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202701-CR01-C 28-32 Somerset Street, Kingswood – Stormwater Management Report

Rev	Description	Prepared by	Reviewed by	Issue Date	Client App	Approval Date
Α	Issued for Information	AC	PC	10/11/20		
В	Issued for Development Application	AC	PC	13/11/20		
С	Issued for Development Application	SH	PC	22/07/21		



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1. Introduction

Northrop Consulting Engineers (Northrop) has been engaged by Boston Global to prepare documentation in support of a Development Application (DA) Submission to Penrith City Council (PCC) for the proposed residential development at 28-32 Somerset Street, Kingswood.

The proposed development will involve the demolition of an existing residential dwellings within the subject site and the construction of a 4-star hotel.

Northrop has been engaged to prepare a Stormwater Management Report (and accompanying plans) for the proposed development.

The report outlines the stormwater management strategy developed for managing stormwater runoff from the proposed development, so to document that the proposed concepts meet Council's specifications and requirements within the 2014 DCP as well as Stormwater Drainage for Building Developments, and relevant referenced report from Penrith City Council – College, Orth and Werrington Creeks Catchment Overland Flow Flood Study-Final Report-Volume 2 of 2: Figures.

This report should be read in conjunction with Northrop's prepared civil DA drawing set 202701 DA1.01-DA6.02.



1.1. Existing Site Description

The address of the subject site is 28-32 Somerset Street, Kingswood. The site is located within B4 mixed use land zoning area. Refer to **Figure 1** for the site location.



Figure 1 - Locality Plan

The site is generally trapezoidal in shape and covers an area of approximately 1694 m². The site is enclosed by Somerset Street on the western boundary and Hargrave Street on the southern boundary.

The existing (pre-development) site condition consists of a single storey dwelling located on 28 Somerset Street whilst 30 and 32 Somerset Street are vacant.

Based on survey information, the general site levels fall from a maximum RL of approximately 49.10 m AHD along the western boundary to a minimum RL of approximately 47.58 m AHD on the eastern boundary constituting an average grade of 4.4%. Refer to Attachment A for the existing site survey plan.

1.2. Proposed Development

The proposed development will involve the demolition of 1 existing residential dwelling within the subject site and the construction of a 4 star hotel.





Figure 2 - Proposed development 3d view

Refer to the architectural drawings prepared by Rothelowman Architecture for more details.

1.3. Existing Stormwater Infrastructure

Council has confirmed that there is no existing stormwater infrastructure (i.e. below ground pit and pipe system) present within the roadways fronting the site and that the nearest council stormwater asset is approximately 80m east along Hargrave Street. Please refer to Northrop's civil DA drawings for further details.

For further information on services such as electrical, sewer, water and gas refer to other disciplines.

2.4 Existing Services & Utilities

Based on a Dial Before You Dig assessment, the following services & utilities are available for the proposed development;

- Jemena Gas
- NBN Underground Fibre Optic Cable
- Sydney Water (sewer and water)
- Telstra



2.5 Ground Water

Based on the geotechnical report prepared by Douglas Partners, groundwater will not be an issue as the lowest basement level is above the groundwater table. Tanking of the basement is not expected. Pump out system has been provided for basement level to collect any seepage water along with uncovered driveway runoff to basement and it has been designed in accordance with Council's Stormwater Drainage Specification for Building Developments.



2. Overland Flow Study

The site falls within the South Creek Catchment at the upper end of the Werrington Creek subcatchment. Referring to the South Creek Flood Study, we have concluded the site is not affected by flooding due to upwelling of natural water courses or stormwater channels.

The topography of lands surrounding is relatively flat avoiding landforms that would concentrate surface stormwater runoff. Thus, in this basis we believe it is unlikely the site is affected by overland flow.

3. Stormwater Management

The stormwater Management measures have been designed to comply with the following guidelines:

- Australian Rainfall and Runoff;
- Penrith City Council's Development Control Plan (DCP), 2014; and
- Penrith City Council's Stormwater Drainage for Building Developments

3.1. Proposed Stormwater Drainage Network

Stormwater generated across the site will be captured and conveyed across the site via an inground stormwater pit and pipe network. The pit & pipe arrangement will collect/convey site runoff via a 225 diameter outlet pipe to a proposed kerb inlet pit in front of the site and then across Hargrave Street to a proposed kerb inlet pit on the southern side of the road where the stormwater will then be conveyed by a proposed 375 diameter pipe to an existing stormwater pit located in front of No. 10 Hargrave Street, Kingwood. There is a small amount of bypass from the development, with approximately 50m² which is drained via overland flow to Somerset Street. A basement pump-out system is operating for the site drainage system to collect driveway runoff less than 100m².

Runoff from roof areas (i.e areas not accessible to the building occupants) will be directed to rainwater tanks where they will be utilised for re-use opportunities around the site. Three 5000 Litre rainwater tanks are proposed.

The majority of the site, including the rainwater tank overflow, along with the uncovered driveway runoff and landscape areas, will be directed to the proposed water quality chamber to be treated, prior to discharging to existing drainage system. The proposed water quality chamber will be located in landscape area in between bicycle parking and entrance of the building on Hargrave Street.

4. Minor/Major Drainage System.

The major/minor approach to stormwater drainage is the recognised drainage concept for urban catchments within the Penrith City Council Local Government Area. The ILSAX method has been used to model the hydrologic and hydraulic characteristics of stormwater runoff and flow across



the site using DRAINS. The rainfall data adopted for the stormwater design were based on 2016 data from Australian Runoff and Rainfall data hub.

The minor drainage system proposed is comprised of below ground pit and pipe network and is designed to control nuisance flooding and enable effective stormwater management for the site. Council's DCP requires that the minor drainage system be designed to convey the 20-year ARI storm event for surface/piped drainage and 100-year ARI storm to be conveyed via overland flowpaths. Designs for roof drainage will be undertaken as either conventional or siphonic drainage by a qualified and certified Hydraulic Engineer during the detailed design stage of works.

The major system also exists to cater for minor drainage system failures. In accordance with council requirements, the major drainage system is to be designed in a manner that ensures that personal safety is not compromised. An emergency overflow pit has been showed and designed for the 1% AEP storm event in case pits and pipes at the frontage of the development along Somerset Street are blocked. Any areas that cannot be provided with an overland flow path will have a system designed for the 1% AEP storm event. No water will pond around the sag pit 01/02 during a major storm event.

For the purposes of modelling, the rainwater tanks are taken to be full during simulation and added to Cat1-Roof to simulate the roof run-off. All paved and landscaped areas are also collected with either pits or grated drains. Iterations were performed in the DRAINS model to determine the size of the proposed piped network in order to satisfy the major/minor system requirements. (Refer to DRAINS model).

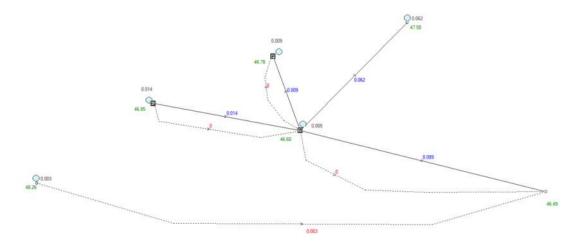


Figure 3 - Overall Site Pit & Pipe Network for 1%AEP



5. Downstream Network Modelling

As the site naturally falls to the rear boundary (i.e. away from Somerset Street towards Orth Street) and the proposed site drainage for development drains the site away from its natural sub-catchment towards Hargrave Street, Council have requested an HGL (hydraulic grade line) analysis to demonstrate that the capacity of the existing drainage system in Hargrave Street will not be adversely affected by the development for the 5% AEP storm event.

Figure 4 shows the existing Council stormwater drainage infrastructure. The existing flows from upstream Hargrave Street contribute to the sag point before draining to the corner of Orth Street and Bringelly Road.



Figure 4 - Existing Drainage System in Hargrave Street

5.1. Catchment Analysis

As discussed with Council Senior Engineer, Joshua Hull, on 20th July 21 with regards to modelling the existing downstream network, it was agreed that it was acceptable to only model the existing stormwater network upstream from the corner of Hargrave Street (adjacent to the drainage reserve) and not to where it connects to Orth Street. This approach has been taken as the downstream network is flooded in the 5%AEP.

Figure 5 is from Penrith City Council – College, Orth and Werrington Creeks Catchment Overland Flow Flood Study-Final Report. It shows that flood water depth for the 5% AEP flood between Somerset Street and Bringelly Road



As can be seen the corner of Hargrave Street is flood affected with a water depth of approximately 0.2m. Based on ground survey information it is estimated that the 5% AEP flood level is approximately 43.77m AHD. This level has been adopted as the downstream tailwater condition in the DRAINS model.



