

Report on **Geotechnical Assessment**

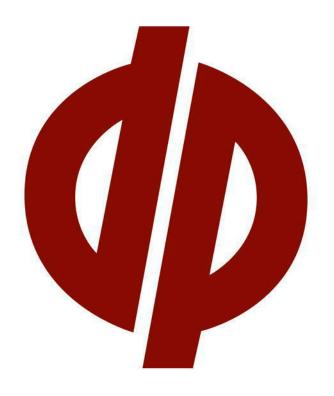
> Fernhill Estate Western Precinct Fairlight Road, Mulgoa

Prepared for Cubelic Holdings Pty Ltd

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The undersigned, on behalf of Douglas Partners Pty Ltd, confirm that this document and all attached drawings, logs and test results have been checked and reviewed for errors, omissions and inaccuracies.

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Report on Geotechnical Assessment Fernhill Estate, Western Precinct Fairlight Road, Mulgoa

1. Introduction

This report presents the results of additional geotechnical studies undertaken by Douglas Partners Pty Ltd (DP) within the western precinct of the proposed Fernhill Estate situated on Fairlight Road, Mulgoa. The work was commissioned by Cubelic Holdings Pty Ltd, on behalf of the property developer.

It is understood that the proposed development will include the subdivision of the site into 38 rural/residential allotments with a typical allotment size of approximately 20,000 m². The new lots will be serviced by a new road aligned centrally through the site that will provide access from two entry points located on Fairlight Road and Nepean Gorge Drive. Presently, the development is at a concurrent rezoning and development application stage, with the proposed development layout shown on Drawing 1, in Appendix B.

DP's previous Report on Land Capability Assessment (refer Project 71706.01, dated 26 June 2010), provided considerable geotechnical information for the western precinct. Due to a revised development layout, however, additional works were required to assess site areas that previously lay outside of the proposed development footprint. The additional geotechnical assessment works targeted the proposed on-site disposal of domestic effluent for the 38 new allotments.

The current geotechnical assessment comprised the drilling of boreholes followed by laboratory testing of selected samples, engineering analysis and reporting. Details of the work undertaken and the results obtained are presented in this report.

2. Site Description

The portion of land proposed for rural/residential development is approximately 115 hectares in plan area and encompasses the central and southern sections of a larger 180 hectare site that comprises two properties, namely:

- Lot 1 in DP549247; and
- Lot 31 in DP237163 (refer to Drawing 1).

Most of the proposed development is planned for the northern and southern parts of Lot 1 although the subdivision will also occupy the southern part of Lot 31 (refer Drawing 1). The proposed development area is bordered by:

North Side – The remainder of Lot 31;



- South Side Fairlight Road and existing rural/residential properties on the northern side of Fairlight Road;
- East Side Existing rural/residential properties on the western side of Nepean Gorge Drive; and
- West Side Fairlight Road.

The site is currently vacant rural land that is covered with grass and scattered to moderately dense natural tree growth. Although the site is essentially undeveloped and appears to follow the inferred natural land form, comparison of the site with those adjoining suggests some degree of vegetation clearing. Prior rural/grazing land use is also indicated by the presence of several existing rural dams.

Topographical relief across the majority of the site is slight, with the overall landform being undulating and varying in elevation from reduced levels of RL 203 m in the south east portion of the site to RL 164 m in the north east portion relative to Australian height datum (AHD). A broad ridge line runs north to south through the centre of the site and falls to the east and west where broad gullies approximate the eastern and western extents of the proposed development area.

3. Geology

Reference to the Penrith 1:100 000 Geological Series Sheet (Ref 2) indicates that the site is almost entirely underlain by Ashfield Shale of the Wianamatta Group of Triassic age. This formation typically comprises dark grey to black claystone-siltstone and fine grained sandstone-siltstone laminate. This formation typically weathers forming clays and silty clays of generally medium plasticity and low permeability.

The same geology map indicates that Hawkesbury Sandstone of Triassic age underlies land immediately to the west of the site and possibly within the western and north-western parts of the site. This formation typically comprises medium to very coarse grained quartz sandstone, minor laminated mudstone and siltstone lenses. This formation typically weathers forming sandy clay, clayey sand and sand soils of low plasticity and medium to high permeability.

4. Field Investigation

4.1 Field Investigation (2010)

The field work for the land capability assessment undertaken in 2010 comprised surface and subsurface investigations that included:

- Excavation of 30 test pits across the Western Precinct;
- Dynamic cone penetrometer (DCP) tests adjacent to selected test pits to aid the assessment of in-situ soil strength;
- Collection of representative bulk and undisturbed soil samples from the test pits for geotechnical laboratory analysis; and



 Collection of additional near-surface soil samples from shallow hand auger bores or manually excavated test pits, where relevant, between test pit locations.

Test pits were excavated by a backhoe, fitted with a 450 mm wide toothed bucket. Test pits were excavated to a maximum depth of 4 m or until practical refusal on rock was reached at depths of between 0.85 m and 4 m.

DCP testing was undertaken adjacent to 18 of the 30 test pits and extended to depths of between 0.9 m and 2.4 m.

Geotechnical sampling from the test pits included large bulk, small disturbed and undisturbed tube samples. A selection of these samples were then scheduled for a variety of laboratory tests including particle size distribution, hydrometer, Atterberg limits, Emerson class number, California bearing ratio, shrink swell index and field moisture content tests to assist the geotechnical assessment.

The results of the 2010 field investigation are presented on the test pit logs sheets in Appendix C.

4.2 Field Investigation (Current)

4.2.1 Field Work Methods

The field work for the current geotechnical assessment was conducted on 18 June 2013 and included:

- A walkover inspection of the site by a geotechnical engineer.
- Drilling of seven boreholes (BH1 to BH7) using a truck-mounted DT100 drill rig. The bores were
 drilled using solid flight augers fitted with a Tungsten-Carbide (TC) bit until termination in
 weathered rock (shale and sandstone) at depths of between 0.2 m and 2.2 m in all boreholes.
- Collection of soil samples from the boreholes for examination, logging and to provide laboratory test specimens for a range of geotechnical and chemical testing.

Borehole locations were selected to cover the areas of the site previously excluded from the development footprint and thus not assessed during the 2010 land capability assessment. The borehole locations are shown on Drawing 1, in Appendix B. Locations were chosen based on drill rig accessibility and existing buried services. Prior to drilling at the site, bore locations were scanned for the presence of in-ground (buried) service lines. The surface level for each bore was estimated from the survey data provided by the client, which is understood to be relative to AHD.

4.2.2 Field Work Results

A summary of the typical sequence of subsurface conditions encountered at site is presented below:

Topsoil:

Consisting of firm brown silty sandy clay with a trace of rootlets. Topsoil was present at boreholes BH1 to BH5 and extended to depths of between 0.1 m and 0.2 m. Topsoils were generally humid to damp, although are probably wetter now, considering the wet weather experienced since the field investigation.



Residual Soil: Comprising stiff to very stiff and hard, orange brown, mottled red brown and

grey silty clay and sandy clay. Residual clays were present in all seven boreholes except BH1 and extended to depths of between 0.7 m and 2.2 m. Residual clays were generally humid to moist and of estimated medium to high

plasticity.

Weathered Rock: Comprising Shale and Sandstone encountered from depths of between 0.2 m

and 2.2 m. Initially of estimated extremely low to low strength, auger penetration in sandstone was typically less than 0.7 m whereas penetration in shale reached 1.2 m in depth before drilling was terminated prior to refusal.

Soil conditions were relatively uniform across the site, confirming that only one soil landscape is present at the site (Blacktown Soil Landscape), as indicated by the soil landscape maps (refer DP's Report on Land Capability). Sandstone was present within the northern and western parts of the site, whereas shale was present elsewhere, which is consistent with geological maps for the site.

In addition to the above soil profiles, filling should be expected within the existing dam walls and is likely to comprise a blend of the residual soils and upper weathered rock profiles.

The results of the current field investigation are presented on the borehole logs sheets in Appendix D.

4.3 Groundwater

Groundwater was not observed in any of the boreholes drilled at site. Although the boreholes were immediately backfilled, preventing long term monitoring of groundwater levels, the moisture contents of the subsurface soils did not indicate free groundwater to be likely within the depth of the investigation. Given the elevation of the site, regional groundwater levels are expected to lie well below the ground surface.

Surface water was identified only in the existing dams on the site. No other surface water bodies or ponded areas were evident during the field investigation.

5. Laboratory Testing

5.1 Land Capability Assessment Test Results (2010)

Soil and weathered rock samples were collected from the test pits excavated during the 2010 field investigation. Representative samples were selected to undergo a suite of geotechnical and chemical tests to assist the land capability assessment. The results of the 2010 laboratory tests relevant to the current assessment are summarised in Tables 1 and 2 below and are presented in Appendix E.



Table 1: Results of Particle Size Distribution Tests (2010)

Test Pit	Depth	Soil		% of Soil Mass	
No.	(m)	Description	Sand ^{-D}	Silt ^{-E}	Clay-F
TP9	1.0	Sandy Silty Clay	28%	25%	43%
TP24	0.5	Sandy Silty Clay	18%	33%	46%
TP25	1.0	Sandy Silty Clay	16%	26%	41%

Notes: D Sand = 2.36 - 0.075 mm

The laboratory test results confirm the consistent clayey nature of the residual soils at the site. In conjunction with the 2010 Atterberg limit test results, the 2010 particle size distribution tests indicated soil classifications for the residual soils, in accordance with the unified soil classification system, corresponding to inorganic clays of low to medium plasticity (CL), inorganic clays of high plasticity (CH) and inorganic silts or fine sandy or silty soils (MH).

Emerson Class Number tests were undertaken on selected soil samples, and can be summarised as follows:

- Emerson Class Number 5 TP5 (0.5 m) and TP14 (0.5 m); and
- Emerson Class Number 6 TP9 (1.0 m), TP11 (0.5 m), TP24 (1.0 m) and TP28 (1.0 m).

The Emerson Class number for a soil relates to the potential for the soil to slake and disperse. Higher Emerson class numbers correspond to soils with a lower tendency to disperse. Emerson class numbers of 5 and 6 indicates a tendency for the soil to slake with a low susceptibility to dispersion.

Table 2: Results of Soil Chemistry Tests (2010)

TP	Depth	EC1:5	Texture	EC.	pHw	ESP	CI	SO ₄	Resis.		Comments	
No.	(m)	(ds/m)	Class	(Ds/m)	(1:2)	(%)	(mg/kg)	(mg/kg)	(Ω.m)	Salinity	Acidity	Sodicity
TP3	0.25	0.04	LC	0.37	7.6	3.00	17	3.8	230	NS	Neutral	38.3
TP3	0.5	0.04	LMC	0.30	6.1	7.4	20	2.5	270	NS	Neutral	S
TP3	1.0	0.04	LMC	0.31	6.8	(*)	18	3.3	250	NS	Neutral	
TP3	1.5	0.06	LMC	0.46	4.8	(*)	31	<2.0	170	NS	Acidic	(*)
TP3	2.0	0.04	LMC	0.30	4.6	(*)	16	3.7	260	NS	Acidic	(90)
TP3	2.5	0.04	MC	0.29	7.6	300	15	3.1	240	NS	Neutral	(*)
TP6	0.25	0.01	L	0.12	5.9	5745	20		770	NS	Acidic	(20)
TP6	0.5	0.02	LMC	0.13	5.9	12.8	20		630	NS	Acidic	S
TP9	0.25	0.01	LC	0.08	6.0	(14)	20		1100	NS	Neutral	(<u>*</u> 0
TP9	0.5	0.01	LC	0.12	6.0	(m)	25		710	NS	Neutral	(4)
TP9	1.0	0.01	МС	0.09	5.4	820	26		770	NS	Acidic	(4)
TP12	0.25	0.06	CL	0.50	4.9	-	<20	<20	180	NS	Acidic	1407

^Æ Silt = 0.075 − 0.002 mm

F Clay = <0.002 mm



TP	Depth	EC1:5	Texture	EC.	pHw	ESP	CI	SO ₄	Resis.		Comments	Ni
No.	(m)	(ds/m)	Class	(Ds/m)	(1:2)	(%)	(mg/kg)	(mg/kg)	(Ω.m)	Salinity	Acidity	Sodicity
TP12	0.5	0.05	LMC	0.36	4.9	10.5	6.0	6.9	220	NS	Acidic	s
TP14	0.25	0.01	CL	0.09	6.2		#0		1100	NS	Neutral	
TP14	0.5	0.01	LMC	0.04	6.0	1.5	**		1900	NS	Neutral	s e s
TP14	1.0	0.01	MC	0.10	5.9	(*)	-	-	710	NS	Acidic	(*)
TP14	1.5	0.02	МС	0.11	5.4	(*)	-		670	NS	Acidic	*
TP16	0.25	0.04	LMC	0.30	5.0	18.1	2.2	7.2	260	NS	Acidic	HS
TP16	0.5	0.04	LMC	0.34	7.8	9.4	2.1	6.6	230	NS	Neutral	S
TP18	0.25	0.01	LMC	0.10	5.9	37#3	20		770	NS	Acidic	150
TP18	0.5	0.03	LMC	0.23	8.4	37#3	5.6	<2.0	340	NS	Basic	(%)
TP20	0.25	0.02	LMC	0.19	5.1	3745	2.8	<2.0	420	NS	Acidic	(20)
TP20	0.5	0.03	МС	0.22	5.1	120	6.2	2.0	320	NS	Acidic	140
TP22	0.25	0.01	MC	0.08	5.8	-	26	, s	910	NS	Acidic	
TP22	0.5	0.01	МС	0.06	6.3	6.6	26	S S	1300	NS	Neutral	S
TP24	0.25	0.01	LMC	0.07	6.1	9920	20	8	1100	NS	Neutral	10_0
TP24	0.5	0.01	LMC	0.10	6.1	9920	20	3	770	NS	Neutral	10_0
TP24	1.0	0.01	МС	0.08	5.4	1920	20	3	830	NS	Acidic	1020
TP26	0.25	0.01	LMC	0.10	5.8				760	NS	Acidic	
TP26	0.5	0.05	LMC	0.36	5.1		12	<2.0	220	NS	Acidic	
TP28	0.25	0.01	CL	0.08	6.0		•	9	1200	NS	Neutral	
TP28	0.5	0.01	MC	0.09	6.2			-	790	NS	Neutral	

EC1:5 Where Electrical Conductivity

EC. Electrical Conductivity corrected for soil texture pHw = pH in water Chloride CI ==

SO4 Sulphate **ESP** Exchangeable Sodium Percentage

Resis. = Resistivity

Loam LC Light Clay

Light Medium Clay Medium Clay LMC = MC = CL = Clay Loam NS = Non-saline

S/HS = Sodic/Highly Sodic

5.2 Geotechnical (Current)

Soil samples were collected from the boreholes during the current field investigation. Representative samples were selected to undergo the following suite of geotechnical tests:

- Emerson class number tests 7 samples
- Particle size distribution tests 7 samples; and
- Bulk Density tests 1 sample.

