion Queued Performance Index	0.7 veh 4.9 m 0.00 223 veh/h 0.16 per veh 0.11 26.3	268 pers/h 0.16 per pers 0.11 26.3
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	558.08 \$/h 97.5 L/h 231.0 kg/h 0.017 kg/h 0.279 kg/h 0.235 kg/h	558.08 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). NA: Intersection LOS for Vehicles is Not Applicable for two-way sign control since the average intersection delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Persons
Demand Flows (Total)	674,526 veh/y	809,432 pers/y
Delay	354 veh-h/y	424 pers-h/y
Effective Stops	107,200 veh/y	128,640 pers/y
Travel Distance	685,545 veh-km/y	822,653 pers-km/y
Travel Time	11,853 veh-h/y	14,224 pers-h/y
Cost	267,876 \$/v	267,876 \$/y
Fuel Consumption	46,806 L/v	, . ,
Carbon Dioxide	110,880 kg/y	
Hydrocarbons	8 kg/y	
Carbon Monoxide	134 kg/y	
NOx	113 kg/y	

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:37 PM Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_Wday_PM_Ex]

New Site Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand I Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
טו	IVIOV	veh/h	%	v/c	sec	Service	verlicies veh	Distance m	Queueu	per veh	km/h
South:	: Mulgoa l	Rd									
1	L2	46	1.0	0.303	5.6	LOS A	0.0	0.0	0.00	0.05	57.8
2	T1	1113	3.0	0.303	0.0	LOS A	0.0	0.0	0.00	0.02	59.7
Appro	ach	1159	2.9	0.303	0.3	NA	0.0	0.0	0.00	0.02	59.6
West:	Factory F	₹d									
10	L2	246	1.0	0.172	9.5	LOS A	0.7	4.9	0.60	0.79	50.6
Appro	ach	246	1.0	0.172	9.5	LOS A	0.7	4.9	0.60	0.79	50.6
All Vel	hicles	1405	2.6	0.303	1.9	NA	0.7	4.9	0.11	0.16	57.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:37 PM
Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_Wday_PM_Ex+Dev]

Giveway / Yield (Two-Way)

Performance Measure	Vehicles	Persons
Fravel Speed (Average)	57.8 km/h	57.8 km/h
Fravel Distance (Total)	1432.5 veh-km/h	1719.0 pers-km/h
Travel Time (Total)	24.8 veh-h/h	29.7 pers-h/h
Demand Flows (Total)	1409 veh/h	1691 pers/h
Percent Heavy Vehicles (Demand)	2.6 %	
Degree of Saturation	0.305	
Practical Spare Capacity	221.8 %	
Effective Intersection Capacity	4628 veh/h	
Control Delay (Total)	0.74 veh-h/h	0.89 pers-h/h
Control Delay (Average)	1.9 sec	1.9 sec
Control Delay (Worst Lane)	12.7 sec	1.9 360
Control Delay (Worst Movement)	9.5 sec	9.5 sec
Geometric Delay (Average)	1.2 sec	0.0 000
Stop-Line Delay (Average)	0.7 sec	
dling Time (Average)	0.3 sec	
ntersection Level of Service (LOS)	NA	
,		
95% Back of Queue - Vehicles (Worst Lane)	0.7 veh	
95% Back of Queue - Distance (Worst Lane)	4.9 m	
Queue Storage Ratio (Worst Lane)	0.00	
Total Effective Stops	226 veh/h	271 pers/h
Effective Stop Rate	0.16 per veh	0.16 per pers
Proportion Queued	0.10	0.10
Performance Index	26.4	26.4
Cost (Total)	560.23 \$/h	560.23 \$/h
Fuel Consumption (Total)	97.9 L/h	·
Carbon Dioxide (Total)	231.8 kg/h	
Hydrocarbons (Total)	0.017 kg/h	
Carbon Monoxide (Total)	0.280 kg/h	
NOx (Total)	0.236 kg/h	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). NA: Intersection LOS for Vehicles is Not Applicable for two-way sign control since the average intersection delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Persons
Demand Flows (Total)	676,547 veh/y	811,857 pers/y
Delay	356 veh-h/y	428 pers-h/y
Effective Stops	108,303 veh/y	129,964 pers/y
Travel Distance	687,597 veh-km/y	825,117 pers-km/y
Travel Time	11,892 veh-h/y	14,270 pers-h/y
Cost	268,908 \$/y	268,908 \$/y
Fuel Consumption	46,977 L/y	
Carbon Dioxide	111,284 kg/y	
Hydrocarbons	8 kg/y	
Carbon Monoxide	135 kg/y	
NOx	113 kg/y	