Figure 5 - Peak Design Floodwater Depths for the 5% AEP Flood

Source : College, Orth and Werrington Creeks Catchment Overland Flow Flood Study-Final Report- Volume 2 of 2

Two drains models were set up to analyse the HGL from the receiving drainage system in Hargrave Street to the point where it connects prior to downstream drainage reserve for pre and post development conditions for 5% AEP. The models were based on five sub catchments draining to Hargrave Street. The catchments that drain to Hargrave Street are represented in the Figure 6 below. Refer to Table 1 for catchment areas summary for existing drainage system.

Table 1 - Catchment Areas for existing drainage system

Contributing catchment	C1	C2	C3	C4	C5
Area (Ha)	0.4539	0.2833	0.8824	0.3214	0.1202





Figure 6 - Catchment Plan

Figures 7 and 8 below show the resulting pipe and gutter flows from the catchments for the pre and post developed conditions in Hargraves Street.

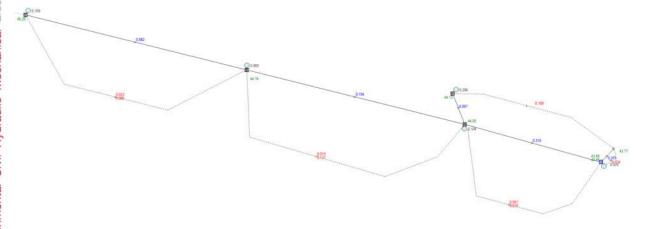


Figure 7 - DRAINS Model Schematic - Pre Developed 5% AEP



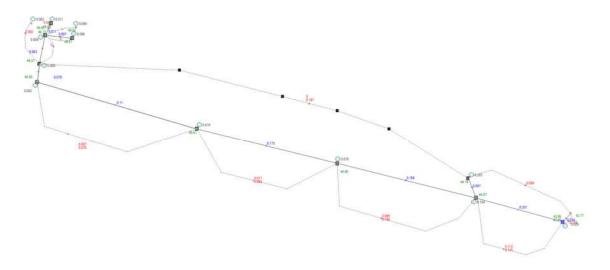


Figure 8 - DRAINS Model Schematic - Post Developed 5% AEP

A review of the results from the 5% AEP analysis of the pre and post developed models shows that the increase in stormwater bypass flows caused by the site discharge is negligible with a maximum increase in gutter flow depths of 21mm observed between Pit 4 and Pit 5, from a pre developed depth of 101mm to a post developed depth of 122mm. The DxV increases from 0.07 to 0.10 sq.m/sec and the maximum flow width increases from 2.5m to 3.2m. Therefore the overland flow route is considered safe as DxV is less than maximum safe depth x velocity in the gutter of 0.6 sq.m/sec.

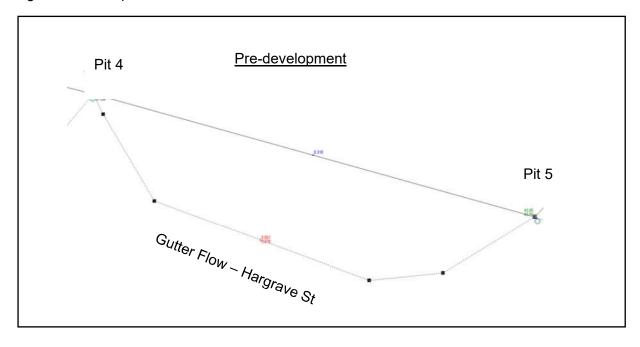


Figure 9 - DRAINS results between pit 4 and 5 for Pre Developed 5% AEP



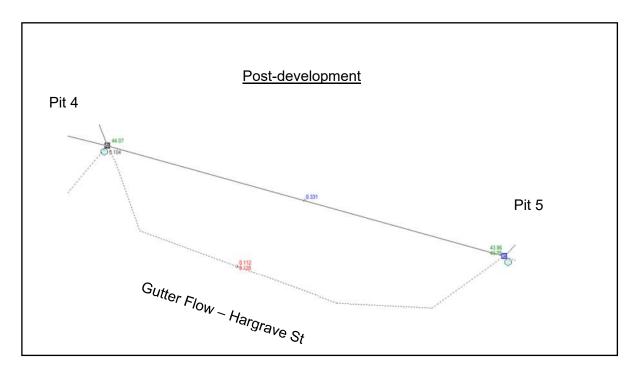


Figure 10 - DRAINS results between pit 4 and 5 for Post Developed 5% AEP

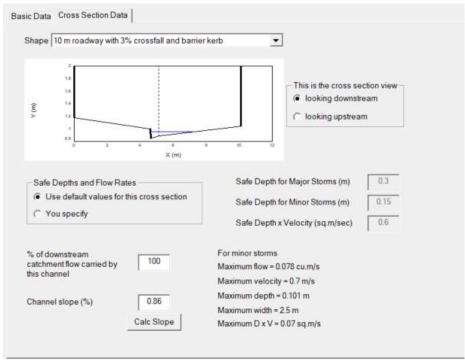


Figure 11 - DRAINS cross section results between Pit 4 and 5 for Pre Developed 5% AEP



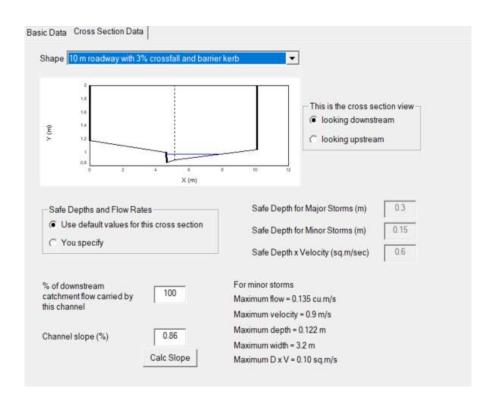


Figure 12 - DRAINS cross section results between Pit 4 and 5 for Post Developed 5% AEP

6. OSD Requirements

On-site detention is not required for the proposed site based on Penrith City Council's Stormwater Drainage for Building Developments document. In this case, stormwater is to discharge to the proposed kerb inlet pits in Hargrave Street as described in section 4.1.

For more details refer to the civil DA drawings.

7. Stormwater Quality

The stormwater quality management measures have been designed to comply with the following guidelines:

- Penrith City Council's Water Sensitive Urban Design Policy;
- Penrith City Council's Water Sensitive Urban Design Technical Guidelines

5.1 Music Modelling

The MUSIC software package was used to assess the extent of pollutant discharged from the site. The effectiveness of the proposed "treatment train" has been assessed based on modelling of the post development conditions with treatment measures



To appropriately manage the volume of pollutants discharged from the site, a treatment train will need to be developed to capture and remove as much of the pollutants from the site before they are discharged to the street;

- Building Runoff via first flush devices to rainwater reuse tank;
- Rainwater Reuse Tank;
- Stormfilter 4 x cartridge system (Ocean Protect); and
- 2 x Ocean Guard (Ocean Protect) filter basket;

The results from the MUSIC model of the site under proposed conditions with the described treatment devices are presented in *Table 2* below.

Table 2 - MUSIC modelling results of the Subject Site under Post Developed Conditions

Pollutants	% Reduction Under Proposed Conditions	% Reduction Targets required by Council
Gross Pollutants (GP)	96.8%	90%
Total Suspended Solids (TSS)	86%	85%
Total Phosphorous (TP)	78.3%	60%
Total Nitrogen (TN)	58.9%	45%

Refer to Figure 13 for the MUSIC modelling results for the proposed development.