The results of these tests are presented in Appendix F and are summarised in Table 3.



Table 3: Results of Laboratory Tests (Geotechnical)

ВН	Depth	Soil	Bulk	Emerson		% of Soil Ma	ss
No.	(m)	Description	Density (t/m³) -A	Class No.	Gravel ⁻⁸	Sand ^{-C}	Silt+Clay ^{-D}
BH1	0.3 - 0.6	Shaly Sandy Clay	583	2	22	33	45
BH2	0.3 - 0.5	Sandy Silty Clay	100	6	12	15	73
внз	0.8 – 1.1	Silty Clay	072	6	3	9	88
ВН4	0.4 - 0.6	Sandy Silty Clay	1.97	6	10	42	48
BH5	0.6 - 0.8	Silty Clayey Sand		6	4	54	42
вн6	0.3 - 0.5	Silty Clayey Sand	180	6	16	56	28
BH7	0.7 - 1.0	Silty Clay	140	6	6	9	85

Notes: A Bulk Density measured at in situ moisture content, which was estimated as within 3-5% of optimum moisture content

The laboratory test results confirm the mostly consistent clayey nature of the residual soils at the site.

The Emerson Class number for a soil relates to the potential for the soil to slake and disperse. Higher Emerson class numbers correspond to soils with a lower tendency to disperse. An Emerson class number of 6 indicates a tendency for the soil to slake with a low susceptibility to dispersion, whereas an Emerson class number of 2 indicates a tendency for the soil to slake and also to partially disperse.

5.3 Chemical (Current)

Representative soil samples collected from the boreholes during the field investigation were selected to undergo the following suite of chemical tests:

- pH;
- Electrical Conductivity (EC_{1:5});
- Cation Exchange Capacity (CEC);
- Sodicity Exchangeable Sodium Percentage (ESP);
- · Phosphorous Sorption (P-sorption); and
- Classification Soil texture.

The results of these tests are presented in Appendix F and are summarised in Table 4.

⁸ Gravel = >2.36 mm

^{-C} Sand = 2.36 - 0.075 mm

D Silt+Clay = < 0.075 mm



Table 4: Results of Laboratory Tests (Chemical)

BH No.	Depth (m)	pHw (1:5)	EC _{1:5} (µs/m)	CEC (mg/kg)	ESP (%)	P-sorp (kg/ha)	Texture Class
BH1	0.3 – 0.6	5.5	16	2.5	4.7	9 500	LC
BH2	0.3 – 0.5	5.8	20	3.3	4.0	9 900	MC
внз	0.8 – 1.1	5.8	34	8.1	3.3	9 300	LMC
BH4	0.4 – 0.6	6.1	13	3.7	1.7	10 000	LC
BH5	0.6 – 0.8	6.5	13	3.1	2.9	8 700	CL
вн6	0.3 – 0.5	6.1	-11	1.6	2.5	7 400	CL
вн7	0.7 – 1.0	5.6	20	3.3	4.7	9 900	LMC

Where pHw = pH in water

EC_{1.E} = Electrical Conductivity

CEC = Cation Exchange Capacity

ESP = Exchangeable Sodium Percentage

P-sorp = Phosphorous Sorption

LC = Light Clay

LMC = Light Medium Clay

MC = Medium Clay

CL = Clay Loam

Proposed Development

It is understood that the proposed development will include the subdivision of the site into 38 rural/residential allotments with a typical allotment size of approximately 20,000 m². The new lots will be serviced by a new road aligned centrally through the site that will provide access from two entry points located on Fairlight Road and Nepean Gorge Drive. Presently, the development is at a concurrent rezoning and development application stage, with the current proposed development layout shown on Drawing 1, in Appendix B.

The following sections provide general comments on suggested design parameters for the design and construction of on-site effluent disposal absorption beds.

7. Comments

7.1 Geotechnical Model

Based on the results of the current geotechnical assessment, together with the results of the 2010 land capability assessment, the site may be characterised as exhibiting a typically thin 0.1 m to 0.2 m layer of topsoil overlying stiff to hard residual clay, silty clay and sandy clay, overlying weathered bedrock comprising mostly shale and some sandstone. Rock depths range between 0.2 m and 2.2 m, although typically the rock surface lies close to 1 m depth.

The excavation of test pits and the drilling of boreholes have indicated the soils to be relatively uniform and of a stiff to hard consistency. Accordingly, the soils are generally weakly structured to massive



and of generally low permeability. It is therefore apparent that the design of on-site effluent disposal beds will need to be based on relatively low permeability values, thus resulting in relatively large absorption areas.

7.2 Design Soil Parameters

Based on the results of the current and 2010 assessments, and according to the procedures outlined in AS1547-2012, the following parameters are suggested for the design of on-site effluent disposal beds.

- Depth to bedrock or hardpan it is suggested that the depth to rock for the given disposal area on
 each new lot be taken from the relevant test pit and/or borehole log sheet that represents the
 closest field investigation point to the proposed disposal area.
- Depth to high soil water table groundwater was not encountered at the site during either the
 current or 2010 field investigations. It is suggested that the highest likely ground water surface for
 this site is expected to coincide with minor subsurface seepage flows that intermittently lie at or
 close to the soil/rock interface.
- Soil structure soil structure at the site is generally considered as being weakly structured to
 massive. On further investigation, it may be possible to identify areas of moderately structure
 clays, although this is considered unlikely.
- Soil texture soil texture classifications are presented above in Tables 2 and 4.
- Permeability category it is suggested that the design permeability category for the given disposal
 area on each new lot be taken from Table 5.1 of AS1547-2012, with relevant soil category and
 indicative permeability to be based on the texture classification results presented in Tables 2 and 4
 and the recommended soil structure outlined above. Where the test results indicate borderline
 conditions, the least permeable category/value should be adopted.
- Hydraulic loading for soil absorption system it is suggested that the design hydraulic loading rate
 for the given disposal area on each new lot be taken from Table 5.2 of AS1547-2012, with relevant
 soil category, indicative permeability design loading rate to be based on the texture classification
 results presented in Tables 2 and 4, the recommended soil structure outlined above and the type
 of disposal system under consideration. Where the test results indicate borderline conditions, the
 least permeable category/value should be adopted.
- Coarse fragments the percentage content of coarse fragments within the site's residual soils is low. Design values should be based on the results of particle size distribution tests presented above in Tables 1 and 3.
- Bulk density a single result of 1.97 t/m³ was obtained for soil bulk density. The density value was based on the soil conditions at the time of the field work, which were estimated as having a field moisture content within 3% to 5% dry of Standard optimum moisture content. The result is considered reasonable to slightly high for the given clay soil, which is likely to be the result of a relatively well graded soil that comprises an even mix of sand with silt and clay fines and some fine gravel. Based on the results of the investigations, it is considered that soil densities across the site would typically range between 1.8 t/m³ to 2.0 t/m³.



- Chemical test results it is suggested that the design values for soil pH, electrical conductivity (EC), exchangeable sodium percentage (ESP), cation exchange capacity (CEC) and phosphorous sorption (P-sorp) are based on the laboratory test results presented above in Tables 2 and 4.
- Dispersiveness it is suggested that the design values for soil dispersiveness are based on the Emerson class number tests results presented above in Section 5.1 and Table 3.

8. Limitations

Douglas Partners (DP) has prepared this report for this project at Fernhill Estate, Western Precinct, Fairlight Road, Mulgoa, in accordance with instructions received from Mr Paul Cubelic of Cubelic Holdings Pty Ltd and DP's fee proposal (Ref No. SYD130552, dated 14 June 2013). The work was carried out under DP's Conditions of Engagement. This updated report is provided for the exclusive use of Cubelic Holdings Pty Ltd for this project only and for the purposes as described in the report. It should not be used by or relied upon for other projects or purposes on the same or other site or by a third party. Any party so relying upon this report beyond its exclusive use and purpose as stated above, and without the express written consent of DP, does so entirely at its own risk and without recourse to DP for any loss or damage. In preparing this report DP has necessarily relied upon information provided by the client and/or their agents.

The results provided in the report are indicative of the sub-surface conditions on the site only at the specific sampling and/or testing locations, and then only to the depths investigated and at the time the work was carried out. Sub-surface conditions can change abruptly due to variable geological processes and also as a result of human influences. Such changes may occur after DP's field testing has been completed.

DP's advice is based upon the conditions encountered during this investigation. The accuracy of the advice provided by DP in this report may be affected by undetected variations in ground conditions across the site between and beyond the sampling and/or testing locations. The advice may also be limited by budget constraints imposed by others or by site accessibility.

This report must be read in conjunction with all of the attached and should be kept in its entirety without separation of individual pages or sections. DP cannot be held responsible for interpretations or conclusions made by others unless they are supported by an expressed statement, interpretation, outcome or conclusion stated in this report.

This report, or sections from this report, should not be used as part of a specification for a project, without review and agreement by DP. This is because this report has been written as advice and opinion rather than instructions for construction.

The contents of this report do not constitute formal design components such as are required, by the Health and Safety Legislation and Regulations, to be included in a Safety Report specifying the hazards likely to be encountered during construction and the controls required to mitigate risk. This design process requires risk assessment to be undertaken, with such assessment being dependent upon factors relating to likelihood of occurrence and consequences of damage to property and to life. This, in turn, requires project data and analysis presently beyond the knowledge and project role respectively of DP. DP may be able, however, to assist the client in carrying out a risk assessment of



potential hazards contained in the Comments section of this report, as an extension to the current scope of works, if so requested, and provided that suitable additional information is made available to DP. Any such risk assessment would, however, be necessarily restricted to the geotechnical components set out in this report and to their application by the project designers to project design, construction, maintenance and demolition.

Douglas Partners Pty Ltd

Appendix A

About this Report

About this Report Douglas Partners The state of the sta

Introduction

These notes have been provided to amplify DP's report in regard to classification methods, field procedures and the comments section. Not all are necessarily relevant to all reports.

DP's reports are based on information gained from limited subsurface excavations and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

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Borehole and Test Pit Logs

The borehole and test pit logs presented in this report are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling or excavation. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable or possible to justify on economic grounds. In any case the boreholes and test pits represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes or pits, the frequency of sampling, and the possibility of other than 'straight line' variations between the test locations.

Groundwater

Where groundwater levels are measured in boreholes there are several potential problems, namely:

 In low permeability soils groundwater may enter the hole very slowly or perhaps not at all during the time the hole is left open;

- A localised, perched water table may lead to an erroneous indication of the true water table:
- Water table levels will vary from time to time with seasons or recent weather changes.
 They may not be the same at the time of construction as are indicated in the report;
- The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water measurements are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Reports

The report has been prepared by qualified personnel, is based on the information obtained from field and laboratory testing, and has been undertaken to current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal, the information and interpretation may not be relevant if the design proposal is changed. If this happens, DP will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface conditions, discussion of geotechnical and environmental aspects, and recommendations or suggestions for design and construction. However, DP cannot always anticipate or assume responsibility for:

- Unexpected variations in ground conditions.
 The potential for this will depend partly on borehole or pit spacing and sampling frequency;
- Changes in policy or interpretations of policy by statutory authorities; or
- The actions of contractors responding to commercial pressures.

If these occur, DP will be pleased to assist with investigations or advice to resolve the matter.

About this Report

Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, DP requests that it be immediately notified. Most problems are much more readily resolved when conditions are exposed rather than at some later stage, well after the event.

Information for Contractual Purposes

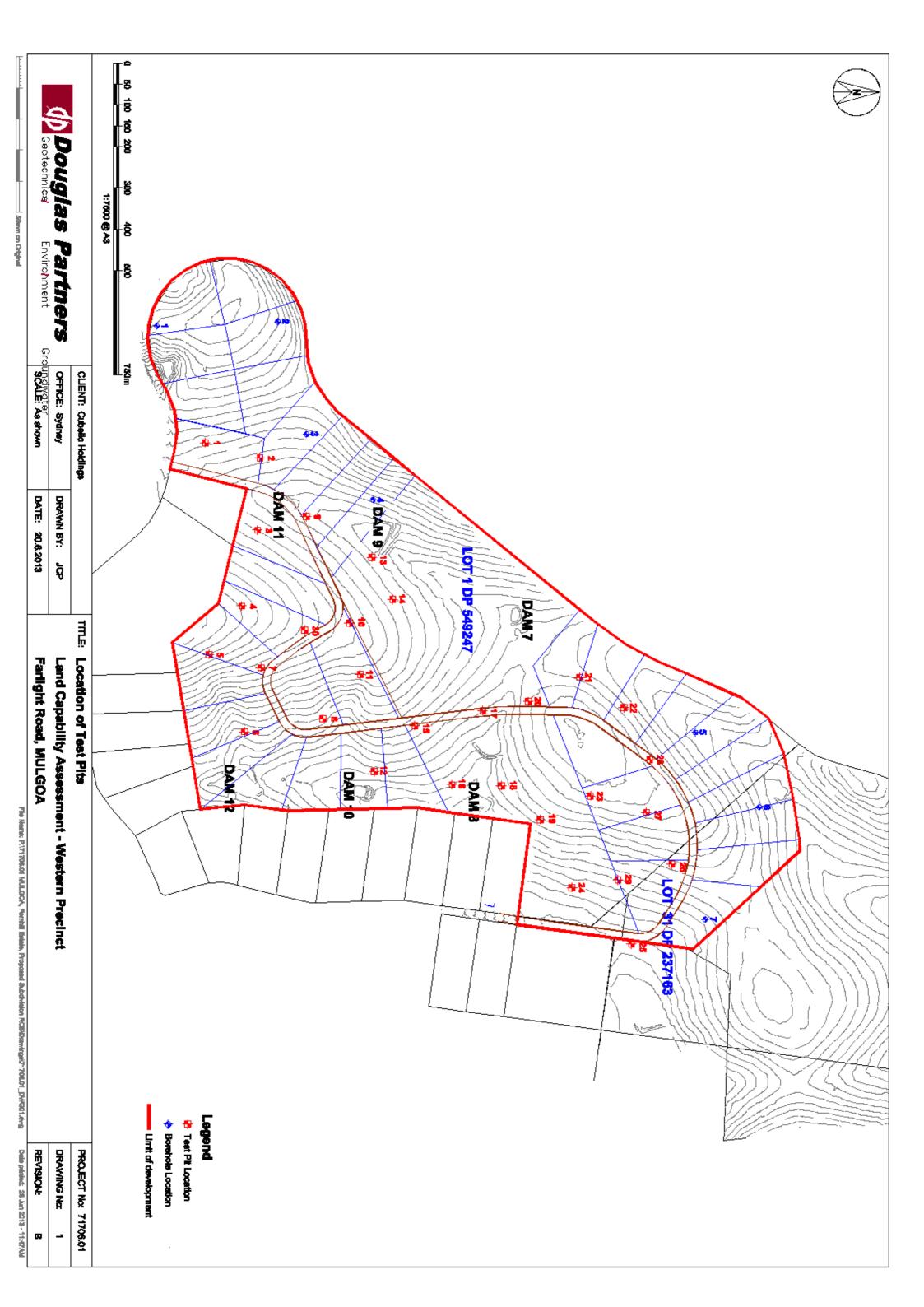
Where information obtained from this report is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. DP would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The company will always be pleased to provide engineering inspection services for geotechnical and environmental aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

Appendix B

Drawing 1



Appendix C

Field Work Results (2010)

Sampling Methods

Sampling

Sampling is carried out during drilling or test pitting to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling provide information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thinwalled sample tube into the soil and withdrawing it to obtain a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

Test Pits

Test pits are usually excavated with a backhoe or an excavator, allowing close examination of the insitu soil if it is safe to enter into the pit. The depth of excavation is limited to about 3 m for a backhoe and up to 6 m for a large excavator. A potential disadvantage of this investigation method is the larger area of disturbance to the site.