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:37 PM Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_Wday_PM_Ex+Dev]

Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa l	Rd									
1	L2	51	1.0	0.305	5.6	LOS A	0.0	0.0	0.00	0.05	57.8
2	T1	1113	3.0	0.305	0.0	LOS A	0.0	0.0	0.00	0.02	59.7
Appro	ach	1163	2.9	0.305	0.3	NA	0.0	0.0	0.00	0.03	59.6
West:	Factory F	₹d									
10	L2	246	1.0	0.172	9.5	LOS A	0.7	4.9	0.60	0.79	50.6
Appro	ach	246	1.0	0.172	9.5	LOS A	0.7	4.9	0.60	0.79	50.6
All Vel	hicles	1409	2.6	0.305	1.9	NA	0.7	4.9	0.10	0.16	57.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:37 PM
Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_WEnd_AM_Ex]

Giveway / Yield (Two-Way)

Intersection Performance - Hourly Values		
Performance Measure	Vehicles	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total)	58.7 km/h 1369.5 veh-km/h 23.3 veh-h/h	58.7 km/h 1643.4 pers-km/h 28.0 pers-h/h
Demand Flows (Total) Percent Heavy Vehicles (Demand) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	1347 veh/h 2.8 % 0.322 204.8 % 4190 veh/h	1617 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane) Control Delay (Worst Movement) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	0.40 veh-h/h 1.1 sec 13.2 sec 9.7 sec 0.7 sec 0.4 sec 0.2 sec NA	0.48 pers-h/h 1.1 sec 9.7 sec
95% Back of Queue - Vehicles (Worst Lane) 95% Back of Queue - Distance (Worst Lane) Queue Storage Ratio (Worst Lane) Total Effective Stops Effective Stop Rate Proportion Queued Performance Index	0.3 veh 2.4 m 0.00 119 veh/h 0.09 per veh 0.05 24.1	142 pers/h 0.09 per pers 0.05 24.1
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	511.09 \$/h 90.7 L/h 215.0 kg/h 0.016 kg/h 0.261 kg/h 0.227 kg/h	511.09 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). NA: Intersection LOS for Vehicles is Not Applicable for two-way sign control since the average intersection delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Persons
Demand Flows (Total)	646,737 veh/y	776,084 pers/y
Delay	193 veh-h/y	231 pers-h/y
Effective Stops	56,955 veh/y	68,346 pers/y
Travel Distance	657,346 veh-km/y	788,815 pers-km/y
Travel Time	11,193 veh-h/y	13,432 pers-h/y
Cost	245,325 \$/y	245,325 \$/y
Fuel Consumption	43,525 L/y	-
Carbon Dioxide	103,187 kg/y	
Hydrocarbons	8 kg/y	
Carbon Monoxide	125 kg/y	
NOx	109 kg/y	

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:38 PM Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_WEnd_AM_Ex]

New Site Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa	Rd									
1	L2	43	1.0	0.322	5.6	LOS A	0.0	0.0	0.00	0.04	57.9
2	T1	1185	3.0	0.322	0.0	LOS A	0.0	0.0	0.00	0.02	59.7
Appro	ach	1228	2.9	0.322	0.2	NA	0.0	0.0	0.00	0.02	59.7
West:	Factory F	₹d									
10	L2	119	1.0	0.089	9.7	LOS A	0.3	2.4	0.60	0.78	50.5
Appro	ach	119	1.0	0.089	9.7	LOS A	0.3	2.4	0.60	0.78	50.5
All Vel	hicles	1347	2.8	0.322	1.1	NA	0.3	2.4	0.05	0.09	58.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:38 PM
Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_WEnd_AM_Ex+Dev]

Giveway / Yield (Two-Way)