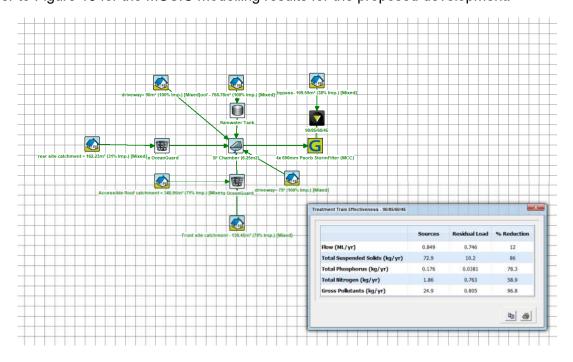


Figure 13 - MUSIC screenshot of Modelling Results of the Site Area under Proposed Development Conditions

202701-CR01 STORMWATER MANAGEMENT REPORT



The results in *Table 2* show that the implementation of the proposed treatment devices within the treatment train can effectively capture and remove a sufficient amount of pollutants from the site. The results demonstrate that the proposed treatment train can effectively reduce the total volume of pollutant discharged from the site under proposed conditions to ensure they meet Council's requirements of:

Gross Pollutants (GP) Reduction: 90%

Total Suspended Solids (TSS) Reduction: 85%

Total Phosphorus (TP) Reduction: 60%

Total Nitrogen (TN) Reduction: 45%

7.1.1. PROPOSED STORMWATER TREATMENT TRAIN

In order to achieve the reduction targets presented in Section 5.1, the following treatment devices are required as part of the treatment train:

Ocean Protect OceanGuard 200 inserts

A total of 2 x Ocean Protect OceanGuard 200 inserts will be used as pre-treatment for stormwater runoff to capture litter and coarse sediment from part of the site. The following capture rates have been adopted for the MUSIC model, based on information provided by Stormwater360:

0	TSS	85%
0	TN	45%
0	TP	60%
0	GP	90%

15kL Rainwater Tank

A 15kL rainwater tank will be implemented to capture stormwater runoff generated off the roof of the building (approx. 768.78m²). The collected rainwater will be used for irrigation of the landscaped areas across the site.

The reuse rate adopted for the rainwater 0.4kL/m²/year as per Penrith's WSUD Technical Guidelines (2015).

According to the MUSIC model, the percentage of non-potable demand met is 91.96%.

A draft operation and maintenance Schedule for the above treatment devices is presented in Appendix C.



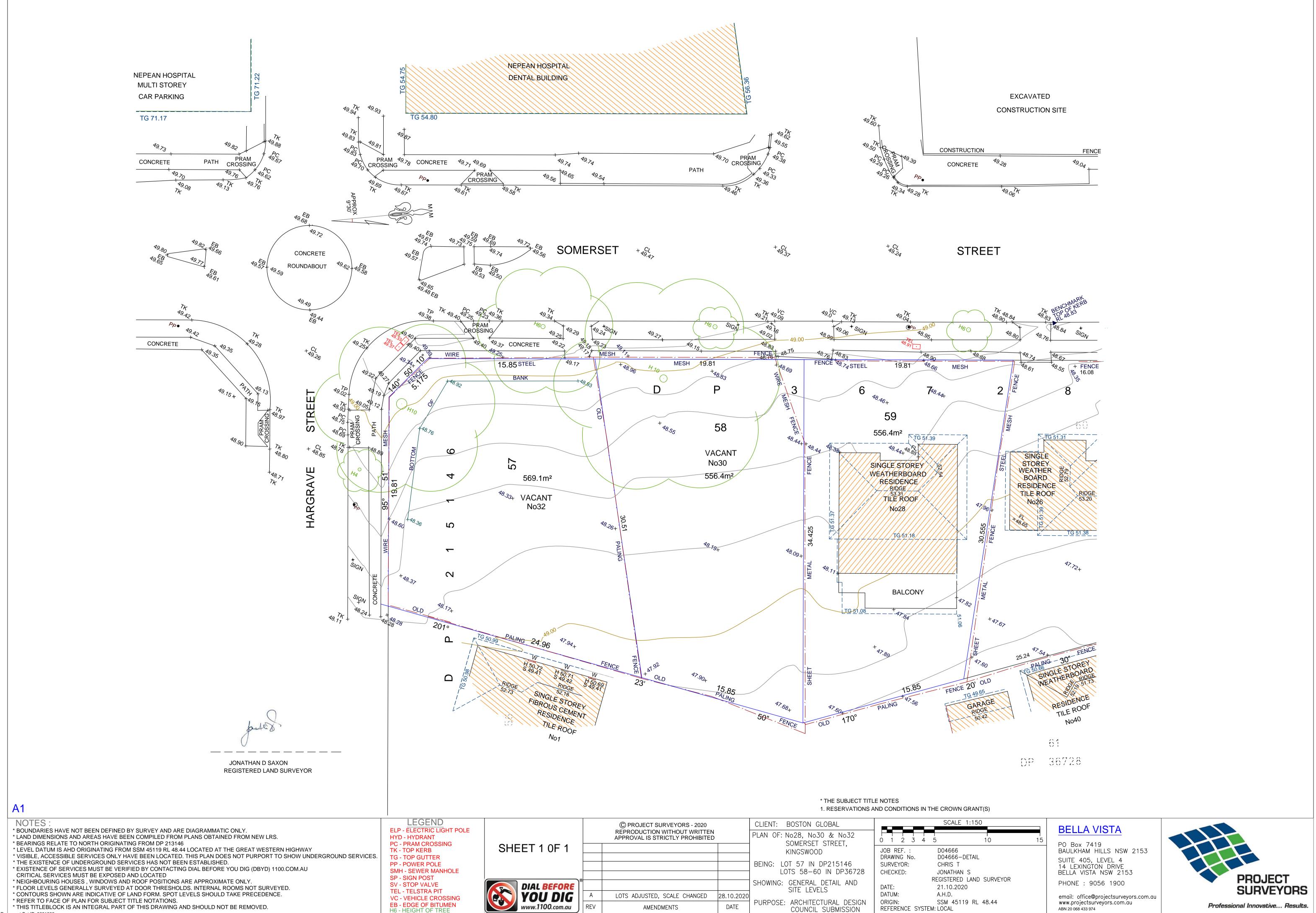
8. Conclusion

Northrop Consulting Engineers has prepared this report and the corresponding drawings to provide information to Penrith City Council on the stormwater management requirements for the development to assist Council in assessing the Development Application.

The findings of this report and associated concept designs indicates effective stormwater management measures can be integrated into the proposed development, in accordance with the Penrith City Council's engineering standards, and that no major factors relating to stormwater management would preclude the proposed development of the site.



Attachment A - Site Survey



Document Set ID: 9681866 Version: 1, Version Date: 02/08/2021



Attachment B - Civil Design DA Package

28 - 32 SOMERSET STREET, KINGSWOOD

CONCEPT STORMWATER MANAGEMENT PLAN CIVIL ENGINEERING PACKAGE - DEVELOPMENT APPLICATION





CIVIL DRAWING SCHEDULE

CONCEPT SEDIMENT EROSION CONTROL PLAN

SITEWORKS & STORMWATER MANAGEMENT PLAN - BASEMENT 3

DRIVEWAY LONGITUDINAL SECTIONS

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ISSUED FOR INFORMATION 02 ISSUED FOR DEVELOPMENT APPLICATION 03 RE-ISSUED FOR DEVELOPMENT APPLICATION

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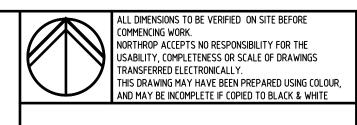
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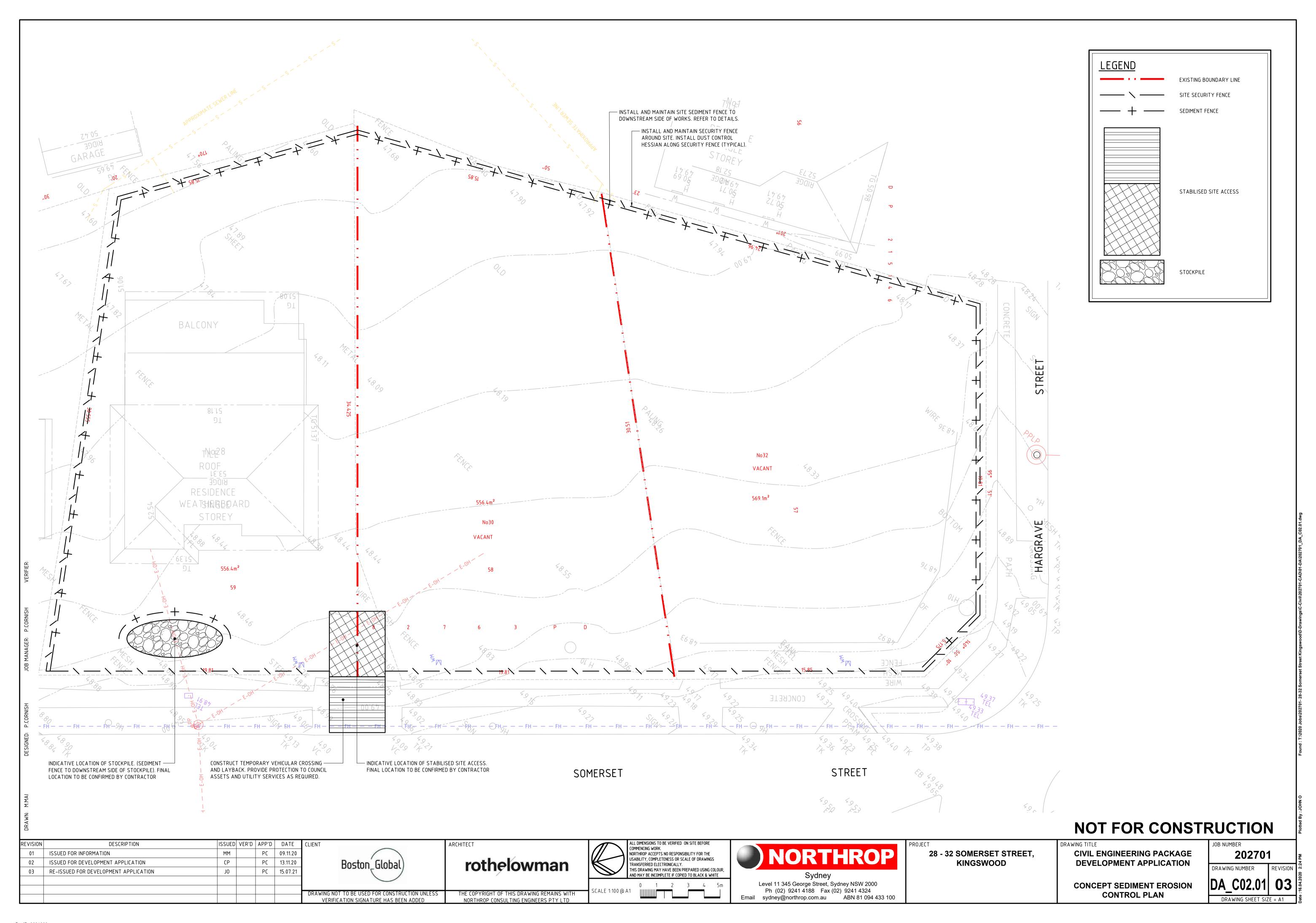
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CIVIL ENGINEERING PACKAGE **DEVELOPMENT APPLICATION**