Large Diameter Augers

Boreholes can be drilled using a rotating plate or short spiral auger, generally 300 mm or larger in diameter commonly mounted on a standard piling rig. The cuttings are returned to the surface at intervals (generally not more than 0.5 m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube samples.

Continuous Spiral Flight Augers

The borehole is advanced using 90-115 mm diameter continuous spiral flight augers which are withdrawn at intervals to allow sampling or in-situ testing. This is a relatively economical means of drilling in clays and sands above the water table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are disturbed and may be mixed with soils from the sides of the hole. Information from the drilling (as distinct from specific sampling by SPTs or undisturbed samples) is of relatively low

reliability, due to the remoulding, possible mixing or softening of samples by groundwater.

Non-core Rotary Drilling

The borehole is advanced using a rotary bit, with water or drilling mud being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from the rate of penetration. Where drilling mud is used this can mask the cuttings and reliable identification is only possible from separate sampling such as SPTs.

Continuous Core Drilling

A continuous core sample can be obtained using a diamond tipped core barrel, usually with a 50 mm internal diameter. Provided full core recovery is achieved (which is not always possible in weak rocks and granular soils), this technique provides a very reliable method of investigation.

Standard Penetration Tests

Standard penetration tests (SPT) are used as a means of estimating the density or strength of soils and also of obtaining a relatively undisturbed sample. The test procedure is described in Australian Standard 1289, Methods of Testing Soils for Engineering Purposes - Test 6.3.1.

The test is carried out in a borehole by driving a 50 mm diameter split sample tube under the impact of a 63 kg hammer with a free fall of 760 mm. It is normal for the tube to be driven in three successive 150 mm increments and the 'N' value is taken as the number of blows for the last 300 mm. In dense sands, very hard clays or weak rock, the full 450 mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

 In the case where full penetration is obtained with successive blow counts for each 150 mm of, say, 4, 6 and 7 as:

> 4,6,7 N=13

In the case where the test is discontinued before the full penetration depth, say after 15 blows for the first 150 mm and 30 blows for the next 40 mm as:

15, 30/40 mm

Sampling Methods

The results of the SPT tests can be related empirically to the engineering properties of the soils.

Dynamic Cone Penetrometer Tests / Perth Sand Penetrometer Tests

Dynamic penetrometer tests (DCP or PSP) are carried out by driving a steel rod into the ground using a standard weight of hammer falling a specified distance. As the rod penetrates the soil the number of blows required to penetrate each successive 150 mm depth are recorded. Normally there is a depth limitation of 1.2 m, but this may be extended in certain conditions by the use of extension rods. Two types of penetrometer are commonly used.

- Perth sand penetrometer a 16 mm diameter flat ended rod is driven using a 9 kg hammer dropping 600 mm (AS 1289, Test 6.3.3). This test was developed for testing the density of sands and is mainly used in granular soils and filling.
- Cone penetrometer a 16 mm diameter rod with a 20 mm diameter cone end is driven using a 9 kg hammer dropping 510 mm (AS 1289, Test 6.3.2). This test was developed initially for pavement subgrade investigations, and correlations of the test results with California Bearing Ratio have been published by various road authorities.

Soil Descriptions

Description and Classification Methods

The methods of description and classification of soils and rocks used in this report are based on Australian Standard AS 1726, Geotechnical Site Investigations Code. In general, the descriptions include strength or density, colour, structure, soil or rock type and inclusions.

Soil Types

Soil types are described according to the predominant particle size, qualified by the grading of other particles present:

Туре	Particle size (mm)
Boulder	>200
Cobble	63 - 200
Gravel	2.36 - 63
Sand	0.075 - 2.36
Silt	0.002 - 0.075
Clay	< 0.002

The sand and gravel sizes can be further subdivided as follows:

Туре	Particle size (mm)
Coarse gravel	20 - 63
Medium gravel	6 - 20
Fine gravel	2.36 - 6
Coarse sand	0.6 - 2.36
Medium sand	0.2 - 0.6
Fine sand	0.075 - 0.2

The proportions of secondary constituents of soils are described as:

Term	Proportion	Example
And	Specify	Clay (60%) and Sand (40%)
Adjective	20 - 35%	Sandy Clay
Slightly	12 - 20%	Slightly Sandy Clay
With some	5 - 12%	Clay with some sand
With a trace of	0 - 5%	Clay with a trace of sand

Definitions of grading terms used are:

- Well graded a good representation of all particle sizes
- Poorly graded an excess or deficiency of particular sizes within the specified range
- Uniformly graded an excess of a particular particle size
- Gap graded a deficiency of a particular particle size with the range

Cohesive Soils

Cohesive soils, such as clays, are classified on the basis of undrained shear strength. The strength may be measured by laboratory testing, or estimated by field tests or engineering examination. The strength terms are defined as follows:

Description	Abbreviation	Undrained shear strength (kPa)
Very soft	vs	<12
Soft	s	12 - 25
Firm	f	25 - 50
Stiff	st	50 - 100
Very stiff	vst	100 - 200
Hard	h	>200

Cohesionless Soils

Cohesionless soils, such as clean sands, are classified on the basis of relative density, generally from the results of standard penetration tests (SPT), cone penetration tests (CPT) or dynamic penetrometers (PSP). The relative density terms are given below:

Relative Density	Abbreviation	SPT N value	CPT qc value (MPa)
Very loose	vI	<4	<2
Loose	ı	4 - 10	2 -5
Medium dense	md	10 - 30	5 - 15
Dense	d	30 - 50	15 - 25
Very dense	vd	>50	>25

Soil Descriptions

Soil Origin

It is often difficult to accurately determine the origin of a soil. Soils can generally be classified as:

- Residual soil derived from in-situ weathering of the underlying rock;
- Transported soils formed somewhere else and transported by nature to the site; or
- Filling moved by man.

Transported soils may be further subdivided into:

- Alluvium river deposits
- Lacustrine lake deposits
- · Aeolian wind deposits
- · Littoral beach deposits
- · Estuarine tidal river deposits
- · Talus scree or coarse colluvium
- Slopewash or Colluvium transported downslope by gravity assisted by water. Often includes angular rock fragments and boulders.

Rock Strength

Rock strength is defined by the Point Load Strength Index (Is₍₅₀₎) and refers to the strength of the rock substance and not the strength of the overall rock mass, which may be considerably weaker due to defects. The test procedure is described by Australian Standard 4133.4.1 - 1993. The terms used to describe rock strength are as follows:

Term	Abbreviation	Point Load Index Is ₍₅₀₎ MPa	Approx Unconfined Compressive Strength MPa*
Extremely low	EL	< 0.03	<0.6
Very low	VL	0.03 - 0.1	0.6 - 2
Low	L	0.1 - 0.3	2 - 6
Medium	M	0.3 - 1.0	6 - 20
High	Н	1-3	20 - 60
Very high	VH	3 - 10	60 - 200
Extremely high	EH	>10	>200

^{*} Assumes a ratio of 20:1 for UCS to Is(50)

Degree of Weathering

The degree of weathering of rock is classified as follows:

Term	Abbreviation	Description	
Extremely weathered	EW	Rock substance has soil properties, i.e. it can be remoulded and classified as a soil but the texture of the original rock is still evident.	
Highly weathered	HW	Limonite staining or bleaching affects whole of rock substance and other signs of decomposition are evident Porosity and strength may be altered as a result of iron leaching or deposition. Colour and strength of original fresh rock is not recognisable	
Moderately weathered	MW	Staining and discolouration of rock substance has taken place	
Slightly weathered	SW	Rock substance is slightly discoloured but shows little or no change of strength from fresh rock	
Fresh stained	Fs	Rock substance unaffected by weathering but staining visible along defects	
Fresh	Fr	No signs of decomposition or staining	

Degree of Fracturing

The following classification applies to the spacing of natural fractures in diamond drill cores. It includes bedding plane partings, joints and other defects, but excludes drilling breaks.

Term	Description
Fragmented	Fragments of <20 mm
Highly Fractured	Core lengths of 20-40 mm with some fragments
Fractured	Core lengths of 40-200 mm with some shorter and longer sections
Slightly Fractured	Core lengths of 200-1000 mm with some shorter and loner sections
Unbroken	Core lengths mostly > 1000 mm

Rock Descriptions

Rock Quality Designation

The quality of the cored rock can be measured using the Rock Quality Designation (RQD) index, defined as:

RQD % = <u>cumulative length of 'sound' core sections</u> ≥ 100 mm long total drilled length of section being assessed

where 'sound' rock is assessed to be rock of low strength or better. The RQD applies only to natural fractures. If the core is broken by drilling or handling (i.e. drilling breaks) then the broken pieces are fitted back together and are not included in the calculation of RQD.

Stratification Spacing

For sedimentary rocks the following terms may be used to describe the spacing of bedding partings:

Term	Separation of Stratification Planes
Thinly laminated	< 6 mm
Laminated	6 mm to 20 mm
Very thinly bedded	20 mm to 60 mm
Thinly bedded	60 mm to 0.2 m
Medium bedded	0.2 m to 0.6 m
Thickly bedded	0.6 m to 2 m
Very thickly bedded	> 2 m

Symbols & Abbreviations

Introduction

These notes summarise abbreviations commonly used on borehole logs and test pit reports.

Drilling or Excavation Methods

C	Core Drilling
R	Rotary drilling
SFA	Spiral flight augers
NIL II	D: 50

NMLC Diamond core - 52 mm dia NQ Diamond core - 47 mm dia HQ Diamond core - 63 mm dia PQ Diamond core - 81 mm dia

Water

Sampling and Testing

Α	Auger sample
В	Bulk sample
D	Disturbed sample
E	Environmental sample

U₅₀ Undisturbed tube sample (50mm)

W Water sample

pp pocket penetrometer (kPa)
PID Photo ionisation detector
PL Point load strength Is(50) MPa
S Standard Penetration Test

V Shear vane (kPa)

Description of Defects in Rock

The abbreviated descriptions of the defects should be in the following order: Depth, Type, Orientation, Coating, Shape, Roughness and Other. Drilling and handling breaks are not usually included on the logs.

Defect Type

J P -
Bedding plane
Clay seam
Cleavage
Crushed zone
Decomposed seam

F Fault
J Joint
Lam lamination
Pt Parting
Sz Sheared Zone

V Vein

Orientation

The inclination of defects is always measured from the perpendicular to the core axis.

h	horizontal
٧	vertical
sh	sub-horizontal
sv	sub-vertical

Coating or Infilling Term

cln	clean
co	coating
he	healed
inf	infilled
stn	stained
ti	tight
vn	veneer

Coating Descriptor

ca	calcite
cbs	carbonaceous
cly	clay
fe	iron oxide
mn	manganese
slt	siltv

Shape

cu	curved
ir	irregular
pl	planar
st	stepped
un	undulating

Roughness

ро	polished
ro	rough
sl	slickensided
sm	smooth
VI	very rough

Other

fg	fragmented
bnd	band
qtz	quartz

Symbols & Abbreviations

Graphic Symbols for Soil and Rock

General		Sedimentary	Rocks
	Asphalt	Q94	Boulder conglomerate
000000	Road base		Conglomerate
0.0.0.0	Concrete	0.	Conglomeratic sandstone
	Filling		Sandstone
Soils			Siltstone
0000	Topsoil		Laminite
	Peat		Mudstone, claystone, shale
	Clay		Coal
	Silty clay		Limestone
	Sandy clay	Metamorphic	Rocks
	Gravelly clay		Slate, phyllite, schist
	Shaly clay	+ + +	Gneiss
	Silt		Quartzite
-//.	Clayey silt	Igneous Roc	ks
	Sandy silt	++++	Granite
	Sand	<	Dolerite, basalt, andesite
	Clayey sand	× × × × ×	Dacite, epidote
	Silty sand	V V V	Tuff, breccia
10,000	Gravel	PO	Porphyry
0.50°C	Sandy gravel		
[QĞQ	Cobbles, boulders		
	Talus		

CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 192.5 AHD PIT No: 1

278381

PROJECT No: 71706

EASTING: NORTHING: 6253145

DIP/AZIMUTH: 90°/--

DATE: 19/4/2010 SHEET 1 OF 1

_												
Ш	Dep		Description	을				& In Situ Testing	h	Dynamic Panatromater Teet		
교	(m	1)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20		
			TOPSOIL - firm to stiff, dark brown, sitty clay with some rootlets, damp		D	0.25						
192		0.45	SILTY CLAY - very stiff to hard, orange brown silty clay, medium to high plasticity		D	0.5						
	-1		1.1m: mottled red brown and grey		D	1.0				-1		
	-2	1.6	SHALE - extremely low to very low strength, highly weathered, grey shale with some ironstone bands		D	2.0				-2		
190												
169	-3				Đ	3.0				-3		
8	-4	4.0	Pit discontinued at 4.0m - target depth reached		_D	-4.0-				-		
188												

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3

Survey levels taken from survey plans provided by Urbis Pty Ltd

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dia.)
 Water sample
 Care drilling
- PECSTING LEGEND

 pp Pocket penetrometer (kPa)

 PID Prato ionisation detector

 S Standard penetration test

 PL Point load strength (s/50) MPa

 V Sirear Vane (kPa)

 D Water seep \$ Water level

Initials: RCB O.S. E sate

CHECKED



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 189.5 AHD PIT No: 2

EASTING: 278418

PROJECT No: 71706

NORTHING: 6253277 DATE: 19/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

Γ	<u> </u>	Description	. <u>e</u>		San	pling &	& In Situ Testing						
뭅	Depth (m)	of	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dy	namic P (blow	enetror s per 0	neter T mm)	est
-		Strata TOPSOIL - firm, dark brown, silly clay with some rootlets, damp		D	0.25	Sa	Comments			5 10) 15	5	20
189	0,37	SILTY CLAY - stiff, orange brown silty clay, low to medium plasticity		B D	0.4 0.5 0.6								
168	-2 2.0	SHALE - extremely low strength, extremely weathered, grey shale with some ironstone banding						- 100	-2				
186	-3	grey shall with some nonstance variating			7777				3				
188	-4 4,0	Pit discontinued at 4.0m - target depth reached				and the second			-				