Performance Measure	Vehicles	Persons
Travel Speed (Average) Travel Distance (Total) Travel Time (Total)	58.7 km/h 1373.7 veh-km/h 23.4 veh-h/h	58.7 km/h 1648.5 pers-km/h 28.1 pers-h/h
Demand Flows (Total) Percent Heavy Vehicles (Demand) Degree of Saturation Practical Spare Capacity Effective Intersection Capacity	1352 veh/h 2.8 % 0.323 203.7 % 4189 veh/h	1622 pers/h
Control Delay (Total) Control Delay (Average) Control Delay (Worst Lane) Control Delay (Worst Movement) Geometric Delay (Average) Stop-Line Delay (Average) Idling Time (Average) Intersection Level of Service (LOS)	0.41 veh-h/h 1.1 sec 13.3 sec 9.7 sec 0.7 sec 0.4 sec 0.2 sec NA	0.49 pers-h/h 1.1 sec 9.7 sec
95% Back of Queue - Vehicles (Worst Lane) 95% Back of Queue - Distance (Worst Lane) Queue Storage Ratio (Worst Lane) Total Effective Stops Effective Stop Rate Proportion Queued Performance Index	0.3 veh 2.4 m 0.00 121 veh/h 0.09 per veh 0.05 24.2	145 pers/h 0.09 per pers 0.05 24.2
Cost (Total) Fuel Consumption (Total) Carbon Dioxide (Total) Hydrocarbons (Total) Carbon Monoxide (Total) NOx (Total)	513.25 \$/h 91.0 L/h 215.8 kg/h 0.016 kg/h 0.262 kg/h 0.228 kg/h	513.25 \$/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). NA: Intersection LOS for Vehicles is Not Applicable for two-way sign control since the average intersection delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Persons
Demand Flows (Total)	648,758 veh/y	778,510 pers/y
Delay	196 veh-h/y	235 pers-h/y
Effective Stops	58,113 veh/y	69,736 pers/y
Travel Distance	659,399 veh-km/y	791,279 pers-km/y
Travel Time	11,232 veh-h/y	13,478 pers-h/y
Cost	246,361 \$/y	246,361 \$/y
Fuel Consumption	43,697 L/y	-
Carbon Dioxide	103,591 kg/y	
Hydrocarbons	8 kg/y	
Carbon Monoxide	126 kg/y	
NOx	109 kg/y	

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:39 PM Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Factory_WEnd_AM_Ex+Dev]

Giveway / Yield (Two-Way)

Move	ment Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa	Rd									
1	L2	47	1.0	0.323	5.6	LOS A	0.0	0.0	0.00	0.05	57.9
2	T1	1185	3.0	0.323	0.0	LOS A	0.0	0.0	0.00	0.02	59.7
Appro	ach	1233	2.9	0.323	0.3	NA	0.0	0.0	0.00	0.02	59.6
West:	Factory F	Rd									
10	L2	119	1.0	0.089	9.7	LOS A	0.3	2.4	0.59	0.78	50.5
Appro	ach	119	1.0	0.089	9.7	LOS A	0.3	2.4	0.59	0.78	50.5
All Vel	hicles	1352	2.8	0.323	1.1	NA	0.3	2.4	0.05	0.09	58.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:39 PM
Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Jamison_Wday_PM_Ex]

New Site

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Optimum Cycle Time - Minimum Delay)

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	33.2 km/h	2.1 km/h	32.3 km/h
Travel Distance (Total)	4317.7 veh-km/h	9.4 ped-km/h	5190.6 pers-km/h
Travel Time (Total)	130.2 veh-h/h	4.5 ped-h/h	160.8 pers-h/h
Demand Flows (Total)	4224 veh/h	211 ped/h	5280 pers/h
Percent Heavy Vehicles (Demand)	2.1 %	ZTT ped/II	0200 pers/11
Degree of Saturation	0.909	0.080	
Practical Spare Capacity	-1.0 %		
Effective Intersection Capacity	4646 veh/h		
0 1 10 1 (7 1 1)	57.04	0.50	74.07
Control Delay (Average)	57.61 veh-h/h	2.53 ped-h/h	71.67 pers-h/h
Control Delay (Average) Control Delay (Worst Lane)	49.1 sec 69.8 sec	43.3 sec	48.9 sec
Control Delay (Worst Movement)	69.8 sec	49.3 sec	69.8 sec
Geometric Delay (Average)	2.3 sec	10.0 000	00.0 000
Stop-Line Delay (Average)	46.8 sec		
Idling Time (Average)	41.3 sec		
Intersection Level of Service (LOS)	LOS D	LOS E	
95% Back of Queue - Vehicles (Worst Lane)	29.8 veh		
95% Back of Queue - Distance (Worst Lane)	213.6 m		
Queue Storage Ratio (Worst Lane) Total Effective Stops	0.26 3840 veh/h	187 ped/h	4794 pers/h
Effective Stop Rate	0.91 per veh	0.89 per ped	0.91 per pers
Proportion Queued	0.97	0.89	0.96
Performance Index	338.1	5.6	343.7
Cost (Total)	3989.07 \$/h	114.41 \$/h	4103.48 \$/h
Fuel Consumption (Total)	456.3 L/h		
Carbon Dioxide (Total) Hydrocarbons (Total)	1077.7 kg/h 0.098 kg/h		
Carbon Monoxide (Total)	1.164 kg/h		
NOx (Total)	1.176 kg/h		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Pedestrians	Persons
Demand Flows (Total)	2,027,621 veh/y	101,053 ped/y	2,534,198 pers/y
Delay	27,653 veh-h/y	1,216 ped-h/y	34,399 pers-h/y
Effective Stops	1,843,155 veh/y	89,536 ped/y	2,301,322 pers/y
Travel Distance	2,072,485 veh-km/y	4,509 ped-km/y	2,491,491 pers-km/y
Travel Time	62,509 veh-h/y	2,179 ped-h/y	77,190 pers-h/y
Cost	1,914,754 \$/y	54,915 \$/y	1,969,668 \$/y
Fuel Consumption	219,002 L/y		
Carbon Dioxide	517,308 kg/y		
Hydrocarbons	47 kg/y		
Carbon Monoxide	559 kg/y		
NOx	564 kg/y		