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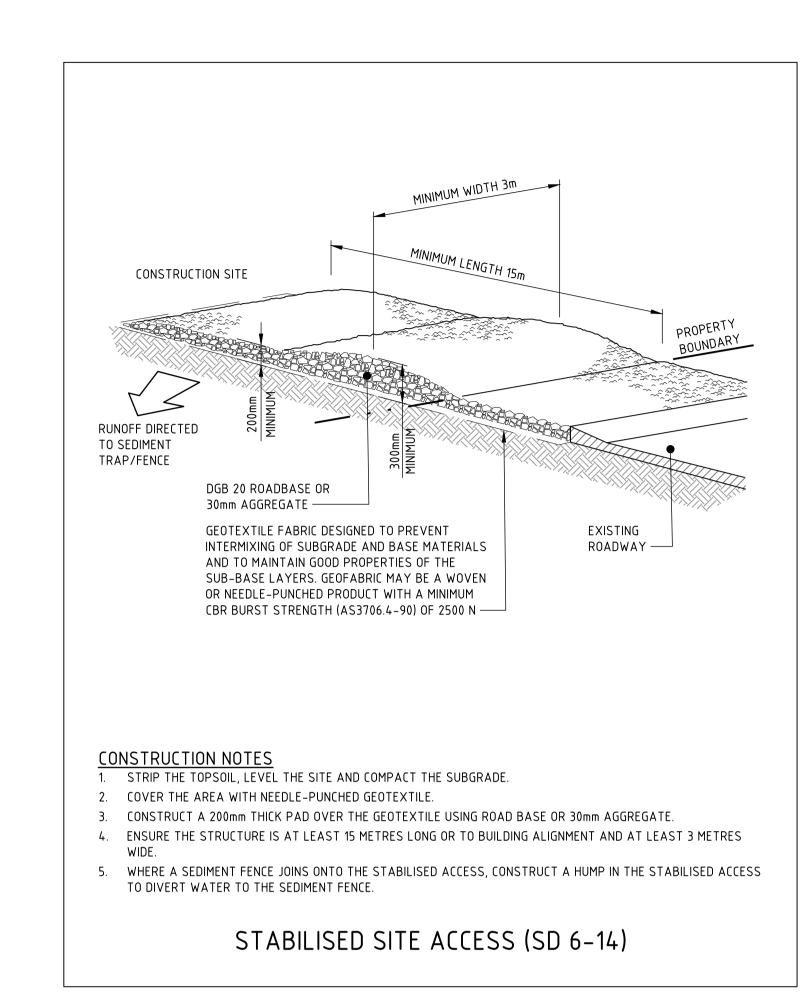
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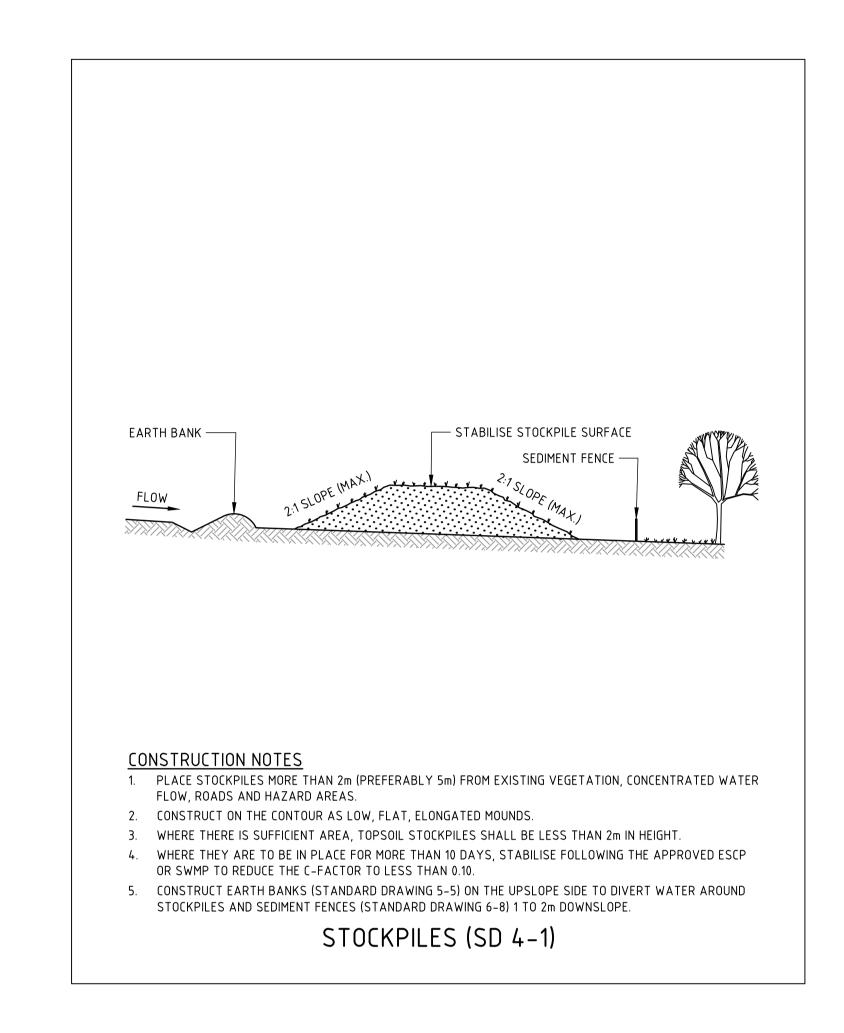


- 3. DRIVE 1.5 METRE LONG STAR PICKETS INTO GROUND AT 2.5 METRE INTERVALS (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
- FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

SEDIMENT FENCE (SD 6-8)