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND pp Pockat penetrometer (kPa) pp Pockat penetrometer (kPa) PlD Phote ignisation detector S standard penetration test mm dia.) PL Peint load strength (kPa) V Shear Vano (kPa) Vater seep Water level

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dia.)
 Water sample
 Core drilling





CLIENT:

Owston Nominees No. 2 Pty Ltd PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 189.0 AHD PIT No: 3

EASTING: 278591

PROJECT No: 71706 NORTHING: 6253273 DATE: 19/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

П		Description	1	.o		San	npling &	& In Situ Testing	1.	
쿋	Depth (m)	of		Graphic Log	Type			-	Water	Dynamic Penetrometer Test (blows per 150mm)
		Strata		<u></u>	ķ	Depth	Sample	Results & Comments	>	5 10 15 20
	0.2	TOPSOIL - firm, dark brown, rootlets, damp								
		SILTY CLAY - stiff to hard, o plasticity	range brown silfy clay, high		D	0.25				፟
					D	0.5		pp>400kPa		
		- mottled orange brown and	grey from 0.8m		U ₅₀	0.87				<u> </u>
188					۵	1.0				
281	1.2	SHALE/SILTSTONE - extrem strength, grey and orange bro some ironstone banding	ely low to very low own shale/siltstone with		D	2.0				-2
188	3.4	- high strength ironstone ban	đ		D	3.0				-3
		Pit discontinued at 3.4m - practical refusal on ironstor	e tand		-	-3.4				
199	4									-4

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd □ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm die.)
 Water sample
 Core drilling
- pp Pockst penelrometer (kPa)
 pp Pockst penelrometer (kPa)
 pp Pockst penelrometer (kPa)
 pp Pockst penelrometer (kPa)
 pp Point load strength (s/50) MPa
 V Shear Vane (kPa)
 p Water seep Water level





CLIENT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

PROJECT:

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 196.0 AHD PIT No: 4

EASTING:

278776

PROJECT No: 71706 DATE: 19/4/2010

NORTHING: 6253233 DATE: 19/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

T	D		Description	.jc		San		& In Situ Testing	١.					
-	Dep (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20				
<u>\$</u>			TOPSOIL - stiff, dark brown, silly clay with some rootlets		Đ	0.25	S			5 10 15 20				
		0.35	SILTY CLAY - stiff to very stiff, mottled red brown and grey silty clay, low to medium plasticity		D	0.5								
198	1	1.1	SHALE - medium strength, slightly weathered to fresh, grey shale		Đ	1.0				-1				
			Pit discontinued at 1.1m - refusal in medium strength shale											
- 10 m	2									-2				
193	3									-3				
192							MAAA AT			-4				

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3 □ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

Pickat penetrometer (kPa)
PiD Phote ionisation detector
S Standard penetration test
Pi, Point load strength (s/50) MPa
V Sheer Vane (kPa)
D Water seep Water lovel

CHECKED Initials: RCB 01.8.E :Date



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT:

Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 195.0 AHD PIT No: 5 EASTING:

278892

PROJECT No: 71706

NORTHING: DIP/AZIMUTH: 90°/--

6253156

DATE: 19/4/2010 SHEET 1 OF 1

			Description	o o		San	npling a	& In Situ Testing		
쿋	De (n	pth n)	of	Graphic Log	Туре				Water	Dynamic Penetrometer Test (blows per 150mm)
52		_	Strata	Ō	Ļ	Depth	Sample	Results & Comments	>	5 10 15 20
17			TOPSOIL - firm to stiff, dark brown, silty clay with some rootlets, damp	W	Ę	0.0				
$\ \cdot\ $			•	W	D	0.25				[
		0.4		XX		0.4				
\mathbb{H}			SILTY CLAY - stiff to very stiff, mottled red brown and grey, silty clay, medium to high plasticity		E_	0.5				
H				1/1/						
П										
$\left\{ \cdot \right\}$		0.9	SUALE outcomety less to see the street less to	<u> </u>						
134	1		SHALE - extremely low to very low strength, highly to moderately weathered, grey shale with some ironstone	===	D	1.0				-1
			bands	===						
} }										[
H										
										' i i i i i
}}			1.6m: medium to high strength							
\mathbf{H}				===						
8	2				D	2.0				-2
-		2.1	Pit discontinued at 2.1m			2.0				[2
H			- practical refusal in medium to high strength shale							
\mathbf{H}										
1										<u> </u>
П										
\mathbb{H}				i						
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RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

E = Environmental sample

Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

pp Pocket penetrometer (kPa)
PID Photo ionisation detector
S Standard penetration test
V Shear Vane (kPa)
V Water seep
Water seep
Water seep

CHECKED Initials: DCR Of. 78. E :BD



CLIENT:

Owston Nominees No. 2 Pty Ltd PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 181.0 AHD PIT No: 6

EASTING: 279079 NORTHING:

6253239

PROJECT No: 71706 DATE: 19/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

								111. 80 /	•	SHEET FOR I				
	Depti	П	Description	Pic C		San		& In Situ Testing	<u></u>	Dynamic Penetrometer Test				
H. R.	(m)	"	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	(blows per 150mm)				
#			TOPSOIL - stiff, dark brown, silty clay with some rootlets, damp		D	0.25				4				
		0.3	SILTY CLAY - stiff, red brown silty clay, low to medium plasticity		D	0.5								
180	0.5	95	SHALE - extremely low strength, extremely weathered, grey shale with a trace of orange brown silty clay - medium strength, slightly weathered from 1.2m		D	1.0				-, -				
	. 1	1.4	Pit discontinued at 1.4m - practical refusal in medium strength shale		-D-	-1.4-								
179	-2									-2				
176	-3									-3				
177	-4	and the second s				- The State of the				-4				
ŀ		-												

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dia.)
 Water sample
 Core driting

- PD Podkat penetrometer (kPa)
 PID Photo ionisation detector
 S Standard penetration test
 PL Point load strength (s(50) MPa
 V Shear Vane (kPa)
 D Water seep # Water level





CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT:

Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 190.5 AHD PIT No: 7

EASTING: 278920 NORTHING: 6253280 PROJECT No: 71706 DATE: 19/4/2010 SHEET 1 OF 1

DIP/AZIMUTH: 90°/--

	Depth	Description	ie.		San	npting a	& In Situ Testing	[_			
2	(m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water		nic Pene (blows po	er Omm)	
-	-	TOPSOIL - firm, dark brown, silty clay with some rootlets, damp		D	0.25	- Ø				10	15	20
190	0.4	SILTY CLAY - stiff, mottled red brown and grey, silty clay, low to medium plasticity		B D	0.4 0.5 0.6							
	-1			D	0.8				-1			
169	1.3	35 1.4 SHALE - medium strength, slightly weathered, grey shale Pit discontinued at 1.4m - refusal on medium strength shale					N. IV.					
	-2								-2			
188												
	-3								-3			
187												
	-4								-4			
981												

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd □ Sand Penetrometer AS1289.6.3.3 ☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Bulk sample
 Tube sample (x mm dis.)
 Water sample
 Core drilling
- Pickat penetrometer (kPa)
 PiD Phote ionisation detector
 S Standard penetration tast
 P Point load strength is(50) MPa
 V Shear Vane (kPa)
 V Water seep
 Water level

CHECKED Initials: Per 01.8.8:ata



CLIENT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

PROJECT:

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 180.5 AHD PIT No: 8

EASTING: 279045

6253429

PROJECT No: 71706 DATE: 19/4/2010 SHEET 1 OF 1

NORTHING: DIP/AZIMUTH: 90°/--

$\overline{}$	$\overline{}$			_						
١.	Do	Depth	Description	윤 _		Sampling & In Situ Testing				
₩	(r	n)	of	Graphic Log	Type	Depth	Sample	Results &	Water	Dynamic Penetrometer Test (blows per 150mm)
L	_		Strata	Ō	Ě	De la	Sam	Results & Comments	>	5 10 15 20
			TOPSOIL - stiff, dark brown, silty clay with some rootlets, damp		D	0.25				
180		0.4	SILTY CLAY - stiff, red brown, silty clay with a trace of rootlets, medium plasticity		D	0.5				
			- mottled red brown and grey, with a trace of ironstone		D	1.0				
179	-2	1.6	SHALE - extremely low to very low strength, extremely weathered, grey shale							
178					D	2.0				
	-3 -	3.1	2.9m: low to medium strength, slightly weathered		_					-3
			Pit discontinued at 3.1m - practical refusal on medium strength shale		-D-	3.1-				
441				;						
	-4									-4
178										
1 1		- 1		- 1		ı 1				

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

PD Pocket penetrometer (kPa)
PD Phote ionisation detector
S Standard penetration test
PL Point load strength (s/50) MPa
V Shear Vane (kPa)
D Water seep
Water seep
Water level

CHECKED Initials: DCR 01.8,&: oteQ



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

EASTING;

278556

SURFACE LEVEL: 185.5 AHD PIT No: 9

PROJECT No: 71706

NORTHING: 6253390 DATE: 19/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

П			Description	.0		San	npling i	& In Situ Testing		
교	Dej (m	pth 1)	of	Graphic Log	Type	t)c	eldi	Results &	Water	Dynamic Penetrometer Test (blows per 150mm)
Ц			Strata	Ō	Tyl	Depth	Sample	Results & Comments	>	5 10 15 20
$ \cdot $			TOPSOIL - stiff, brown, silty clay with some rootlets, humid to damp	M						
H		0.2			D	0.25				
[]		-	SILTY CLAY - very stiff to hard, orange brown silty clay, low to medium plasticity	[9]	_	0.20				
활			anna innatana haadlaa faan 8 5	XX	D	0.5				h
H			- some ironstone banding from D.5m	XX						├
]]										<u> </u>
} }	,			177						
Ħ	-1			KXI	D	1.0				⊦¹
	,			KX						
╁		1.3	SHALE - extremely low strength, extremely weathered,							
į,		-	grey shale							
M		Ì		薑						
łŀ			- ironstone bands between 1.6m and 2.5m							
ÌÌ				===						
}	-2				D	2.0				-2
H										
[[
╁				===						
쁄										
[[2.6	SANDSTONE - low to medium strength, moderately weathered, fine grained sandstone		_D_	-2.7-				
H		Í	Pit discontinued at 2.7m							
ÌÌ	-3		- practical refusal on medium strength sandstone							
										-3
H										
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-8										
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t	-4									-4
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}										
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ļļ										
Ш										

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample Disturbed sample Bulk sample Tube sample (x mm dia.) Water sample Core drilling

pp Pocket penetrometer (kPa) PID Photo ionisation detector

PI Priori bisalan decelor
S Standard penetration test
PL Point load strength Is(50) MPa
V Shear Vene (kPa)
V Water seep

Water level

Initials: LCB Date: 3.8.10



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT:

Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 189.0 AHD PIT No: 10

EASTING: 278815

6253493 NORTHING: DIP/AZIMUTH: 90°/--

PROJECT No: 71706 DATE: 19/4/2010 SHEET 1 OF 1

П				_					
	Depth	Description	ohic g	<u> </u>			& In Situ Testing	Į.	Dynamic Penetrometer Test
189 RL	(m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	(blows per 150mm) 5 10 15 20
	. ,	TOPSOIL - firm to stiff, dark brown, silty clay with some rootlets, damp	M	E	0.0		-		
П		roototo, dump	<i>}}</i>	D	0.25				
11	0.39		XX						
	. 0,35	SILTY CLAY - soft to firm, mottled red brown and grey, silty clay with a trace of ironstone gravel, medium to high plasticity	1/1/	E	0.4				
11	.	high plasticity	XX	D	0,5				[] [
11			1//						· : : : : : : : : : : : : : : : : : :
			11/1						
188	-1		XX	D	1.0				
1			1//						.
			133						ነ ነ
1			XX						
\mathbf{H}	i								· -
	1.6	SHALE - low to medium strength, slightly weathered,							
$\left\{ \cdot \right\}$	1.8	shale Pit discontinued at 1.8m		_b_	-1.8-				
		- refusal on medium strength shale							
	2								-2
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} }									
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1									
									}

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

E = Environmental sample

Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk semple
Tube sample (x mm dia.)
Water sample
Core drilling

PlD Pockat penetrometer (kPa)
PlD Phote ionisation detector
S Standard penetration test
PL Point load strength (s(50) MPa
V Sheer Vana (kPa)

Water seep
Water seep
Water level





CLIENT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

PROJECT:

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 186.0 AHD PIT No: 11 EASTING:

278939 PROJECT No: 71706

NORTHING: 6253518 DATE: 19/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1 DATE: 19/4/2010

	Dent	th	Description	hic C		San		& In Situ Testing	ļ.,	Dynamic	Panatro	meter	Toet
 5	Dept (m))	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	(blo	ws per 0	0mm) 15	20
#			TOPSOIL - firm to stiff, dark brown, silty clay with some rootlets, damp		E	0.2							20
		0.9	SILTY CLAY - very stiff, orange brown, silty clay with a trace of ironstone gravel, medium plasticity		E_D	0.4							
186		0.5	SHALE - extremely low strength, extremely weathered, grey shale with some orange brown silty clay seams		D	1.0				-1			
ŀ	1	1.5-	1.3m: medium strength Pit discontinued at 1.5m							- :			
-			- refusal on medium strength shale										
<u>ĕ</u> -2	2									-2			
Amandum.J.										_			
- 183	3		•							-3		b b c	
-													
ĺ													
28-4	1									4			
don de la constante de													
-													

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

E = Environmental sample Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3 ☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

Picket penetrometer (kPa)
pp Pocket penetrometer (kPa)
phote ionisation detector
s standard penetration test
PL Point load strength is(50) MPa
V Shear Vane (kPa)
valer seep
Water seep

CHECKED Initials: LCB Date: 3.8,10



CLIENT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

PROJECT:

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 174.5 AHD PIT No: 12

EASTING: 279174 NORTHING: 6253552 DIP/AZIMUTH: 90°/--

PROJECT No: 71706 DATE: 19/4/2010 SHEET 1 OF 1

	Depth	Description	jệ c		Sam		& In Situ Testing)tr	Dunamia Panetrometer Text
Ζ	(m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
+		TOPSOIL - firm to stiff, dark brown, silty clay with some rootlets, damp		D	0.25	Ĭ			5 10 15 20
224	0.33	SILTY CLAY - very stiff, mottled orange brown and grey silty clay, medium to high plasticity		D	0.5		pp>400kPa		
	0.8	SHALE - extremely low strength, extremely weathered, grey shale with a trace of orange brown silty clay		Uso	0.77				
-									7
	1.7	1.6m: low to medium strength Pit discontinued at 1.7m							
	-2	- practical refusal on medium strength shale							-2
-									
-									
-	3								-3