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:30 PM

Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101_[Mulgoa_Jamison_Wday_PM_Ex]

New Site

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Optimum Cycle Time - Minimum Delay)

Move	ement Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa I	₹d									
1	L2	20	1.0	0.322	36.9	LOS C	7.8	55.9	0.81	0.69	38.7
2	T1	1076	3.0	0.766	37.6	LOS C	22.8	163.4	0.94	0.85	37.2
3	R2	373	2.0	0.700	56.4	LOS D	10.0	71.2	1.00	0.85	31.0
Appro		1468	2.7	0.766	42.4	LOS C	22.8	163.4	0.96	0.85	35.4
East:	Jamison F										
4	L2	381	1.0	0.909	65.9	LOS E	24.2	170.9	1.00	1.00	28.4
5	T1	268	1.0	0.610	41.5	LOS C	13.1	92.4	0.95	0.80	35.9
6	R2	326	1.0	0.885	69.8	LOS E	10.0	70.9	1.00	0.99	27.8
Appro	ach	976	1.0	0.909	60.5	LOS E	24.2	170.9	0.99	0.94	29.9
North	: Mulgoa F	₹d									
7	L2	246	1.0	0.432	38.2	LOS C	10.6	74.6	0.85	0.80	36.2
8	T1	1028	3.0	0.870	47.7	LOS D	29.8	213.6	1.00	1.02	33.8
9	R2	239	1.0	0.891	67.8	LOS E	14.8	104.3	1.00	0.99	28.2
Appro	ach	1514	2.4	0.891	49.3	LOS D	29.8	213.6	0.98	0.98	33.1
West:	Jamison I	Rd									
10	L2	113	1.0	0.269	43.5	LOS D	5.0	35.4	0.87	0.77	34.4
11	T1	109	1.0	0.249	37.7	LOS C	4.8	34.1	0.86	0.69	37.3
12	R2	44	1.0	0.240	56.6	LOS E	2.3	16.0	0.96	0.74	31.0
Appro	ach	266	1.0	0.269	43.3	LOS D	5.0	35.4	0.88	0.73	34.9
All Ve	hicles	4224	2.1	0.909	49.1	LOS D	29.8	213.6	0.97	0.91	33.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov	December	Demand	Average		Average Back		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	39.4	LOS D	0.1	0.1	0.85	0.85
P3	North Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	35.3	LOS D	0.1	0.1	0.80	0.80
All Pe	destrians	211	43.3	LOS E			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com

Site: 101 [Mulgoa_Jamison_Wday_PM_Ex+Dev]

New Site

Signals - Fixed Time Isolated Cycle Time = 100 seconds (Optimum Cycle Time - Minimum Delay)