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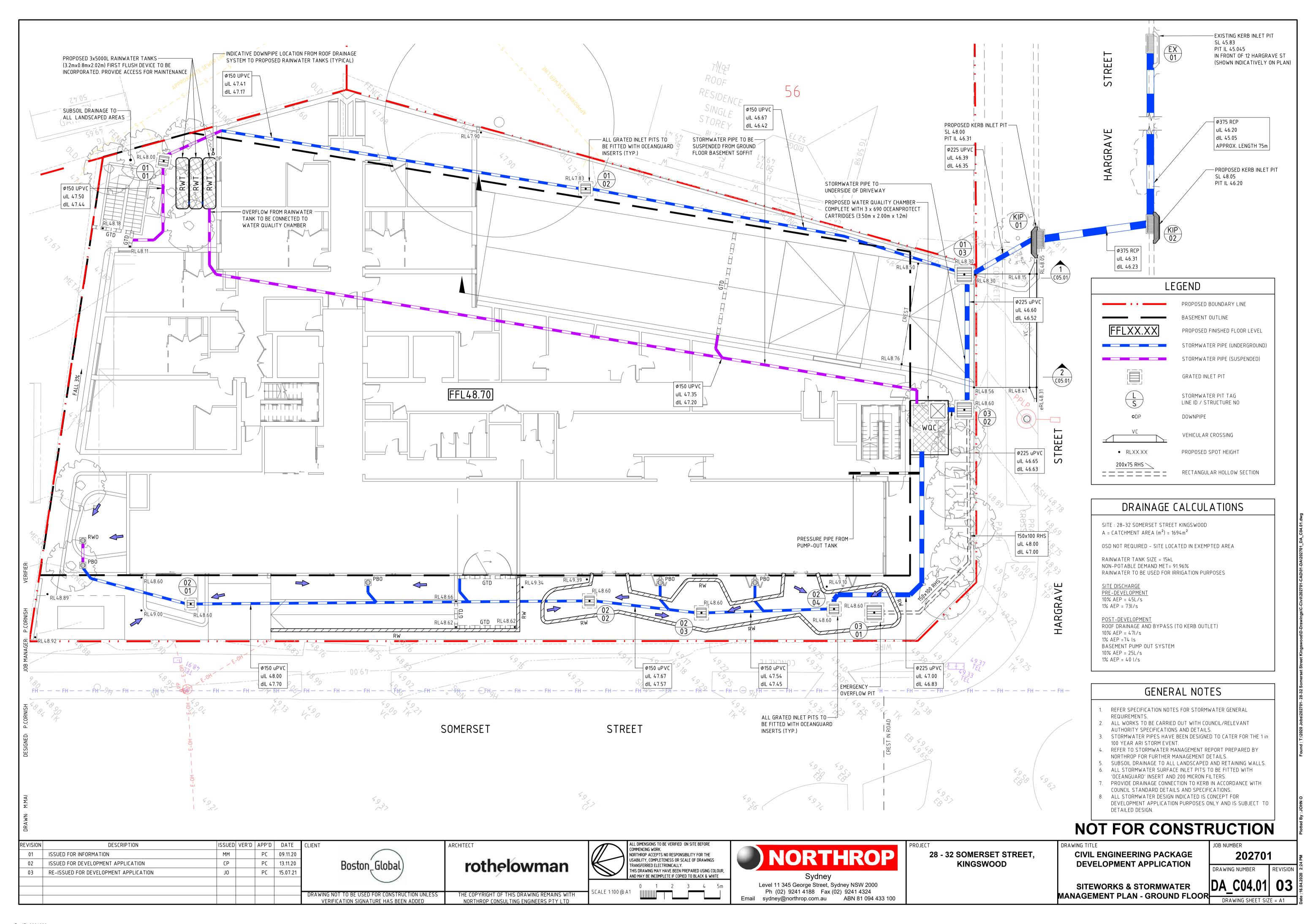
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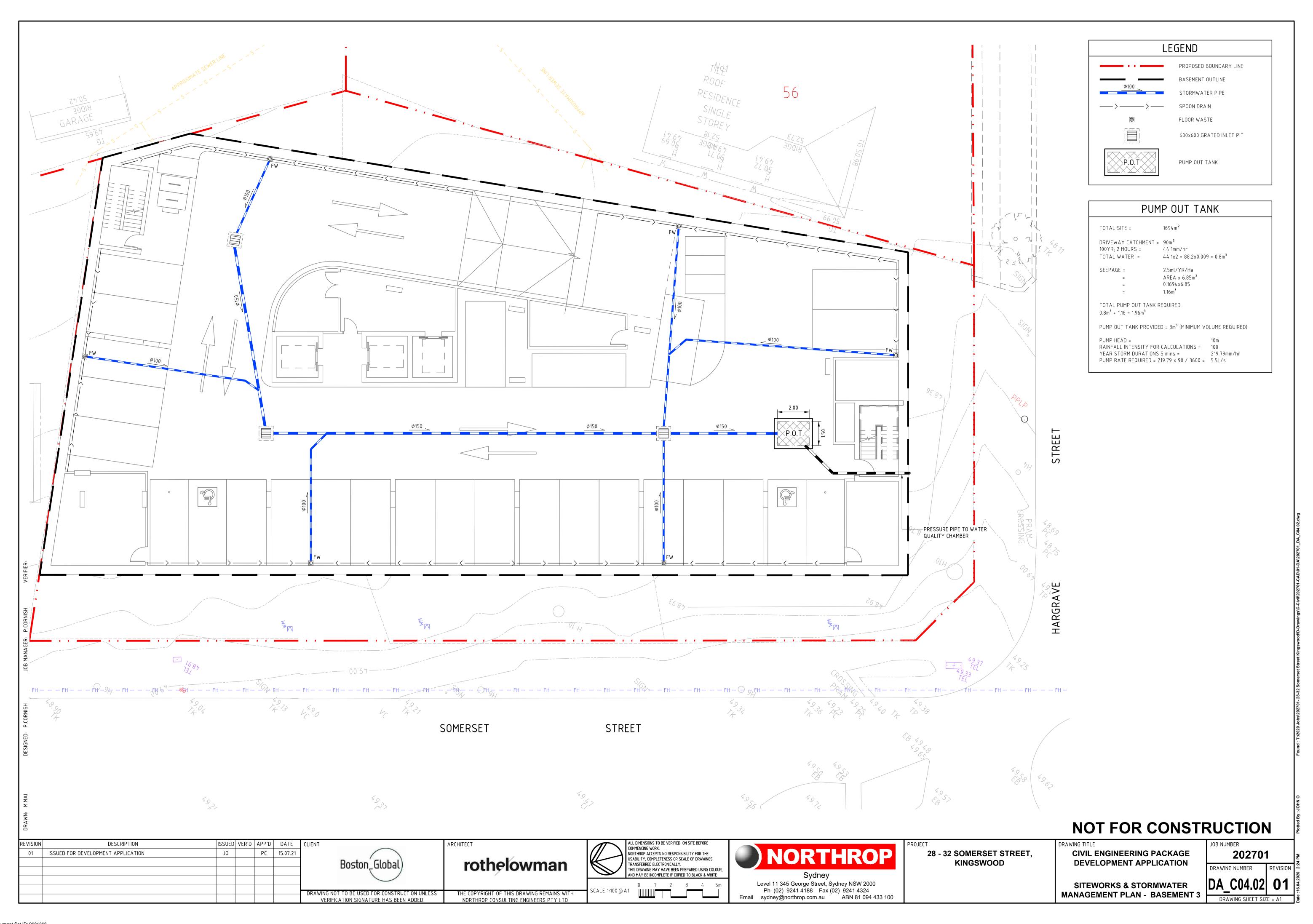
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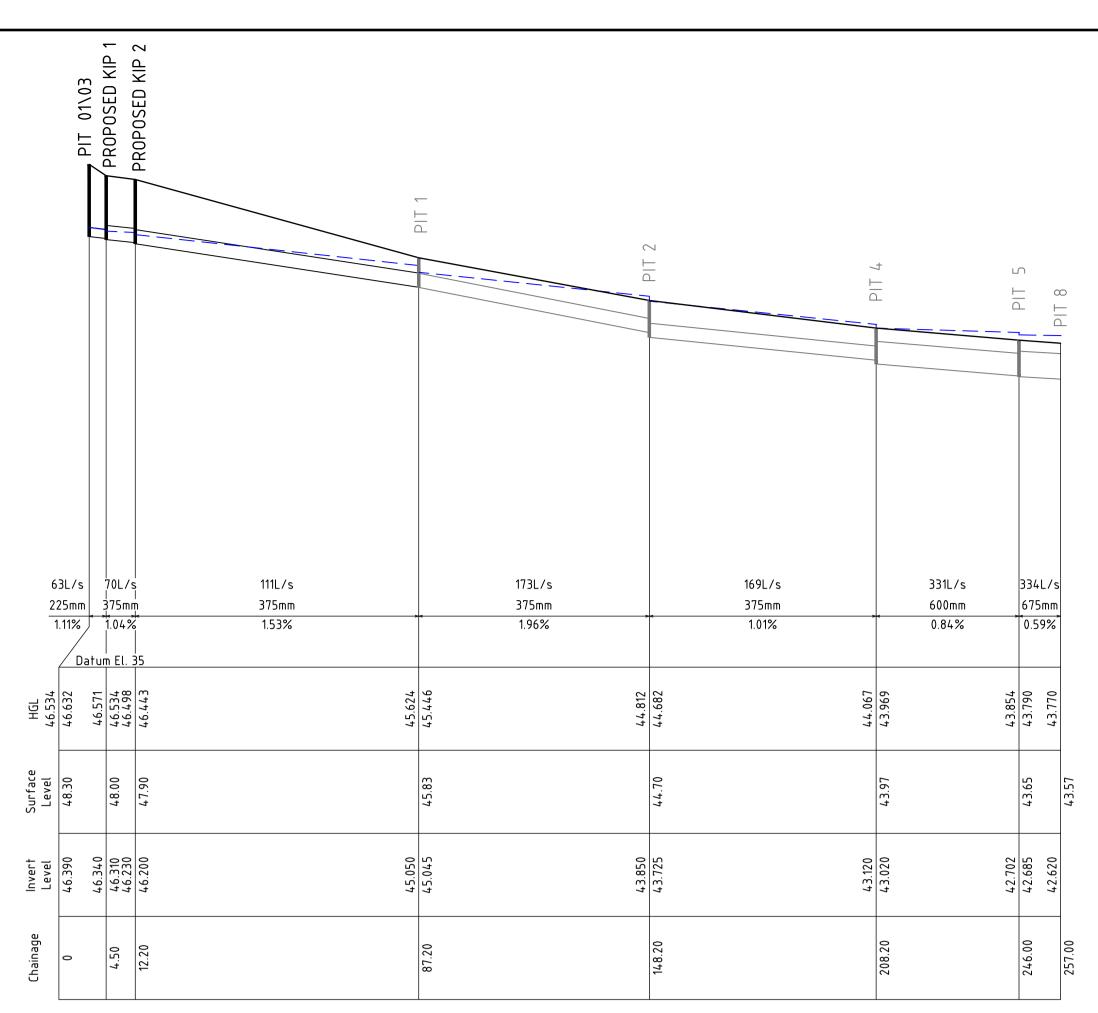
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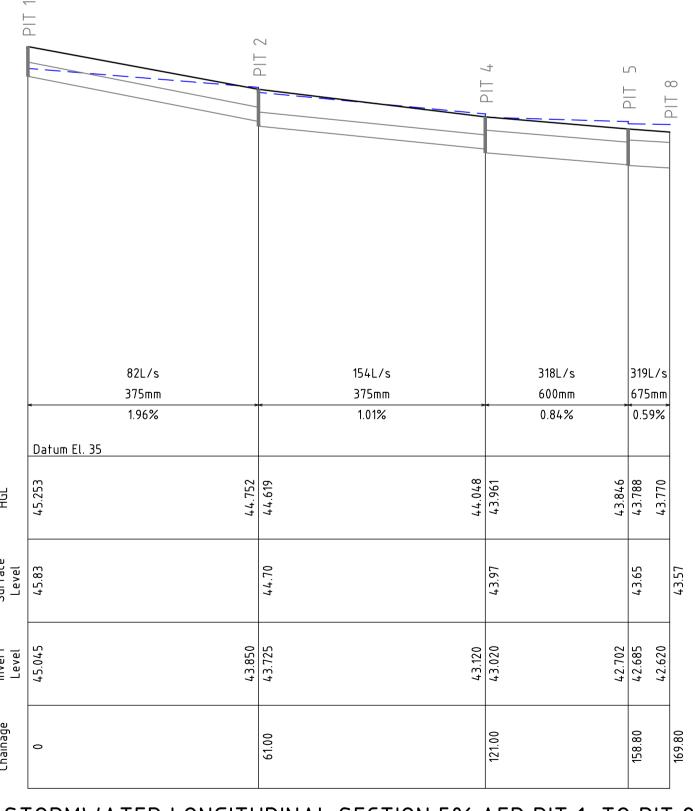
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STORMWATER LONGITUDINAL SECTION 5% AEP PIT 01\03 TO PIT 8 (POST DEVELOPED)