RIG: Case 58 Backhoe

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

Auger sample
Disturbed sample
Bulk sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

SAMPLING & IN SITU TESTING LEGEND

pp Pocket penetrometer (kPa)
pp Standard penetration test
pr Point load strength is(50) MPa
v Shear Vane (kPa)
p Water seep Water level

CHECKED Initials: QCB Date: 3, 8, 10

LOGGED: AP



CLIENT: PROJECT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 184.5 AHD PIT No: 13

EASTING: 278656 NORTHING: 6253547

DIP/AZIMUTH: 90°/--

PROJECT No: 71706 DATE: 20/4/2010 SHEET 1 OF 1

П									
	Depth	Description	·윤 도				& In Situ Testing	, h	Danamia Panetsometes Ta-+
교	(m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20
	0.3	TOPSOIL - soft to firm, dark brown, silty clay with stootlets, damp		E	0.2				
184	1	SILTY CLAY - very stiff, orange brown, silty clay w some ironstone gravel	rith	E	0.4				
183	1.	SANDSTONE - low strength, extremely weathered grey, fine grained sandstone						NAME OF THE OWNER O	
	2 2,1	- low to medium strength Pit discontinued at 2.0m - practical refusal on medium strength sandstone							2
182									
-	3								-a
161									
	4								-4
180									

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

E = Environmental sample Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3 □ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

pp Pockst penetrometer (kPa)
pp Pots penetrometer (kPa)
pp Pots ionisation detector
S Standard penetration test
PL Point load strangth is(50) MPa
V Shear Vane (kPa)
D Water seep
Water level

CHECKED Initials: RCB 01.8.8 Date:



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 185.0 AHD PIT No: 14 EASTING:

278759

PROJECT No: 71706

NORTHING: 6253597 DATE: 20/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

П		Description	.0		San	npling a	& In Situ Testing	Τ.	
띺	Depth (m)	of	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 0mm)
<u>\$2</u>		Strata TOPSOIL - firm, dark brown, silty clay with some	W X	F	ă	Sai	Comments	L	5 10 15 20
	0.42	rootlets, damp		D	0.25				
	0.42	SILTY CLAY - stiff, orange brown, silty clay with some ironstone gravel and cobbles		В	0.5				
					0.7				
184	-1			D	1.0				-1
	1.4	SHALE - extremely low strength extremely weathered, grey shale		D	1.5				
183		2.3m: low to medium strength with a trace of grey, fine grained sandstone							-2
	2.5	Pit discontinued at 2.5m - practical refusal on medium strength sandstone							
182	3								3
181	4				and a				-4
<u> </u>									

RIG: Case 58 Backhoe

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample

Core drilling

pp Pocket genetrometer (kPa)
pp Pocket genetrometer (kPa)
PlD Phote ionisation detector
S Standard penetration test
PL Point load strength is(50) MIPa
V Shear Vane (kPa)
D Water seep # Water level

CHECKED Initials: RCB Date: 3.8-10

LOGGED: AP



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 179.0 AHD PIT No: 15 EASTING:

279064 PROJECT No: 71706

NORTHING:

6253652

DATE: 20/4/2010

				DIP/AZIMUTH: 90°/ SHEET 1 OF 1	
	Donath	Description	5	Sampling & In Situ Testing	
펎	Depth (m)	of	Sraph	B E Results & Dynamic Penetrol (blows per 0	

П		Т	Donatician		_	Sarr	nolina i	& In Situ Testing	<u> </u>			
균	Dept	th	Description of	Graphic Log	d)				Water	Dynamic	Penetromet	er Test
	(m)		Strata	S .	Type	Depth	Sample	Results & Comments	Š	5 (blo	ws per 0mm	20
	_	.32	TOPSOIL - firm, dark brown, silly clay with some rootlets, damp							-		
	۷.	.52	SILTY CLAY - stiff to very stiff, orange brown silty clay, low plasticity		В	0,4				-		
178	1											
•		1.3	CHAIR contramely law to you law atravets outcome.									
			SHALE - extremely low to very low strength, extremely to highly weathered, grey shale									
	2									-2		
	2	2.3	- medium strength, slightly weathered									
			Pit discontinued at 2.3m - practical refusal on medium strength shale									
178	3									3		
- 272	4									4		
·												
		\perp					L			11		

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample

Core drilling

PD Pocket penetrometer (kPa)
PID Photo ionisation detector
S Standard penetration test
V Shear Vane (kPa)
D Water seep # Water level

CHECKED Initials: ROB Date: පි.රි. ර



CLIENT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

PROJECT:

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 172.0 AHD PIT No: 16 EASTING:

279206

PROJECT No: 71706

NORTHING: DIP/AZIMUTH: 90°/--

6253740

DATE: 20/4/2010 SHEET 1 OF 1

			Description	-E		San	npling a	& In Situ Testing	Γ_	
굺	(n	pth n)	of	Graphic	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
42		-	Strata	0	È	8	Sar	Comments	Ĺ	5 10 15 20
		0.36	TOPSOIL - stiff, brown, silty clay with some rootlets, humid to damp		E	0,2 0,25 0,3				
171	-1	1.1	SILTY CLAY - very stiff, orange brown, silty clay with some ironstone gravel and cobbles, low to medium plasticity		E D	0.4				-1
		1.95	SHALE - extremely weathered, extremely low strength, grey shale with some fine grained sandstone bands 1.9m: low to medium strength							
Ē	-2	1.85	Pit discontinued at 1.95m - practical refusal on medium strength shale						\vdash	-2
691	3									-3
163	-4									-4

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

E = Environmental sample

Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

p Pocket penetrometer (kPa)
pp Pocket penetrometer (kPa)
PID Phot: ionisation detector
S Stancard penetration test
PL Pointload strength (s(50) MPa
V Shear Vane (kPa)
Vator seep
Water seep
Water level

CHECKED Initials: ROR Date: 3.8. 16



CLIENT:

Owston Nominees No. 2 Pty Ltd Land Capability Assessment

PROJECT:

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 176.0 AHD PIT No: 17

279027 EASTING: 6253814 NORTHING: DIP/AZIMUTH: 90°/--

PROJECT No: 71706 DATE: 20/4/2010 SHEET 1 OF 1

П		-					n I- Olive Tears		
湿	Depth	Description of	Graphic Log				& In Situ Testing	Water	Dynamic Penetrometer Test (blows per 150mm)
	(m)	Strata	Sa L	Type	Depth	Sample	Results & Comments	8	(blows per 150mm) 5 10 15 20
-	0.37	TOPSOIL - stiff, brown, silty clay with some rootlets, humid to damp				93			
	0.37	SILTY CLAY - stiff to very stiff, orange brown, silty clay with some ironstone gravel, medium plasticity							
176	1.3	SHALE extremely law to law atravells are sent							-1
		SHALE - extremely low to low strength, extremely weathered, grey shale				200			
174	2.4	2.3m: low to medium strength, trace of ironstone							-2
	2.4	Pit discontinued at 2.4m - practical refusal on medium strength shale							
173	3								-3
172	4								-4
	0.00								

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

□ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dia.)
 Water sample
 Core drilling
- Pocket penetrometer (kPa)
 PD Phote ionisation detector
 S Standard penetration test
 PL Point load strength is(50) MPa
 V Sheai Vane (kPa)

 Water seep
 Water seep
 Water level

CHECKED Initials: RCB Date: 3.8.60



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 173.5 AHD PIT No: 18

EASTING: 279208

NORTHING: 6253859

PROJECT No: 71706 DATE: 20/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

٦		Description	.0		San	pling &	& In Situ Testing	Τ.			
딦	Depth (m)	of	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dy	namic Penetron (blows per 0r	eter Test nm)
		Strata TOPSOIL - firm, brown, silty clay with some rootlets, humid to damp		F	Δ	S	Connents	-		5 10 15	20
	0.23	SANDY CLAY - stiff, orange brown, sandy clay with some ironstone gravel, low plasticity		D	0.25						
				B D	0,5 0.6						
	0.8- 0.85	SANDSTONE - medium to high strength, slightly weathered, yellow brown, fine grained sandstone Pit discontinued at 0.85m	<u> </u>			İ			-1		
		- refusal on medium to high strength sandstone									
172											
-											
-	2								-2		
Ĕ.			ļ								
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RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dia.)
 Water sample
- Core drilling

- PING LEGEND
 pp Pocket penetromater (kPa)
 PID Phote ionisation detector
 S Stancard penetration test
 PL Point(load strength ls(50) MPa
 V Shear Vane (kPa)
 D Water seep \$ Water level





CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 174,0 AHD PIT No: 19

EASTING: 279289 PROJECT No: 71706

NORTHING: 6253955 DATE: 20/4/2010 DIP/AZIMUTH: 90°/--SHEET 1 OF 1

		Description	٥.		Sam	nplling i	& In Situ Testing	Τ.	
쿈	Depth (m)	of	Graphic Log	g l	Ę	음	Posuito £	Water	Dynamic Penetrometer Test (blows per 150mm)
27	(,	Straţa	\@_	Туре	Depth	Sample	Results & Comments	5	5 10 15 20
	0.22	TOPSOIL - firm to stiff, brown, silty clay with some rootlets, humid							
	0.22	SANDY CLAY - stiff to hard, orange brown, sandy clay with some ironstone gravel and cobbles, low to medium plasticity							
12	- 0.8	SANDSTONE - extremely low to very low strength, grey, fine grained sandstone							
	[-1
+1	1.3	1.25m: low to medium strength sandstone						 -	
		Pit discontinued at 1.3m - practical refusal on medium strength sandstone							
172	- -2								-2
Ė	-3								-3
			ĺ						
170	-4								-4
}									
	_								

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd □ Sand Penetrometer AS1289.6.3.3 □ Cone Penetrometer AS1289.6.3.2

Core drilling

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample

SAMPLING & IN SITU TESTING LEGEND

pp Pocket penetrometer (kPa)

le PID Photo ionisation defector

S Standard penetration test

mm dia.) PI, Pointload strength 1s(50) MPa

V Shear Vane (kPa)

D Water seep Water level

Initials: RCB 04.8.8 :Date:



CLIENT: Owston Nominees No. 2 Pty Ltd PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 177.5 AHD PIT No: 20

EASTING: 279004 PROJECT No: 71706 NORTHING: 6253925 DATE: 20/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

_								in: 90 /	•	SHEET OF
	De	pth	Description	hic				k In Situ Testing	>=	Dynamic Pagetrameter Test
Ζ	(1)	m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20
П		0.19	TOPSOIL - stiff, brown, silty clay with some rootlets, damp				<u> </u>		-	
		0.10	SILTY CLAY - stiff to hard, orange brown, silty clay with ironstone gravel, low to medium plasticity		D	0,25				
Ē		İ			D	0.5				
[]										
-		Į								
	-1									-1
-		1.3	SANDSTONE - extremely low strength, extremely	22						
178			SANDSTONE - extremely low strength, extremely weathered, grey, fine grained sandstone							
1 1		1.7	1.6m: medium to high strength							
[[1.7	Pit discontinued at 1.7m						\vdash	
} }			 practical refusal on medium to high strength sandstone 							
H	-2									-2
11				- 1						
l.f		-								
						- 1				
} }										
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[[Ì				

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

SAMPLING & IN SITU TESTING LEGEND

Core drilling

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dis.)
 Water sample
- PD Pocket penetrometer (kPa)
 PD Phote terrisedion detector
 S Stancard penetration test
 PL Point load strength (s(50) MPa
 V Shea Vano (kPa)
 D Water seep # Water level

CHECKED Initials: RCB Date: 3.8.10



CLIENT:

Owston Nominees No. 2 Pty Ltd PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 177.0 AHD PIT No: 21

278946

PROJECT No: 71706

EASTING: 6254047 NORTHING: DIP/AZIMUTH: 90°/--

DATE: 21/4/2010 SHEET 1 OF 1

П	_		Description	. <u>0</u>		San	apling a	& In Situ Testing	Τ.				
교	Dept (m)	th [of	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynam (t	ic Penetro lows per	ometer Omm)	Test
4		4	Strata	0	ŗ	å	San	Comments		5	10	15	20
	. 0.	.10	TOPSOIL - firm, brown, clayey sand with some rootlets, damp			0.2							
			SANDY CLAY - stiff, orange brown sandy clay, low plasticity		E	0.3							
			0.5m: ironstone gravel and cobbles from 0.5m		-	0,5							
178													
-		1.1	SANDSTONE - low to medium strength, slightly to moderately weathered, grey sandstone	///						-1			
	. ,	1.4	Pit discontinued at 1.4m					a			-		
			- practical refusal on medium strength sandstone										
175	-2									-2			
-													
174	-3								-	-3			
[2]	-4									-4			
-													
-													
-													

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

Undisturbed sample refused at 0.5m, no recovery Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3 ☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

PD Pockst penetrometer (kPa)
PD Phote ionisation detector
S Standard penetration test
PL Point load strength (s50) MPa
V Shear Vana (kPa)
D Water seep # Water level

Initials: RCB O). 3, E :Date:



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 176.0 AHD PIT No: 22 EASTING:

279019 PROJECT No: 71706

NORTHING: 6254154 DATE: 20/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

П		Description	.io		San	npling	& In Situ Testing	Τ.	
귈	Depth (m)	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
11		TOPSOIL - brown, silty clay with some rootlets, humid	M	_		ගී			6 10 15 20
	0.21	SANDY CLAY - very stiff, mottled orange brown and red brown, sandy clay with some ironstone gravel, low plasticity		D B D	0.25 0.4 0.5 0.6				
175	0.9	SANDSTONE - extremely low strength, extremely weathered sandstone							-1
1		4 Con modilium to bish story at							
+	1.7	1.6m: medium to high strength Pit discontinued at 1.7m	[-	
174	2	- refusal on medium strength sandstone	,		100				-2
173	3				10.00				-3
172	4								4

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

Survey levels taken from survey plans provided by Urbis Pty Ltd REMARKS:

□ Sand Penetrometer AS1289.6.3.3

☑ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND
pp Packet penetrometer (kPa)
pp Phote ionisation detector
S Standard penetration test
pp Point load strength ts(50) MPa
V Shear Vane (kPa)
V Water seep Water level

- Auger sample
 Disturbed sample
 Bulk sample
 Tubé sample (x mm dia.)
 Water sample
 Core drilling

Initials: ECB σ1. 8. €. :BD



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 178.0 AHD PIT No: 23 EASTING:

279232

PROJECT No: 71706

NORTHING: 6254076 DATE: 20/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

	Dr	epth	Description	을 등		San		& In Situ Testing] [Dumamia Bor	offomata	r Toet
2	()	m)	of Strata	Graphic	Type	Depth	Sample	Results & Comments	Water	Dynamic Pen (blows)	per Omm)	20
41	-		TOPSOIL - firm, brown, silty clay with some rootlets, humid									
		0.37	SILTY CLAY - stiff, orange brown, silty clay with some ironstone gravel, low plasticity			0.7		pp>400kPa				
121	- -1 -				Uso	1.08				-1		
		1.3	SHALE - extremely low strength, extremely weathered, grey shale with some ironstone bands and a trace of fine grained, grey sandstone									
		1.9	- medium to high strength Pit discontinued at 1.9m									
176		A British	- refusal on medium to high strength shale						The state of the s	-3		
P.23	-4									4		

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Eulis sample
Tube sample (x mm dia.)
Water sample
Core drilling

PD Pocket penatrometer (kPa)
PID Photo ionisation detector
S Standard penetration test
PL Point load strength (s50) MPa
V Shear Vane (kPa)
D Water seep
Water seep
Water level

Initials: RCB Date: 3.8,60



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 167.0 AHD PIT No: 24 EASTING:

279453

PROJECT No: 71706

NORTHING: 6254029 DATE: 21/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

Debili	Description	1 ž		Oan		& In Situ Testing	35	Dunamia	Donotro	motor	Tool
Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic (bk			20
	TOPSOIL - firm, brown, silty clay with some rootlets, damp	m							-	-	:
0,18	SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity		D	0,25							
			D	0.5 0.6		pp>400kPa					
			Usa								
1,1	SANDSTONE - extremely low strength, extremely weathered, grey sandstone	22	_D_	1.0				-1			
	•										
1.65	1.6m: low to medium strength, grey and yellow brown							-			-
	- practical refusal on medium strength sandstone										
2								-2			
								-			
.								-3			
.								-4			
									:		
	1.65	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, slity clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, slity clay with a trace of sand, low to medium plasticity 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.5 0.6 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, slity clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, slity clay with a trace of sand, low to medium plasticity D 0.25 0.6 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.25 D 0.5 0.6 pp>400kPa 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.25 D 0.5 0.6 pp>400kPa 1.1 SANDSTONE - extremely low strength, extremely weathered, grey sandstone 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, motited orange brown and grey, silty clay with a trace of sand, low to medium plasticity D	TOPSOIL - firm, brown, silty clay with some rootlets, damp SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.25 SILTY CLAY - stiff to very stiff, mottled orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.5 0.6 pp>400kPa 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone	TOPSOIL - firm, brown, silty clay with some roctlets, damp SILTY CLAY - stiff to very stiff, motited orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.25 SINTY CLAY - stiff to very stiff, motited orange brown and grey, silty clay with a trace of sand, low to medium plasticity D 0.5 0.6 pp>400kPa 1.6m: low to medium strength, grey and yellow brown Pit discontinued at 1.65m - practical refusal on medium strength sandstone

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Core drilling

Auger sample
Disturbed sample
Butk sample
Tube sample (x mm die.)
Water sample

pp Pocks penetrometer (kPa)
pp Pots ionisation detector
S Standard penetration test
PL Point load strength is(50) MPa
V Shear Vano (kPa)
p Water seep
Water level

CHECKED Initials: 2CB 01.8.E :Date:



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 164.0 AHD PIT No: 25 EASTING:

279589

PROJECT No: 71706 DATE: 21/4/2010

NORTHING: 6254173 DIP/AZIMUTH: 90°/--

SHEET 1 OF 1

T		Description	. <u>e</u>		Sam	pling &	& In Situ Testing	Τ_	
	Depth (m)	of	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
7		Strata TOPSOIL - firm, brown, silty clay with some rootlets,	m	_	_	S		+	5 10 15 20
	0.19	damp SILTY CLAY - stiff to very stiff, crange brown and red brown, silty clay with some ironstone gravel and a trace of sand, low to medium plasticity		E_D	0.2 0.3 0.4 0.5				
22-1	0.9	SANDSTONE - extremely low strength, extremely weathered, grey, fine to medium grained sandstone	///	D	1.0				-1
2	2.1	2.0m: medium strength Pit discontinued at 2.1m							-2
3		- refusal on medium strength sandstone							-3
70.									-4

RIG: Case 58 Backhoe

LOGGED; AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS:

E = Environmental sample

Survey levels taken from survey plans provided by Urbis Pty Ltd

☐ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

PD Pockat penetrometer (kPa)
PID Photo ionisation detactor
S Standard penetration test
PL Pointload strength Is(50) MPa
V Shear Vane (kPa)
D Water seep
Water seep
Water level

Initials: RA Date: 3,8,10



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 172.5 AHD PIT No: 26 EASTING:

279397 PROJECT No: 71706

NORTHING: 6254270 DATE: 21/4/2010 SHEET 1 OF 1

Donth	Description	.e_		Sam		& In Situ Testing	Ŀ	P	ie Denci		
Depth (m)	of Strata	Graphic	Type	Depth	Sample	Results & Comments	Water		ic Penet blows pe		
	TOPSOIL - firm, brown, silty clay with some rootlets, damp				Š			5	10	15	20
0.25	SILTY CLAY - stiff to very stiff, orange brown, silty clay with a trace of ironstone gravel, medium plasticity		D	0.25							
- 0.9 -1 1.0 -	SHALE - medium strength, slightly weathered to fresh, grey shale Pit discontinued at 1.0m - refusal on medium strength shale							1			-
-2								-2			
-3 -								-3			
-4								-4			

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6,3,3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

- Auger sample
 Disturbed sample
 Bulk sample
 Tube sample (x mm dia,)
 Water sample
 Core drilling

- Pocket penetrometer (kPa)

 PID Photo ionisation detector

 S Standard penetration test

 PL Point oad strength is(50) MPa

 V Shear Vane (kPa)

 D Water seep Water level

Initials: RCB Date: 3.8.10



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 177.5 AHD PIT No: 27 EASTING:

279271

PROJECT No: 71706

NORTHING: 6254208 DIP/AZIMUTH: 90°/--

DATE: 20/4/2010 SHEET 1 OF 1

		Description	. <u>o</u>		Sam	pling &	& In Situ Testing	Ī.	
교	Depth (m)	of	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
H		Strata TOPSOIL - firm to stiff, brown, sity clay with some rootlets, damp	m	F	ŏ	Sa	Comments		5 10 15 20
- 117	0.27	SILTY CLAY - very stiff to hard, orange brown, silty clay with a trace of ironstone gravel, medium plasticity							
B21	-1 1.0	SHALE - extremely low strength, extremely weathered, grey shale							-1
	-2	2.0m: medium strength							-2
176	2,4	Pit discontinued at 2.4m - refusal on medium strength shale							
	3								-3
124									
	-4								-4
173									

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

Survey levels taken from survey plans provided by Urbis Pty Ltd REMARKS:

☐ Sand Penetrometer AS1289.6.3.3

☑ Cone Penetrometer AS1289.6.3.2

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample
Core drilling

SAMPLING & IN SITU TESTING LEGEND
pp Pocket penetrometer (kPa)
le PID Photo innisation detector
S Standard penetration test
PI. Pointload strength is(50) MPa
V Shear Vane (kPa)
V Water seep \$ Water level

Initials: RCR Date: 3.8.10



CLIENT: Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment LOCATION: Fairlight Road, Mulgoa (Western Precinct) SURFACE LEVEL: 177.0 AHD PIT No: 28

EASTING: 279144 NORTHING: 6254218 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

PROJECT No: 71706 DATE: 20/4/2010

		_												
Ш	De	pth	Description	Graphic Log			_	& In Situ Testing	je.	Dv	namic P	enetro	meter	Test
쮼	(п	n)	of	Log	Type	Depth	Sample	Results & Comments	Water	-,	(blow	s per (mm)	
12			Strata	0	r	å	Sar	Comments			5 10) 1	1,5	20
17			TOPSOIL - firm, brown, slity clay with some rootlets, humid	W						-				
11			numia	KXX										
} }				KKK	D	0.25								
1		0.37	SANDY CLAY - stiff to very stiff, orange brown, sandy	7.7						-				
11			SANDY CLAY - stiff to very stiff, orange brown, sandy clay with some ironstone gravel and cobbles, low to medium plasticity	1://	D	0.5				-				1
11			medium plasticity	1//						-				
11				199										
[[1//										1
<u></u>	-1			1/:/.	D	1.0				[,				'
17				1/./.		1,0				ļ ' l				
11				1//										
} }		1.3	CANDOTONE automotivity to your law attended	7.7.										
} }			SANDSTONE - extremely low to very low strength, extremely weathered, grey, fine grained sandstone							-				
1					D	1.5				-				
11			1.65m: low to medium strength							ŀ				
11		1.7	Pit discontinued at 1.7m										-	-
[[refusal on low to medium strength sandstone 											
12	-2									-2				
17														
11														
1										-				
H										-				
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11									ĺ					
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11										ŀ				
11										1				
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} }										-			:	į
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H														
11										ŀ				
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RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd □ Sand Penetrometer AS1289.6.3.3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dia.)
Water sample Core drilling

PD Pecket penetremeter (kPa)
PID Phob ionisation detector
S Standard penetration test
V Shear Vane (kPa)
V Water seep Water level





CLIENT:

Owston Nominees No. 2 Pty Ltd PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgos (Western Precinct)

SURFACE LEVEL: 169.0 AHD PIT No: 29 EASTING:

279435

PROJECT No: 71706

NORTHING: 6254141 DATE: 20/4/2010 DIP/AZIMUTH: 90°/-- SHEET 1 OF 1

-	Т	Description			San	oplina i	& In Situ Testing						
ᆔ	Depth (m)	of	Graphic Log	9				Water	Dy	namic F	enetro vs per (meter	Test
189	,	Strata	<u>5</u>	Туре	Depth	Sample	Results & Comments	S				15	20
	0,4	TOPSOIL - firm, brown, silty clay with some rootlets, humid							[2 2 2 2 3 4 4 4 4 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7	
	0.4	SILTY CLAY - stiff to very stiff, silty clay with a trace of sand and ironstone gravel, medium to high plasticity		В	0.4								
168	-1								-1				
	1.4	SHALE - medium strength, slightly weathered, grey shale Pit discontinued at 1.4m	ÉÉ										-
		- refusal on medium strength shale							-				
187	-2								-2				
168	-3								-3				
· ·									-				
											,		
165	4								-4				
-									- -				
											; ; ; ; ; ;		
\perp									[:	1	<u> </u>

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3,3

☐ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Auger sample
Disturbed sample
Bulk sample
Tube sample (x mm dis.)
Water sample
Core drilling

pp Pocket penetrometer (kPa)
pp Pocket penetrometer (kPa)
PID Photo ionisation detector
S Stendard penetration test
PL Pointload strength (s(50) MPa
V Shoar Vane (kPa)
D Water seep Water level

Initials: RCB Date: 3.8.10



CLIENT:

Owston Nominees No. 2 Pty Ltd

PROJECT: Land Capability Assessment

LOCATION: Fairlight Road, Mulgoa (Western Precinct)

SURFACE LEVEL: 191.5 AHD PIT No: 30 EASTING:

278832 PROJECT No: 71706

NORTHING: DIP/AZIMUTH: 90°/--

6253386

DATE: 21/4/2010 SHEET 1 OF 1

Ţ	Donth	Description	pic_		Sam		& In Situ Testing	Jn.	Dungmin Bonotrometer Tori
꿊	Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20
		TOPSOIL - firm, dark brown, silty clay with some rootlets, damp		D	0.25				
181	0.36	SILTY CLAY - stiff, mottled red brown and grey, silty clay with a trace of ironstone gravel, low to medium plasticity		D	0.5				
180	-1 1.0	SHALE - extremely low strength, extremely weathered, grey shale with some orange brown sitty clay seams							-1
	2 2.1	2.0m: medium strength Pit discontinued at 2.1m							-2
. B		- refusal on medium trength shale							
	-3								-3
=	-4								-4
187									
-									

RIG: Case 58 Backhoe

LOGGED: AP

WATER OBSERVATIONS: No free groundwater observed

REMARKS: Survey levels taken from survey plans provided by Urbis Pty Ltd ☐ Sand Penetrometer AS1289.6.3.3 □ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND

Core drilling

Auger sample Disturbed sample Bulk sample Tube sample (x mm dia.) Water sample

PICO ING LEGEND

PIC Prote innisation detector

S Standard penetration test
V Shear Vene (kPa)

Water seep

Water seep

Water level





Appendix D Field Work Results (Current)

CLIENT: Cubelic Holdings Pty Ltd

PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: --**EASTING**: 278100 NORTHING: 6253030 DIP/AZIMUTH: 90%--

BORE No: BH1 PROJECT No: 71706.01 DATE: 18/6/2013

SHEET 1 OF 1

C/10-00400-004	Description	Ö		Sam	pling & l	n Situ Testing		
Depth (m)	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
8	TOPSOIL - brown silty sandy clay topsoil with a trace of ironstone gravel and grass rootlets	m	EX.//	ı	Ø		8 3	5 10 15 20
0.2	SHALE - extremely low strength brown shale			0.3			08 85	
3 3		麠	A	2000			25	
				0.6			2	
	-from 0.8m: becoming very low to low strength brown			0.8			18	
1	shale		A	1.0			80 50	-1
3 3 3							65 65 65	
1.4	Bore discontinued at 1.4m-practical refusal on medium strength shale	1		8) 8			84	
5 5							88	
8							8a 8a	
2							- S4 - 36	-2
							- 69	
8							16	
8							18	
8							8	
3							88	-3
8							03 03	
8							33	
8							25	
8							22	
4							10	
							25	4
8							63	
8							00	
8							88	
si							89	

LOGGED: SB CASING: Uncased DRILLER: SS

TYPE OF BORING: Solid flight auger (TC bit) to 1.4m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND A Auger sample
B Bulk sample
BLK Block sample
C Core drilling
D Disturbed sample Gas sample
Piston sample
Tube sample (x mm dia.)
Water sample
Water sample
Water level LEGEND
PID Photo ionisation detector (ppm)
PL(A) Point load axial test Is(50) (MPa)
PL(D) Point load diametral test Is(50) (MPa)
pp Pocket penetrometer (kPa)
Standard penetroin test
V Shear vane (kPa)



CLIENT: Cubelic Holdings Pty Ltd

PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: --EASTING: 278090 NORTHING: 6253320 DIP/AZIMUTH: 90%-- BORE No: BH2 PROJECT No: 71706.01 DATE: 18/6/2013