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	32.7 km/h	2.1 km/h	31.9 km/h
Travel Distance (Total)	4356.3 veh-km/h	9.4 ped-km/h	5236.9 pers-km/h
Travel Time (Total)	133.2 veh-h/h	4.4 ped-h/h	164.2 pers-h/h
Demand Flows (Total)	4262 veh/h	211 ped/h	5325 pers/h
Percent Heavy Vehicles (Demand)	2.1 %	, .	
Degree of Saturation	0.939	0.073	
Practical Spare Capacity	-4.2 %		
Effective Intersection Capacity	4538 veh/h		
Control Delay (Total)	59.90 veh-h/h	2.37 ped-h/h	74.24 pers-h/h
Control Delay (Average)	50.6 sec	40.5 sec	50.2 sec
Control Delay (Worst Lane)	69.1 sec		
Control Delay (Worst Movement)	69.1 sec	44.3 sec	69.1 sec
Geometric Delay (Average)	2.3 sec		
Stop-Line Delay (Average) Idling Time (Average)	48.3 sec 42.1 sec		
Intersection Level of Service (LOS)	LOS D	LOS E	
intersection Level of Service (LOS)	LO9 D	LU3 E	
95% Back of Queue - Vehicles (Worst Lane)	31.2 veh		
95% Back of Queue - Distance (Worst Lane)	224.0 m		
Queue Storage Ratio (Worst Lane)	0.27		"
Total Effective Stops	4112 veh/h	189 ped/h	5123 pers/h
Effective Stop Rate Proportion Queued	0.96 per veh 0.97	0.90 per ped 0.90	0.96 per pers 0.97
Performance Index	339.4	5.4	344.9
Cost (Total)	4098.05 \$/h	110.23 \$/h	4208.28 \$/h
Fuel Consumption (Total)	465.9 L/h		
Carbon Dioxide (Total) Hydrocarbons (Total)	1100.4 kg/h 0.101 kg/h		
Carbon Monoxide (Total)	1.184 kg/h		
NOx (Total)	1.207 kg/h		
` '	<u> </u>		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Pedestrians	Persons
Demand Flows (Total)	2,045,811 veh/y	101,053 ped/y	2,556,025 pers/y
Delay	28,751 veh-h/y	1,136 ped-h/y	35,638 pers-h/y
Effective Stops	1,973,562 veh/y	90,894 ped/y	2,459,169 pers/y
Travel Distance	2,091,016 veh-km/y	4,509 ped-km/y	2,513,729 pers-km/y
Travel Time	63,923 veh-h/y	2,100 ped-h/y	78,807 pers-h/y
	· ·		
Cost	1,967,063 \$/y	52,909 \$/y	2,019,972 \$/y
Fuel Consumption	223,625 L/y	-	-
Carbon Dioxide	528,209 kg/y		
Hydrocarbons	48 kg/y		
Carbon Monoxide	568 kg/y		
NOx	579 kg/y		

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:32 PM

Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Jamison_Wday_PM_Ex+Dev]

New Site

Signals - Fixed Time Isolated Cycle Time = 100 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa l										
1	L2	20	1.0	0.343	35.8	LOS C	7.3	52.6	0.83	0.70	39.2
2	T1	1076	3.0	0.817	38.7	LOS C	22.4	161.1	0.97	0.91	36.7
3	R2	373	2.0	0.678	51.3	LOS D	9.1	64.4	1.00	0.84	32.4
Appro	ach	1468	2.7	0.817	41.9	LOS C	22.4	161.1	0.97	0.89	35.6
East:	Jamison F	₹d									
4	L2	381	1.0	0.939	69.1	LOS E	23.9	168.6	1.00	1.05	27.7
5	T1	268	1.0	0.630	38.7	LOS C	12.1	85.2	0.96	0.81	36.9
6	R2	326	1.0	0.885	64.8	LOS E	9.2	65.0	1.00	1.01	28.9
Appro	ach	976	1.0	0.939	59.3	LOS E	23.9	168.6	0.99	0.97	30.2
North:	: Mulgoa F	₹d									
7	L2	246	1.0	0.461	37.1	LOS C	9.9	70.1	0.87	0.81	36.6
8	T1	1028	3.0	0.927	56.3	LOS D	31.2	224.0	1.00	1.14	31.3
9	R2	256	1.0	0.925	68.5	LOS E	15.4	108.5	1.00	1.05	28.1
Appro	ach	1531	2.3	0.927	55.3	LOS D	31.2	224.0	0.98	1.07	31.4
West:	Jamison	Rd									
10	L2	129	1.0	0.319	41.3	LOS C	5.4	37.9	0.88	0.78	35.2
11	T1	109	1.0	0.257	35.1	LOS C	4.5	31.5	0.87	0.69	38.3
12	R2	48	1.0	0.263	52.3	LOS D	2.3	16.1	0.96	0.74	32.1
Appro	ach	287	1.0	0.319	40.8	LOS C	5.4	37.9	0.89	0.74	35.7
All Ve	hicles	4262	2.1	0.939	50.6	LOS D	31.2	224.0	0.97	0.96	32.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pe	destrians						
Mov	Description	Demand 	Average		Average Bac		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P2	East Full Crossing	53	38.8	LOS D	0.1	0.1	0.88	0.88
P3	North Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P4	West Full Crossing	53	34.5	LOS D	0.1	0.1	0.83	0.83
All Pe	destrians	211	40.5	LOS E			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com