STORMWATER LONGITUDINAL SECTION 5% AEP PIT 1 TO PIT 8 (PRE DEVELOPED)

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STORMWATER LONGITUDINAL

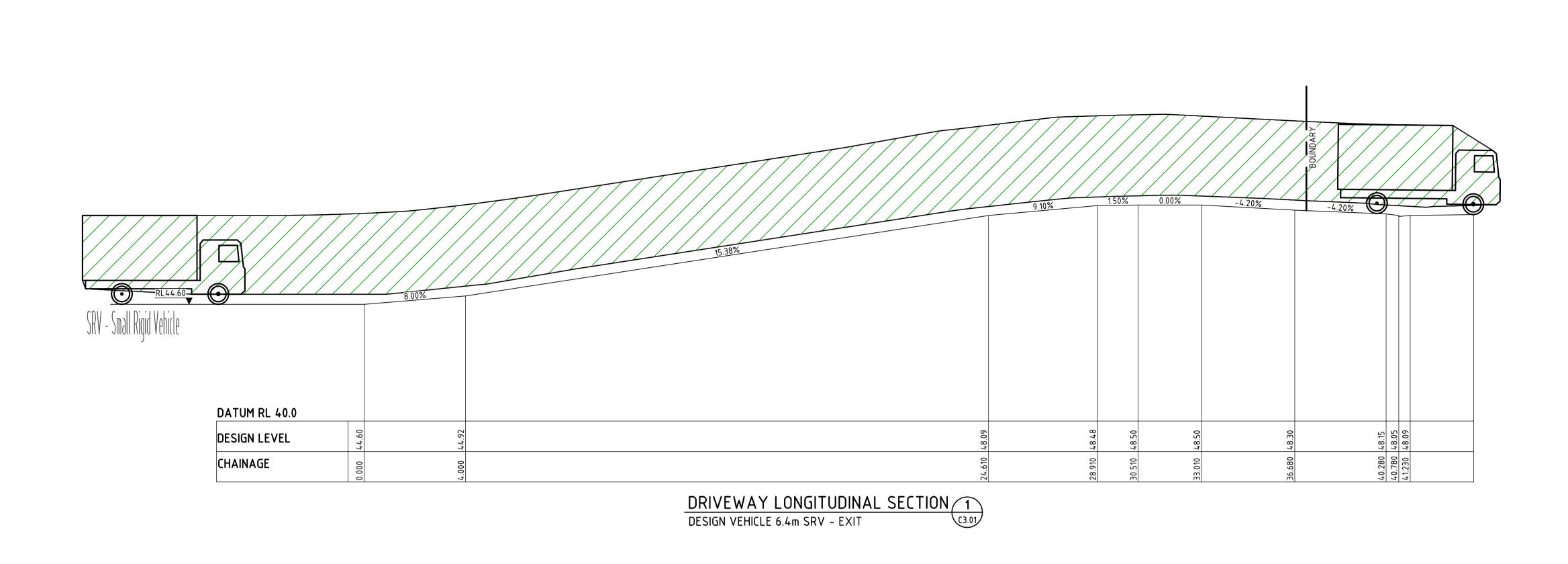
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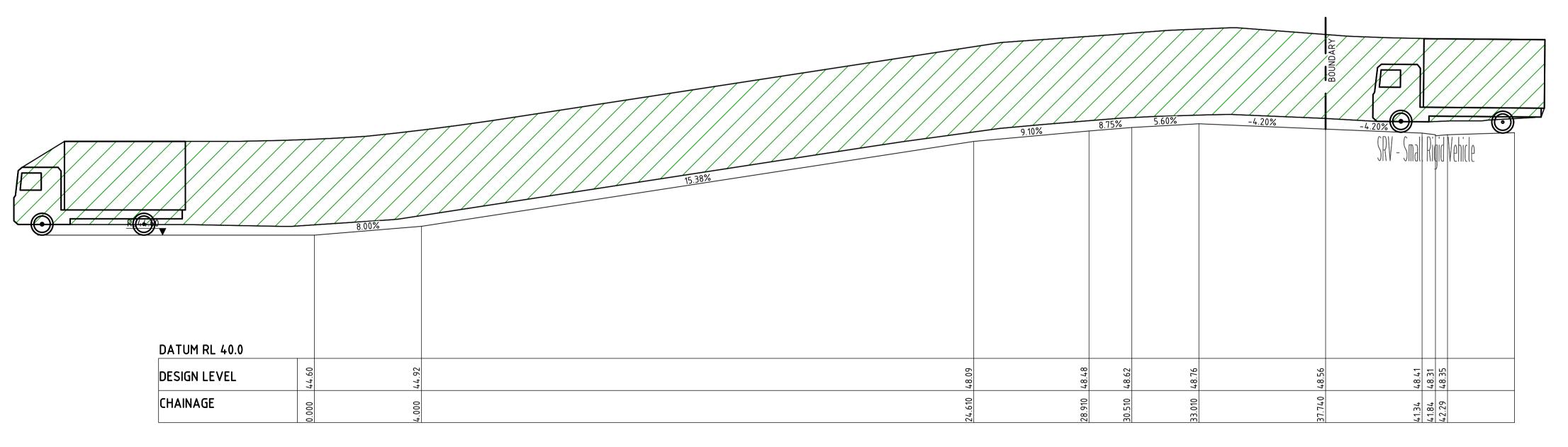
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DRIVEWAY LONGITUDINAL SECTION 2 DESIGN VEHICLE 6.4m SRV - ENTRY