SHEET 1 OF 1

C/1000A00-00		Description	ic		Sam	pling &	In Situ Testing		
Depth (m)		of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20
18	r	FOPSOIL - brown silty sandy clay topsoil with some grass pootlets	a		30 3		2	3	4
0.2	S	SILTY CLAY - very stiff orange brown silty clay with a race of fine ironstone gravel	1//	A	0.3			1 2	H
22 23				0	0.5			2	
	4	from 0.7m: grey mottled orange brown			0.8			1 1	
-1 1.0			1//	Α	1.0				4
	,	SHALE - extremely low to very low strength brown shale	臣		0.000			1 34	
1.3	E	Bore discontinued at 1.3m-practical refusal on low to nedium strength shale			8) 8		ő	04	8
								84	
								84	
2									-2
								100	
								8	
3									3
								3	
								E .	
								1	
4									4
								1 32	4
								84	
8 8								84	

RIG: DT100 DRILLER: SS LOGGED: SB CASING: Uncased

TYPE OF BORING: Solid flight auger (TC bit) to 1.3m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3
 □ Cone Penetrometer AS1289.6.3.2

A Auger sample G G Gas sample PID Phot Bulk sample P Piston sample (x mm dia.) PL(A) Poin C C Core drilling W Water sample P Pool D Disturbed sample P Water seep S Stan E Environmental sample Water level V Shei

LEGEND
PID Photo ionisation detector (ppm)
PL(A) Point load axial test Is(50) (MPa)
PL(D) Point load diametral test Is(50) (MPa)
pp Pocket penetrometer (kPa)
Standard penetroin test
V Shear vane (kPa)



CLIENT: Cubelic Holdings Pty Ltd PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: --**EASTING:** 278360 NORTHING: 6253390 DIP/AZIMUTH: 90%--

BORE No: BH3 PROJECT No: 71706.01 DATE: 18/6/2013

SHEET 1 OF 1

n kozunsken son	Description	Ö		Sam	pling & l	In Situ Testing		Anne and the American and the Control
Depth (m)	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
	TOPSOIL - brown silty clayey sand topsoil with a trace of grass rootlets	a		97 8	0,		0.77	
0.2	SILTY CLAY - very stiff then hard orange brown silty clay with a trace of ironstone gravel	1//	-	0.3			1 1	4
			A	0.5			22	4
	-from 0.7m: orange brown mottled light grey/brown	1/2		0.8				
-1		1//	Α	9.0			12	1
				1.1			01. 01.	
1.4	SHALY CLAY - orange brown and grey brown shaly clay	1//					014 014	
25 25	SHALT CLAT - orange brown and grey brown shary day						894	
23 23							84	
-2								2
2.2	SHALE - extremely low strength grey shale	[//						
## ##		薑					64E	
10 10	-from 2.6m: very low to low strength brown shale	篋					90	
2.9	Born discontinued at 2.0m toward death marked	匿					800	
-3	Bore discontinued at 2.9m-target depth reached							3
18 18							0 7	
製 製							075 245	
炎 炎							11 12	
-4								
							10	
23 23							01. 01.	4
18 18							01. 01.	
25 25							894 894	
20 Ve				54 8	x 56		84.	

LOGGED: SB DRILLER: SS CASING: Uncased

TYPE OF BORING: Solid flight auger (TC bit) to 2.9m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND A Auger sample
B Bulk sample
BLK Block sample
C Core drilling
D Disturbed sample Gas sample
Piston sample
Tube sample (x mm dia.)
Water sample
Water sample
Water level



CLIENT: Cubelic Holdings Pty Ltd

PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: --**EASTING:** 278520 NORTHING: 6253550 DIP/AZIMUTH: 90%--

BORE No: BH4 PROJECT No: 71706.01

DATE: 18/6/2013 SHEET 1 OF 1

	Description	.o		Sam	pling &	In Situ Testing		
Depth (m)	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
0.1	TOPSOIL - brown silty sandy clay topsoil with a trace of grass rootlets SANDY CLAY - firm light red brown sandy clay, moist			9) 9	0,		372	
0.4	SILTY CLAY - stiff red brown silty clay with some sand and a trace of ironstone gravel		A	0.4				
				0.9			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
1.1	SANDSTONE - extremely low strength grey and brown sandstone with some medium strength ironstone bands	(1/1/	A	1.1				
1.6	Bore discontinued at 1.6m- refusal on ironstone band			25 - £	x 20		84	
-2							+	2
							## ##	
							#E	
10 10							9	
-3							1	3
							312 312 312	
							# # # # # # # # # # # # # # # # # # #	
-4								
							1 1 1	
							35	
							S)4.	
				50 8	x 5		8940	

LOGGED: SB DRILLER: SS CASING: Uncased

TYPE OF BORING: Solid flight auger (TC bit) to 1.6m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND A Auger sample
B Bulk sample
BLK Block sample
C Core drilling
D Disturbed sample Gas sample
Piston sample
Tube sample (x mm dia.)
Water sample
Water sample
Water level

LEGEND
PID Photo ionisation detector (ppm)
PL(A) Point load axial test Is(50) (MPa)
PL(D) Point load diametral test Is(50) (MPa)
pp Pocket penetrometer (kPa)
Standard penetroin test
V Shear vane (kPa)



CLIENT: Cubelic Holdings Pty Ltd

PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: --**EASTING**: 279079 NORTHING: 6254326 DIP/AZIMUTH: 90%--

BORE No: BH5 PROJECT No: 71706.01

DATE: 18/6/2013 SHEET 1 OF 1

		Description	Ö		Sam	pling & l	In Situ Testing		
Det (m	oth n)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20
8	0.1	TOPSOIL - brown silty clayey sand topsoil with some grass rootlets	11		91 3			23	
		SANDY CLAY - firm light brown sandy clay		А	0.2			23 23	4
	0.5	SILTY SANDY CLAY -very stiff then hard red brown and light brown silty sandy clay with some ironstone gravel			0.5 0.6				
37 37				A	0.8			n n	
1	1.0	SANDSTONE - very low to low strength light brown and orange brown sandstone	4/4/						-1
	1.3				85 B	. 8			
	8673	Bore discontinued at 1.3m						64 84 84	
								84	
2								34 36	-2
								18	
								8	
								8	
3								31	13
								01	
								13 13	
4									-4
								12	
								8	4
								8	
								89	

LOGGED: SB CASING: Uncased DRILLER: SS

TYPE OF BORING: Solid flight auger (TC bit) to 1.3m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2

A Auger sample
B Bulk sample
BLK Block sample
C Core drilling
D Disturbed sample

SAMPLING & IN SITU TESTING LEGEND Gas sample
Piston sample
Tube sample (x mm dia.)
Water sample
Water sample
Water level

LEGEND
PID Photo ionisation detector (ppm)
PL(A) Point load axial test Is(50) (MPa)
PL(D) Point load diametral test Is(50) (MPa)
pp Pocket penetrometer (kPa)
Standard penetroin test
V Shear vane (kPa)



CLIENT: Cubelic Holdings Pty Ltd

PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: -EASTING: 279260
NORTHING: 6254480
DIP/AZIMUTH: 90%--

BORE No: BH6 PROJECT No: 71706.01 DATE: 18/6/2013

SHEET 1 OF 1

	Description	Ö		Sam	pling &	In Situ Testing		
군 Depth (m)	of Strata	Graphic Log	Type	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm)
	SANDY CLAY - very stiff light brown and orange brown sandy clay with a trace of grass rootlets		A	0.3	0)		07 07 27 27	
0.7	SANDSTONE - extremely weathered orange and red brown sandstone		Α	0.8				1
	-from 1.1m: low strength grey/orange brown sandstone						01 01	
- 1.4	Bore discontinued at 1.4m-target depth reached							- 7

RIG: DT100 DRILLER: SS LOGGED: SB CASING: Uncased

TYPE OF BORING: Solid flight auger (TC bit) to 1.4m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3
 □ Cone Penetrometer AS1289.6.3.2

A Auger sample G G Gas sample PID Phot Bulk sample P Piston sample (x mm dia.) PL(A) Poin C C Core drilling W Water sample P Pool D Disturbed sample P Water seep S Stan E Environmental sample Water level V Shei

TESTING LEGEND
PID Photo ionisation detector (ppm)
PID Photo ionisation detector (ppm)
PL(A) Point load axial test Is(50) (MPa)
x mm dia.)
P(L(D) Point load diametral test Is(50) (MPa)
pp Pooket penetrometer (kPa)
Standard penetrion test
V Shear vane (kPa)



CLIENT: Cubelic Holdings Pty Ltd

PROJECT: Fernhill Estate

LOCATION: Fairlight Road, Mulgoa

SURFACE LEVEL: --**EASTING**: 279530 NORTHING: 6254350 DIP/AZIMUTH: 90%--

BORE No: BH7 PROJECT No: 71706.01

DATE: 18/6/2013 SHEET 1 OF 1

		Description	.o		Sam	pling & l	In Situ Testing		
보	Depth (m)	of Strata	Graphic Log	Туре	Depth	Sample	Results & Comments	Water	Dynamic Penetrometer Test (blows per 150mm) 5 10 15 20
	8	SILTY CLAY - firm then stiff to very stiff light brown silty clay with a trace of grass rootlets	1//		30 3				LI III
) ()		1//	A	0.2			: : ::	Ц
-	8			^	0.5			3	H
-		-from 0.6m: orange brown mottled gey brown with a trace of ironstone gravel	1//		0.7			1	i h
-	8	of total and grand	1	Α	40.77			1 25	
-	1		1		1.0			1	1 1
-	1.2	SHALY CLAY - grey and orange brown shaly clay (extremely low strength shale) /	17/					1	
-	e E	SHALE - very low to low strength grey brown shale	題					01. 81.	
	1.7		題		06 A	x 200		84	
	(C)	Bore discontinued at 1.7m- practical refusal						84.	
-	2							1	2
	85 85							(4) (4)	
ļ								100	
ŀ	8								
-								9	
-									
-	3								3
	§ 8							32	
-	8							3	
	8							12	
ŀ	8 8								
-	4								4
-								1 1	4
-	8								
F	8							1	
-	8							34 84	
-	13 15							894	
	w Ye				30 S	s (20			

LOGGED: SB CASING: Uncased DRILLER: SS

TYPE OF BORING: Solid flight auger (TC bit) to 1.7m WATER OBSERVATIONS: No free groudwater observed

REMARKS:

□ Sand Penetrometer AS1289.6.3.3 ☑ Cone Penetrometer AS1289.6.3.2

SAMPLING & IN SITU TESTING LEGEND A Auger sample
B Bulk sample
BLK Block sample
C Core drilling
D Disturbed sample G Gas sample
P Piston sample
U, Tube sample (x mm dia.)
W Water sample
D Water seep
Water level

PID Photo ionisation detector (ppm)
PL(A) Point load axial test Is(50) (MPa)
PL(D) Point load dametral test Is(50) (MPa)
pp Pocket penetrometer (kPa)
Standard penetrion test
V Shear vane (kPa)



Appendix E

Laboratory Test Results (2010)

Douglas Partners Pty Ltd ABN 75 053 980 117

96 Hermitage Rd West Ryde 2114 NSW AUSTRALIA

PQ Box 472 West Ryde NSW 1685

Phone 02 9809 0666 02 9806 4095 sydney@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION

Client: OWSTON NOMINEES NO.2 PTY LTD

Principal:

LAND CAPABILITY ASSESSMENT

Location:

MULGOA (WESTERN PRECINCT)

Road No:

Sample / Pit No: TP 9

Chainage:

Section / Lot No: -

Project No.:

71706

S10-095 J

Report No. : Report Date :

27-May-10

Date Sampled: 19-23/04/10

Date of Test:

13-May-10

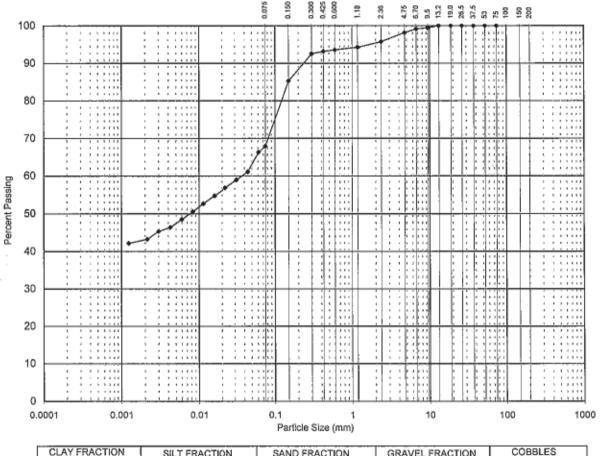
Depth / Layer: 1.0m

Test Request No -

Page:

1 of

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	100%
53.0	100%
37.5	100%
26.5	100%
19.0	100%
13.2	100%
9.5	99%
6.7	99%
4.75	98%
2.36	96%
1.18	94%
0.600	94%
0.425	93%
0.300	93%
0.150	85%
0.075	68%

CLAY FRACTION	SILT FRACTION			SAND	FRACT	ON	GRAV	EL FRAC	COBBLES	
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine Medium Coarse			
0.0	0.0	06 0/	D2 0.	06	2 0		.0		10 6	0

Description:

SANDY SILTY CLAY - Orange brown sandy silty clay with a trace of gravel

Test Method(s):

AS 1289.3.6.1, AS 1289.3.6.3

Sampling Method(s): Sampled by Engineering Department

Remarks:



NATA Accredited Laboratory Number: 828 This Document is issued in accordance with NAFA's accreditation requirements.