Site: 101 [Mulgoa_Jamison_WEnd_AM_Ex]

New Site

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Optimum Cycle Time - Minimum Delay)

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	29.6 km/h	2.1 km/h	29.0 km/h
Travel Distance (Total)	4715.4 veh-km/h	9.4 ped-km/h	5667.9 pers-km/h
Travel Time (Total)	159.4 veh-h/h	4.5 ped-h/h	195.7 pers-h/h
D 151 (T ())	4045	044 1/1	5740 "
Demand Flows (Total)	4615 veh/h 2.0 %	211 ped/h	5748 pers/h
Percent Heavy Vehicles (Demand) Degree of Saturation	0.989	0.080	
Practical Spare Capacity	-9.0 %	0.000	
Effective Intersection Capacity	4666 veh/h		
Emocaro interessalem supusity	1000 7011111		
Control Delay (Total)	79.96 veh-h/h	2.49 ped-h/h	98.45 pers-h/h
Control Delay (Average)	62.4 sec	42.7 sec	61.7 sec
Control Delay (Worst Lane)	95.5 sec		
Control Delay (Worst Movement)	95.5 sec	49.3 sec	95.5 sec
Geometric Delay (Average)	2.5 sec		
Stop-Line Delay (Average)	59.9 sec		
Idling Time (Average)	53.3 sec		
Intersection Level of Service (LOS)	LOS E	LOS E	
050/ Book of Ougus Vahiolog (Maret Lene)	44.5 veh		
95% Back of Queue - Vehicles (Worst Lane) 95% Back of Queue - Distance (Worst Lane)	319.6 m		
Queue Storage Ratio (Worst Lane)	0.39		
Total Effective Stops	4648 veh/h	185 ped/h	5763 pers/h
Effective Stop Rate	1.01 per veh	0.88 per ped	1.00 per pers
Proportion Queued	0.96	0.88	0.96
Performance Index	418.9	5.5	424.4
Cost (Total)	4944.07 \$/h	113.45 \$/h	5057.53 \$/h
Fuel Consumption (Total)	523.2 L/h		
Carbon Dioxide (Total)	1235.6 kg/h		
Hydrocarbons (Total)	0.115 kg/h 1.310 kg/h		
Carbon Monoxide (Total) NOx (Total)	1.310 kg/h 1.302 kg/h		
NON (Total)	1.502 kg/11		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Pedestrians	Persons
Demand Flows (Total)	2,215,074 veh/y	101,053 ped/y	2,759,142 pers/y
Delay	38,382 veh-h/y	1,197 ped-h/y	47,256 pers-h/y
Effective Stops	2,231,137 veh/y	88,846 ped/y	2,766,210 pers/y
Travel Distance	2,263,413 veh-km/y	4,509 ped-km/y	2,720,605 pers-km/y
Travel Time	76,493 veh-h/y	2,161 ped-h/y	93,953 pers-h/y
	· ·		
Cost	2,373,156 \$/y	54,458 \$/y	2,427,613 \$/y
Fuel Consumption	251,152 L/y	-	-
Carbon Dioxide	593,080 kg/y		
Hydrocarbons	55 kg/y		
Carbon Monoxide	629 kg/y		
NOx	625 kg/y		

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:34 PM

Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Jamison_WEnd_AM_Ex]