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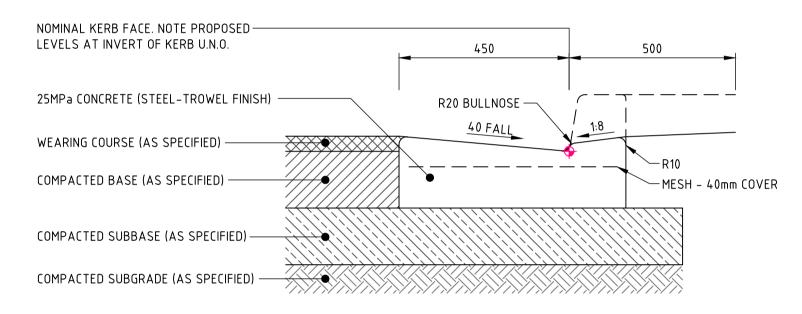
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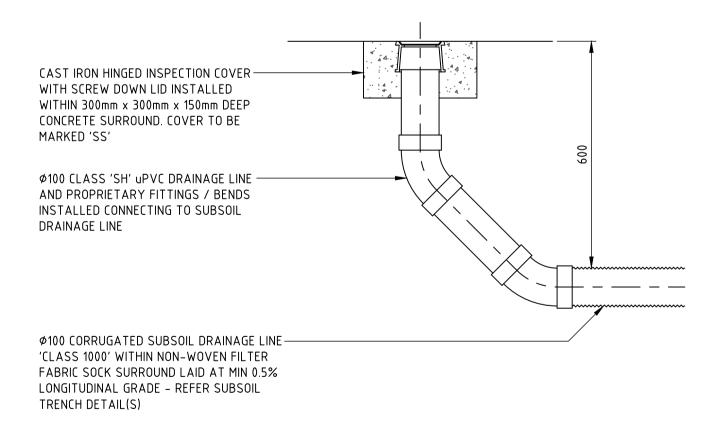
KERB & GUTTER 'KG'

EXPANSION JOINTS @ MAX 12m CTRS / TOOL JOINTS @ MAX 3m CTRS ALL RADII TO BE 20mm U.N.O.



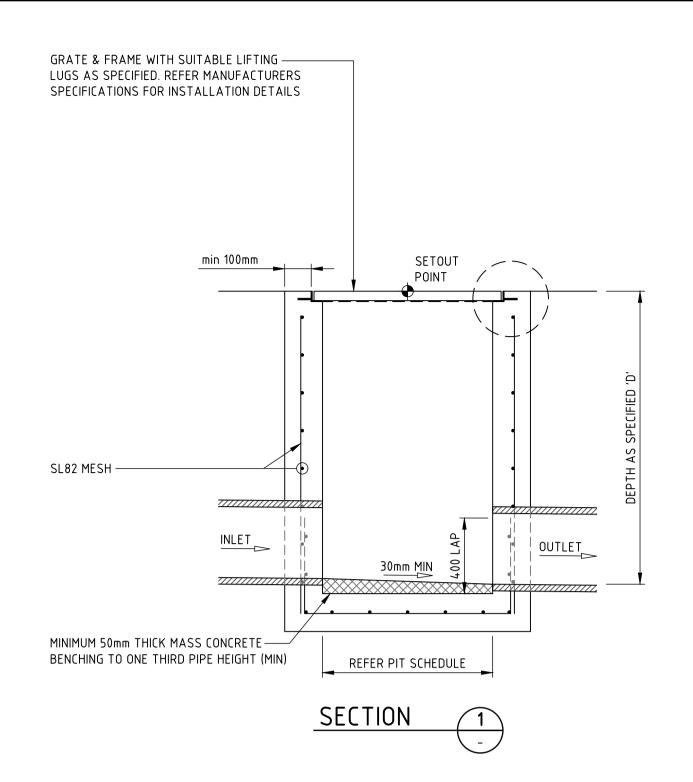
VEHICLE LAYBACK 'VC'

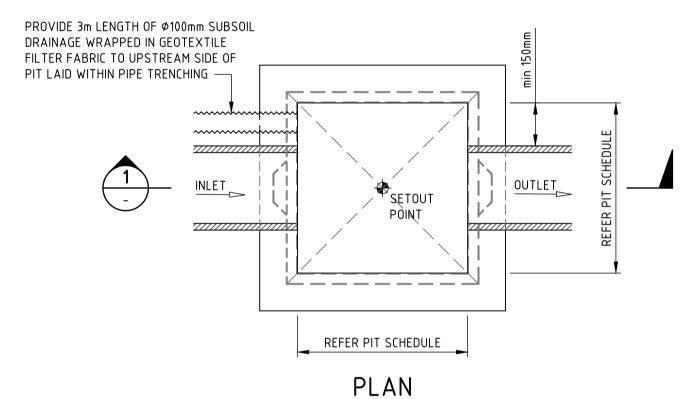
EXPANSION JOINTS @ MAX 12m CTRS / TOOL JOINTS @ MAX 3m CTRS ALL RADII TO BE 20mm U.N.O.



SUBSOIL DRAINAGE CLEAROUT 'CO'

CLEAROUT TO BE INSTALLED AT UPSTREAM POINTS ALONG SUBSOIL DRAINAGE LINES @ MAX 30m CENTRES AND DISCHARGING TO DRAINAGE STRUCTURES @ MAX 60m CENTRES.





SURFACE INLET 'SIP' / JUNCTION PIT 'JP'

PIT STRUCTURE TO BE 200mm THICK UNLESS SHOWN OTHERWISE. DRILL AND EPOXY PLASTIC PROPRIETARY STEP IRONS IN ACCORDANCE WITH AUSTRALIAN STANDARDS AND MANUFACTURERS SPECIFICATIONS (PITS > 1000mm DEPTH). REFER PIT INTERFACE DETAIL 'F' FOR CORNER REINFORCEMENT

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Sydney Level 11 345 George Street, Sydney NSW 2000 Ph (02) 9241 4188 Fax (02) 9241 4324 Email sydney@northrop.com.au ABN 81 094 433 100 28 - 32 SOMERSET STREET, **KINGSWOOD**