New Site

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa F	₹d									
1	L2	74	1.0	0.298	35.9	LOS C	7.2	51.3	0.80	0.71	38.4
2	T1	966	3.0	0.708	35.1	LOS C	20.4	146.6	0.93	0.81	38.1
3	R2	431	2.0	0.924	74.3	LOS F	14.0	99.6	1.00	1.05	27.0
Appro		1471	2.6	0.924	46.6	LOS D	20.4	146.6	0.94	0.87	34.0
East:	Jamison F										
4	L2	445	1.0	0.984	89.7	LOS F	34.2	241.3	1.00	1.11	24.0
5	T1	245	1.0	0.516	38.9	LOS C	11.5	81.0	0.92	0.77	36.8
6	R2	332	1.0	0.989	95.5	LOS F	12.3	86.7	1.00	1.16	23.3
Appro	ach	1022	1.0	0.989	79.4	LOS F	34.2	241.3	0.98	1.04	25.9
North	: Mulgoa F	₹d									
7	L2	320	1.0	0.545	38.9	LOS C	14.2	100.3	0.88	0.82	36.0
8	T1	1148	3.0	0.975	75.8	LOS F	44.5	319.6	1.00	1.26	26.8
9	R2	228	1.0	0.973	88.6	LOS F	16.5	116.4	1.00	1.12	24.3
Appro	ach	1697	2.4	0.975	70.6	LOS F	44.5	319.6	0.98	1.16	27.8
West:	Jamison F	₹d									
10	L2	196	1.0	0.433	43.6	LOS D	9.0	63.2	0.89	0.80	34.4
11	T1	165	1.0	0.348	37.0	LOS C	7.4	51.9	0.87	0.71	37.5
12	R2	64	1.0	0.383	58.7	LOS E	3.4	24.0	0.98	0.76	30.4
Appro	ach	425	1.0	0.433	43.3	LOS D	9.0	63.2	0.90	0.76	34.8
All Ve	hicles	4615	2.0	0.989	62.4	LOS E	44.5	319.6	0.96	1.01	29.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians											
Mov	Description	Demand 	Average		Level of Average Back of Queue			Effective				
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	South Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95				
P2	East Full Crossing	53	38.6	LOS D	0.1	0.1	0.84	0.84				
P3	North Full Crossing	53	48.3	LOS E	0.2	0.2	0.94	0.94				
P4	West Full Crossing	53	34.5	LOS D	0.1	0.1	0.79	0.79				
All Pedestrians		211	42.7	LOS E			0.88	0.88				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com

Site: 101 [Mulgoa_Jamison_WEnd_AM_Ex+Dev]

New Site

Signals - Fixed Time Isolated Cycle Time = 125 seconds (Optimum Cycle Time - Minimum Delay)

Vehicles	Pedestrians	Persons
29.2 km/h 4754.0 veh-km/h 163.1 veh-h/h	2.0 km/h 9.4 ped-km/h 4.7 ped-h/h	28.5 km/h 5714.3 pers-km/h 200.4 pers-h/h
		,
4653 veh/h 2.0 % 0.978 -8.0 % 4757 veh/h	211 ped/h 0.069	5794 pers/h
83.00 veh-h/h 64.2 sec 98.2 sec	2.70 ped-h/h 46.2 sec	102.30 pers-h/h 63.6 sec
98.2 sec 2.6 sec 61.7 sec 55.8 sec	54.9 sec	98.2 sec
LOS E	LOS E	
48.8 veh 350.2 m 0.43 4504 veh/h 0.97 per veh 0.96 445.2	181 ped/h 0.86 per ped 0.86 5.7	5585 pers/h 0.96 per pers 0.95 451.0
5061.05 \$/h 528.4 L/h 1247.8 kg/h 0.117 kg/h 1.322 kg/h 1.299 kg/h	118.68 \$/h	5179.73 \$/h
	29.2 km/h 4754.0 veh-km/h 163.1 veh-h/h 4653 veh/h 2.0 % 0.978 -8.0 % 4757 veh/h 83.00 veh-h/h 64.2 sec 98.2 sec 98.2 sec 2.6 sec 61.7 sec 55.8 sec LOS E 48.8 veh 350.2 m 0.43 4504 veh/h 0.97 per veh 0.96 445.2 5061.05 \$/h 528.4 L/h 1247.8 kg/h 0.117 kg/h 1.322 kg/h	29.2 km/h 4754.0 veh-km/h 163.1 veh-h/h 163.1 veh-h/h 4653 veh/h 2.0 % 0.978 0.069 -8.0 % 4757 veh/h 83.00 veh-h/h 64.2 sec 98.2 sec 98.2 sec 98.2 sec 54.9 sec 1.7 sec 55.8 sec LOS E LOS E 48.8 veh 350.2 m 0.43 4504 veh/h 0.97 per veh 0.96 0.96 445.2 5.7 5061.05 \$/h 118.68 \$/h 1.322 kg/h

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Performance Measure	Vehicles	Pedestrians	Persons
Demand Flows (Total)	2,233,263 veh/y	101,053 ped/y	2,780,969 pers/y
Delay	39,839 veh-h/y	1,297 ped-h/y	49,104 pers-h/y
Effective Stops	2,161,938 veh/y	86,691 ped/y	2,681,018 pers/y
Travel Distance	2,281,944 veh-km/y	4,509 ped-km/y	2,742,842 pers-km/y
Travel Time	78,266 veh-h/y	2,260 ped-h/y	96,180 pers-h/y
	· ·		
Cost	2,429,304 \$/y	56,965 \$/y	2,486,269 \$/y
Fuel Consumption	253,649 L/y	-	-
Carbon Dioxide	598,962 kg/y		
Hydrocarbons	56 kg/y		
Carbon Monoxide	635 kg/y		
NOx	624 kg/y		

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: POSITIVE TRAFFIC PTY LTD | Processed: Thursday, 5 December 2019 6:00:35 PM

Project: Z:\2019 Projects\PT19065 - 44-55 Tench Avenue Jamistown\SIDRA\PT19065.sip7

Site: 101 [Mulgoa_Jamison_WEnd_AM_Ex+Dev]

New Site

Signals - Fixed Time Isolated Cycle Time = 125 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	rformance	- Vehic	les							
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	: Mulgoa I										
1	L2	74	1.0	0.289	38.9	LOS C	8.0	57.2	0.79	0.71	37.3
2	T1	966	3.0	0.687	38.5	LOS C	22.7	163.2	0.91	0.80	36.8
3	R2	431	2.0	0.864	72.4	LOS F	14.5	103.2	1.00	0.96	27.3
Appro	ach	1471	2.6	0.864	48.4	LOS D	22.7	163.2	0.93	0.84	33.4
East:	Jamison F	₹d									
4	L2	445	1.0	0.974	91.5	LOS F	36.6	258.6	1.00	1.07	23.7
5	T1	245	1.0	0.510	43.7	LOS D	12.9	91.2	0.91	0.77	35.1
6	R2	332	1.0	0.937	86.0	LOS F	12.2	86.3	1.00	1.04	24.8
Appro	ach	1022	1.0	0.974	78.3	LOS F	36.6	258.6	0.98	0.99	26.1
North	: Mulgoa F	₹d									
7	L2	320	1.0	0.529	42.2	LOS C	15.8	111.7	0.87	0.82	34.9
8	T1	1148	3.0	0.966	76.9	LOS F	48.8	350.2	1.00	1.20	26.6
9	R2	245	1.0	0.978	98.2	LOS F	19.9	140.7	1.00	1.10	22.9
Appro	ach	1714	2.3	0.978	73.5	LOS F	48.8	350.2	0.98	1.12	27.2
West:	Jamison I	Rd									
10	L2	213	1.0	0.465	48.7	LOS D	11.1	78.2	0.90	0.81	32.8
11	T1	165	1.0	0.344	41.6	LOS C	8.3	58.5	0.87	0.71	35.9
12	R2	68	1.0	0.387	64.8	LOS E	4.1	28.7	0.98	0.76	29.0
Appro	ach	446	1.0	0.465	48.6	LOS D	11.1	78.2	0.90	0.77	33.2
All Ve	hicles	4653	2.0	0.978	64.2	LOS E	48.8	350.2	0.96	0.97	29.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov	Description	Demand	Average	Level of Average Back of Queue			Prop.	Effective			
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate			
		ped/h	sec		ped	m		per ped			
P1	South Full Crossing	53	54.9	LOS E	0.2	0.2	0.94	0.94			
P2	East Full Crossing	53	40.9	LOS E	0.2	0.2	0.81	0.81			
P3	North Full Crossing	53	52.1	LOS E	0.2	0.2	0.91	0.91			
P4	West Full Crossing	53	36.9	LOS D	0.1	0.1	0.77	0.77			
All Pedestrians		211	46.2	LOS E			0.86	0.86			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 7.0 | Copyright © 2000-2017 Akcelik and Associates Pty Ltd | sidrasolutions.com

10. Appendix D – Service Vehicle Area Turning Path Analysis