DRAWING TITLE CIVIL ENGINEERING PACKAGE **DEVELOPMENT APPLICATION**

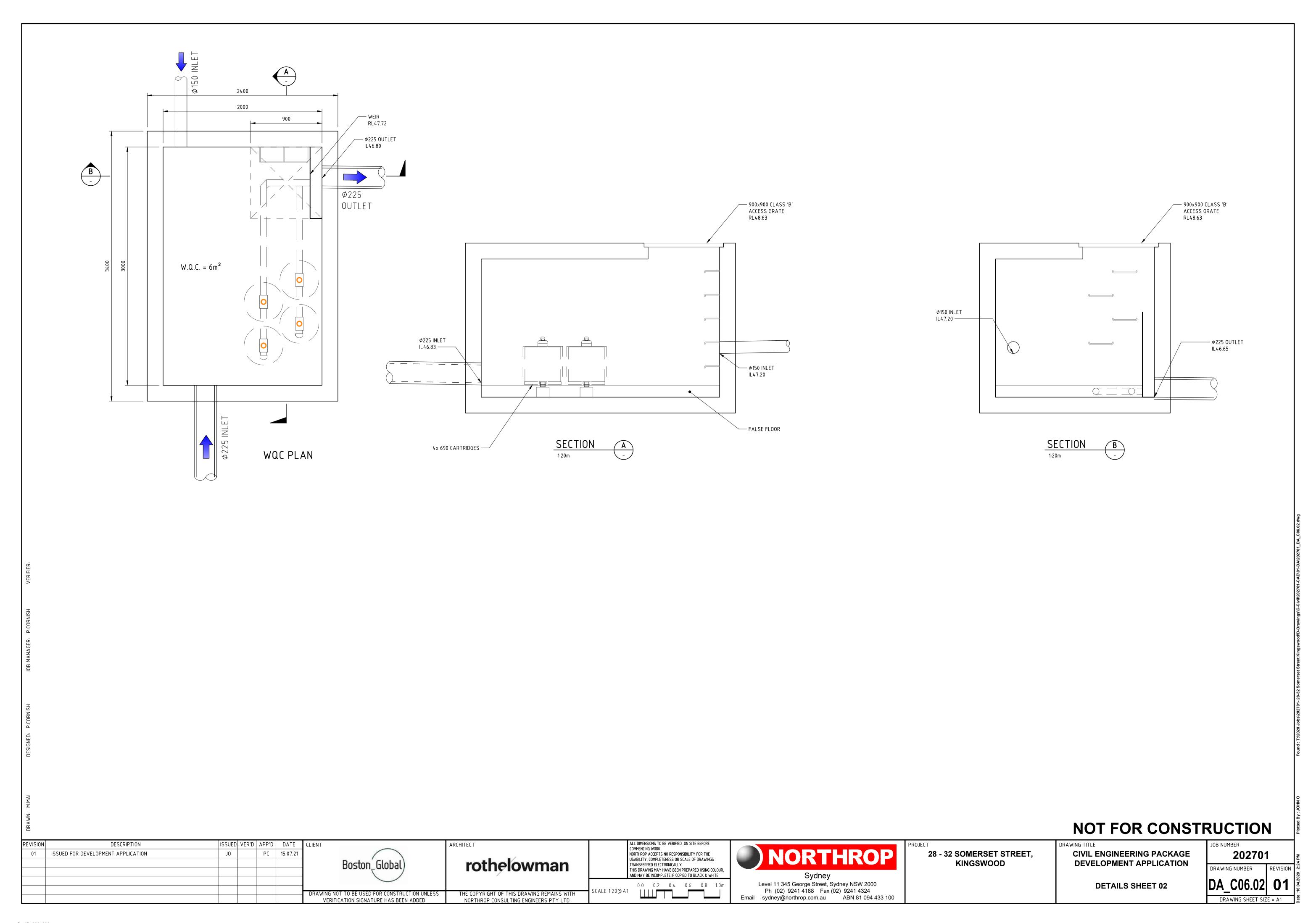
DETAILS SHEET 01

202701 REVISION DRAWING NUMBER

Document Set ID: 9681866 Version: 1, Version Date: 02/08/2021

REVISION

DESCRIPTION





Attachment C - Draft Operation and Maintenance Schedule

Version: 1, Version Date: 02/08/2021



28-32 Somerset Street Kingswood

Stormwater Maintenance Schedule

Prepared on 13.11.20

Site Description: The site is located on the eastern side of Somerset Street and on the corner of Hargrave Street.

Site Area: 1694m^2

Site Access: Direct access to the site will be by Hargrave Street.

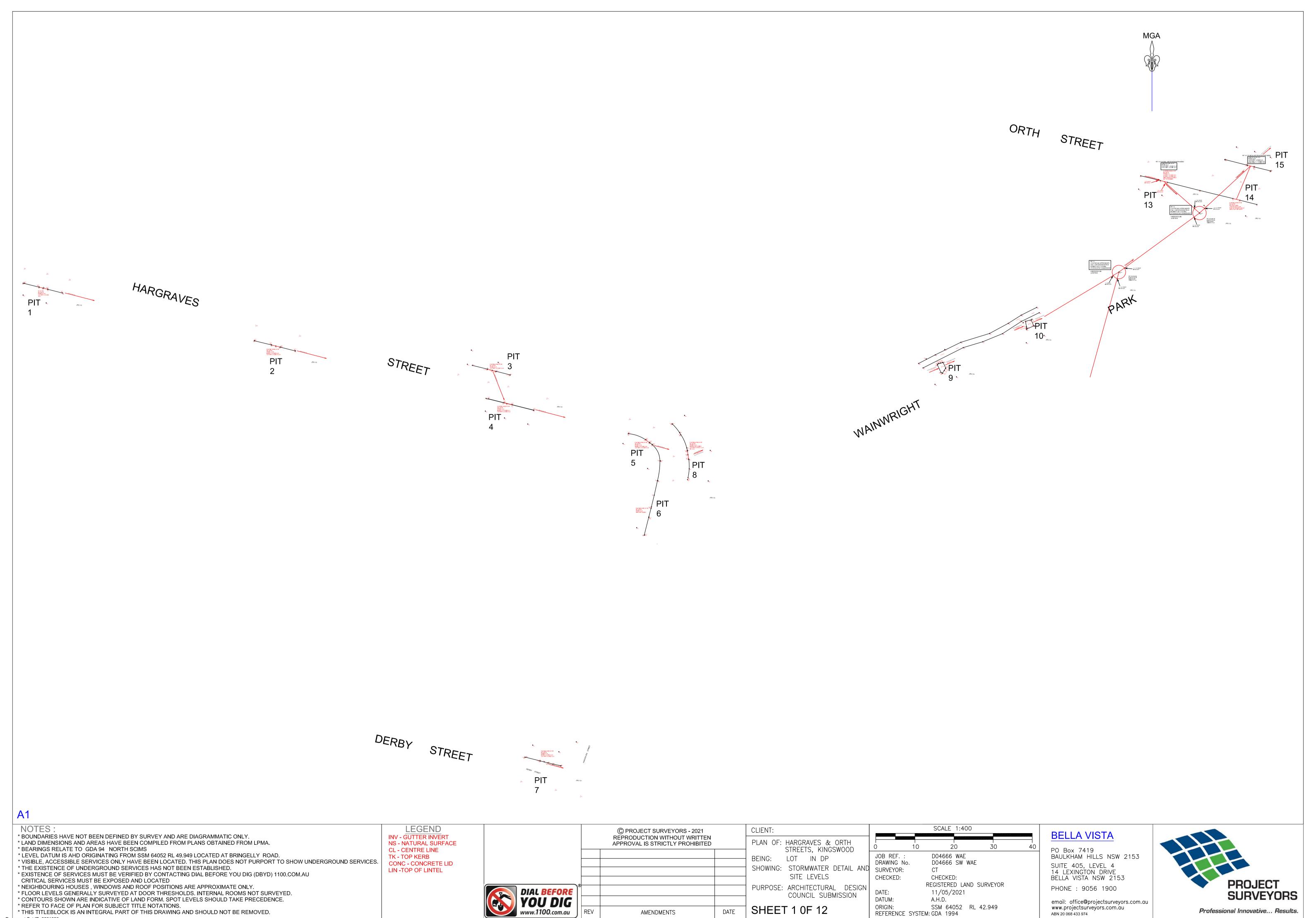
Inspected by:
Date of Inspection:
Next Inspection:

Items to be Inspected	Frequency	Performed by	Insp	ected	Maintenance Needed		Maintenance Procedure	
			Yes No Yes No		No			
General	•							
Stormwater surface inlet pits	Four Monthly/ After Major Storm	Owner / Maintenance Contractor					Remove grate and inspect internal walls and base, repair where required. Remove any collected sediment, debris, litter and vegetation. (e.g. Vacum/eductor truck) Inspect and ensure grate is clear of sediment, debris, litter and vegetation. Ensure flush placement of grate on refitment	
General inspection of complete stormwater drainage system (that's visible - including roof gutters)	Bi-annually	Owner / Maintenance Contractor					Inspect all drainage structures noting any dilapidation, carrry out required repairs.	
Rainwater Tanks								
First Flush Device	6 Monthly	Owner / Maintenance Contractor					Inspect first flush device to ensure correct operation. Remove accumulated litter & debris. If device is not functioning properly repair or replace.	
Internal Inspection	6 Monthly	Owner / Maintenance Contractor					Check for evidence of access by animals, birds or insects including the presence of mosquito larvae. If present, identify access point and close. If evidence of algae growth, find and close points of light entry.	
Tank and Lids	6 Monthly	Owner / Maintenance Contractor					Check structural integrity of tank including roof and access covers. Any dilapidation including holes or gaps are to be noted and repaired.	
Depth of Sediment within Tank	Every 2 Years	Owner / Maintenance Contractor					De-sludge tank(s) by engaging professional cleaner	
Primary Treatment								
Stormwater 360 Enviropod Pit Inserts (or equivalent)	Refer Manufacturers Manual	Maintenance / Specialised Contractor					Refer to manufacturers operation and maintenance manual.	
Secondary Treatment								
Stormwater 360 Stormfilter Cartridges (or equivalent)	Refer Manufacturers Manual	Maintenance / Specialised Contractor					Refer to manufacturers operation and maintenance manual.	

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Attachment D - Existing Drainage System Location Plan



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