Accredited for compliance with ISO/IEC 17025

Approved Signatory:

Meimon

Norman Weimann Laboratory Manager



Douglas Partners Pty Ltd ABN 75 053 980 117 96 Hermitage Rd

West Ryde 2114 NSW AUSTRALIA

PO Box 472 West Ryde NSW 1685 Phone 02 9809 0666 02 9806 4095

sydney@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION

Client:

OWSTON NOMINEES NO.2 PTY LTD

Project:

LAND CAPABILITY ASSESSMENT

Location:

MULGOA (WESTERN PRECINCT)

Road No:

Chainage:

Sample / Pit No: TP 24

Section / Lot No: -

Project No.:

71706

Report No.:

S10-095 K

Report Date :

27-May-10

Date of Test:

Date Sampled: 19-23/04/10

13-May-10

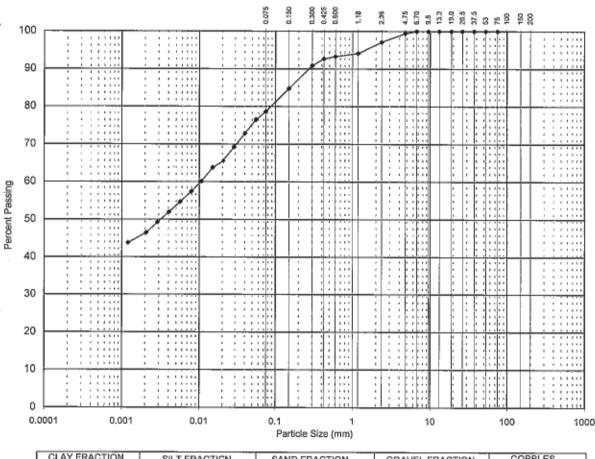
Depth / Layer: 0.5m

Test Request No -

Page:

1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	100%
53.0	100%
37.5	100%
26.5	100%
19.0	100%
13.2	100%
9.5	100%
6.7	100%
4.75	99%
2.36	97%
1.18	94%
0.600	93%
0.425	93%
0.300	91%
0.150	85%
0.075	79%

	CLAY FRACTION	SIL	T FRACTI	CN	SANE	FRACT	ION	GRA	EL FRAC	COBBLES	
l		Fine Medium Coarse				Medium	fedium Coarse Fine Medium Coarse			Coarse	
	0.0	0.0	006 0	00 D.	06 6	.2 0		2.0	.0 2	: :0 :6	0

Description:

SILTY CLAY - Mottled orange brown and grey slightly sandy silty clay with a trace of gravel

Test Method(s):

AS 1289.3.6.1, AS 1289.3.6.3

Sampling Method(s): Sampled by Engineering Department

Remarks:



Approved Signatory:

Tested: LW NW

Norman Weimann Laboratory Manager

NATA Accredited Laboratory Number: 828 This Document is issued in accordance with NATA's socreditation requirements. Accredited for compliance with ISO/IEC 17025

Douglas Partners Pty Ltd ABN 75 053 980 117

96 Hermitage Rd West Ryde 2114 NSW AUSTRALIA

PO Box 472 West Ryde NSW 1685

02 9809 0666 Phone Fax: 02 9806 4095 sydney@douglaspartners.com.au

RESULTS OF PARTICLE SIZE DISTRIBUTION

Client:

OWSTON NOMINEES NO.2 PTY LTD

Principal:

LAND CAPABILITY ASSESSMENT

Location:

MULGOA (WESTERN PRECINCT)

Road No:

Chainage:

Sample / Pit No: TP 25

Section / Lot No: -

Project No.: Report No. :

71706

S10-095 H

Report Date :

27-May-10 Date Sampled: 19-23/04/10

Date of Test:

13-May-10

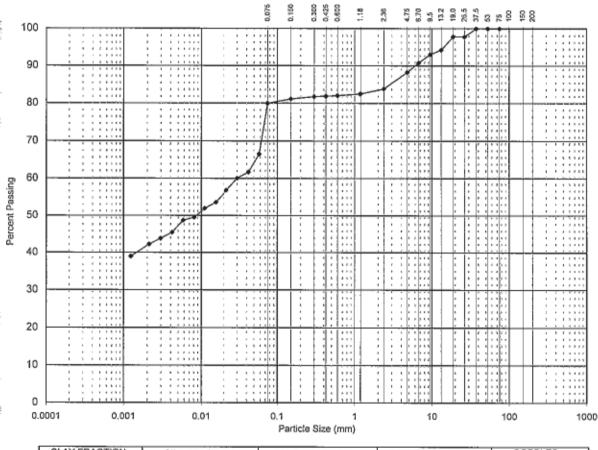
Depth / Layer: 0.5m

Test Request No -

Page:

1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



75.0 53.0	100%
53.0	
	100%
37.5	100%
26.5	98%
19.0	98%
13.2	94%
9.5	93%
6.7	91%
4.75	88%
2.36	84%
1.18	83%
0.600	82%
0.425	82%
0.300	82%
0.150	81%
0.075	80%

CLAY FRACTION	SIL	T FRACTI	ON	SANE	FRACTI	ON	GRAV	EL FRAC	COBBLES	
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine Medium Coarse			
	0.0	005 0/	o.	06			.0	.0 2		0

Description:

SILTY CLAY- Mottled red brown and grey silty clay with a trace of roolets

Test Method(s):

AS 1289.3.6.1, AS 1289.3.6.3

Sampling Method(s): Sampled by Engineering Department

Remarks:

P. 2006 Douglas Partnins Py Ltd

X004A RTHELLUZ 2016



Approved Signatory:

Tested: LW Checked: NW Meimann Norman Weimann Laboratory Manager

Douglas Partners Pty Ltd ABN 75 053 980 117

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DETERMINATION OF EMERSON CLASS NUMBER OF SOIL

Client:

OWSTON NOMINEES NO.2 PTY LTD

Project No:

71706

Project:

LAND CAPABILITY ASSESSMENT

Report No: Report Date: S10-095 N1

27/05/10

Location:

MULGOA (WESTERN PRECINCT)

Date of Test:

24/05/10

Page:

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SAMPLE NO	DEPTH (m)	DESCRIPTION	WATER TYPE	WATER TEMP	CLASS NO.
TP 5	0.5	SILTY CLAY – Mottled red brown and grey silty clay	Distilled	22	5
TP 9	1.0	SANDY SILTY CLAY - Orange brown sandy silty clay with a trace of gravel	Distilled	22	6
TP 11	0.5	SITLY CLAY – Orange brown silty clay with a trace of ironstone gravel	Distilled	22	6
TP 14	0.5	SILTY CLAY - Orange brown silty clay with some ironstone gravel and cobbles	Distilled	22	5
TP 24	1.0	SILTY CLAY – Mottled orange brown and grey silty clay with a trace sand	Distilled	22	6
TP 28	1.0	SANDY CLAY - Orange brown sandy clay with some ironstone gravel and cobbles	Distilled	22	6

Test Method(s):

AS 1289 3.8.1

Sampling Method(s):

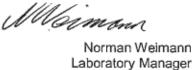
Sampled by Engineering Department

Remarks:



Approved Signatory:

Tested: LW Checked: NW





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CERTIFICATE OF ANALYSIS 40947

Client:

Douglas Partners 96 Hermitage Rd West Ryde NSW 2114

Attention: Adam Podnar

Sample log in details:

Your Reference: 71706, Mulgoa

No. of samples: 63 Soils
Date samples received: 13/05/10

Date completed instructions received: 13/05/10

Analysis Details:

Please refer to the following pages for results, methodology summary and quality control data.

Samples were analysed as received from the client. Results relate specifically to the samples as received.

Results are reported on a cry weight basis for solids and on an as received basis for other matrices.

Please refer to the last page of this report for any comments relating to the results.

Report Details:

Date results requested by: 20/05/10

Date of Preliminary Report: Not Issued

Issue Date: 21/05/10

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Tests not covered by NATA are denoted with *.

Results Approved By:

Rhian Morgan Metals Supervisor

konstaffferst Laboratory Manager

Envirolab Reference:

40947

Revision No:

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Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-1	40947-2	40947-3	40947-4	40947-5
Your Reference		TP3/0.25	TP3/0.5	TP3/1	TP3/1.5	TP3/2
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	7.6	6.1	6.8	4.8	4.6
Electrical Conductivity 1:5 soil:water	μS/cm	44	37	39	58	38
Resistivity in soil*	ohm m	230	270	250	170	260
Chloride, Cl 1:5 soil:water	rng/kg	17	20	18	31	16
Sulphate, SO4 1:5 soil:water	rng/kg	3.8	2.5	3.3	<2.0	3.7

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-6	40947-7	40947-8	40947-9	40947-10
Your Reference		TP3/2.5	TP6/0.25	TP6/0.5	TP9/0.25	TP9/0.5
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	7.6	5.9	5.9	6.0	6.0
Electrical Conductivity 1:5 soil:water	μS/cm	41	12	16	7.0	14
Resistivity in soil*	ohm m	240	770	630	1,100	710
Chloride, Cl 1:5 soil:water	mg/kg	15	[NA]	[NA]	[NA]	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	3.1	[NA]	[NA]	[NA]	[NA]

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-11	40947-13	40947-14	40947-15	40947-16
Your Reference		TP9/1.0	TP12/0.25	TP12/0.5	TP14/0.25	TP14/0.5
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.4	4.9	4.9	6.2	6.0
Electrical Conductivity 1:5 soil:water	μS/cm	13	56	45	10	5.0
Resistivity in soil*	ohm m	770	180	220	1,100	1,900
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	<20	6.0	[NA]	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	<20	6.9	[NA]	[NA]

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Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-17	40947-18	40947-19	40947-20	40947-21
Your Reference		TP14/1.0	TP14/1.5	TP16/0.25	TP16/0.5	TP18/0.25
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.9	5.4	5.0	7.8	5.9
Electrical Conductivity 1:5 soil:water	μS/cm	14	15	38	43	13
Resistivity in soil*	ohm m	710	670	260	230	770
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	[NA]	2.2	2.1	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	[NA]	7.2	6.6	[NA]

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-22	40947-23	40947-24	40947-25	40947-26
Your Reference		TP18/0.5	TP20/0.25	TP20/0.5	TP22/0.25	TP22/0.5
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	8.4	5.1	5.1	5.8	6.3
Electrical Conductivity 1:5 soil:water	μS/cm	29	24	31	11	8.0
Resistivity in soil*	ohm m	340	420	320	910	1,300
Chloride, CI 1:5 soil:water	mg/kg	5.6	2.8	6.2	[NA]	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	<2.0	<2.0	2.0	[NA]	[NA]

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-27	40947-28	40947-29	40947-30	40947-31
Your Reference		TP26/0.25	TP26/0.5	TP28/0.25	TP28/0.5	TP24/0.25
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.8	5.1	6.0	6.2	6.1
Electrical Conductivity 1:5 soil:water	μS/cm	13	45	9.0	13	9.0
Resistivity in soil*	ohm m	760	220	1,200	790	1,100
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	12	[NA]	[NA]	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	<2.0	[NA]	[NA]	[NA]

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Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-32	40947-33	40947-34	40947-35	40947-36
Your Reference		TP24/0.5	TP24/1.0	TP106/0.25	TP106/0.5	TP106/1.0
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	6.1	5.4	6.3	6.1	5.2
Electrical Conductivity 1:5 soil:water	µS/cm	13	12	20	110	100
Resistivity in soil*	ohm m	770	830	500	91	100
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	6.1	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	70	[NA]

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-37	40947-38	40947-39	40947-40	40947-41
Your Reference		TP106/1.5	TP106/2.0	TP111/0.25	TP111/0.5	TP111/1.0
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.5	5.4	6.7	5.0	5.5
Electrical Conductivity 1:5 soil:water	μS/cm	88	150	82	97	88
Resistivity in soil*	ohm m	110	67	120	100	110
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	[NA]	<20	46	50
Sulphate, SQ4 1:5 soil:water	mg/kg	[NA]	[NA]	<20	<20	<20

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-42	40947-43	40947-44	40947-45	40947-46
Your Reference		TP111/1.5	TP111/1.8	TP114/0.25	TP114/0.5	TP114/1.0
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	6.5	6.4	6.2	4.6	5.3
Electrical Conductivity 1:5 soil:water	μS/cm	100	140	21	100	71
Resistivity in soil*	ohm m	99	71	480	100	140
Chloride, Cl 1:5 soil:water	mg/kg	20	77	[NA]	8.5	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	<2.0	<20	[NA]	66	[NA]

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Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-47	40947-48	40947-49	40947-50	40947-51
Your Reference		TP114/1.5	TP114/2.0	TP114/2.5	TP120/0.25	TP120/0.5
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.3	5.1	5.4	5.7	5.7
Electrical Conductivity 1:5 soll:water	μS/cm	110	100	200	50	53
Resistivity in soil*	ohm m	91	100	50	200	190
Chloride, CI 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	980	<20
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	840	<20

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-52	40947-53	40947-54	40947-55	40947-56
Your Reference		TP120/1.0	TP120/1.5	TP120/2.0	TP127/0.25	TP127/0.5
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared		14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.5	5.2	5.1	5.4	5.2
Electrical Conductivity 1:5 soil:water	μS/cm	67	200	230	61	45
Resistivity in soil*	ohm m	150	51	43	160	220
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	<20	<20
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	<20	21

Miscellaneous Inorg - soil						
Our Reference:	UNITS	40947-57	40947-58	40947-59	40947-60	40947-61
Your Reference		TP127/1.0	TP127/1.5	TP127/2.0	TP131/0.25	TP131/1.0
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	14/5/2010	14/5/2010	14/5/2010	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10	17/05/10	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	6.1	5.8	5.8	5.5	6.3
Electrical Conductivity 1:5 soil:water	μS/cm	23	32	23	54	21
Resistivity in soil*	ohm m	430	310	430	190	480
Chloride, Cl 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	<20	[NA]
Sulphate, SO4 1:5 soil:water	mg/kg	[NA]	[NA]	[NA]	<20	[NA]

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Miscellaneous Inorg - soil		-	
Our Reference:	UNITS	40947-62	40947-63
Your Reference		TP131/1.5	TP131/2.0
Date Sampled		19/04/2010	19/04/2010
Type of sample		Soil	Soil
Date prepared	-	14/5/2010	14/5/2010
Date analysed	-	17/05/10	17/05/10
pH 1:5 soil:water	pH Units	5.8	6.4
Electrical Conductivity 1:5 soil:water	μS/cm	24	12
Resistivity in soil*	ohm m	420	830



ESP/CEC Our Reference: Your Reference Date Sampled	UNITS	40947-2 TP3/0.5 19/04/2010	40947-8 TP6/0.5 19/04/2010	40947-14 TP12/0.5 19/04/2010	40947-19 TP16/0.25 19/04/2010	40947-20 TP16/0.5 19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Exchangeable Ca*	meq/100g	0.050	0.17	0.090	0.24	0.13
Exchangeable K*	meq/100g	0.13	0.16	0.19	0.060	0.080
Exchangeable Mg*	meq/100g	9.5	4.1	4.0	0.81	3.6
Exchangeable Na*	meq/100g	0.77	0.65	0.50	0.25	0.39
Cation Exchange Capacity*	meq/100g	11	5.1	4.8	1.4	4.2
ESP*	%	7.4	12.8	10.5	18.1	9.4

ESP/CEC						
Our Reference:	UNITS	40947-26	40947-35	40947-40	40947-51	40947-57
Your Reference		TP22/0.5	TP106/0.5	TP111/0.5	TP120/0.5	TP127/1.0
Date Sampled		19/04/2010	19/04/2010	19/04/2010	19/04/2010	19/04/2010
Type of sample		Soil	Soil	Soil	Soil	Soil
Exchangeable Ca*	meq/100g	0.16	0.28	1.1	2.9	2.0
Exchangeable K*	meq/100g	0.14	0.29	0.33	0.12	0.16
Exchangeable Mg*	meq/100g	4.9	6.4	8.6	3.4	6.1
Exchangeable Na*	meq/100g	0.37	1.2	1.4	0.47	0.52
Cation Exchange Capacity*	meq/100g	5.5	8.2	11	6.9	8.7
ESP*	%	6.6	14.9	11.9	6.9	5.9

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Method ID	Methodology Summary
LAB.1	pH - Measured using pH meter and electrode in accordance with APHA 20th ED, 4500-H+.
LAB.2	Conductivity and Salinity - measured using a conductivity cell and dedicated meter, in accordance with APHA2510 20th ED and Rayment & Higginson.
LAB.81	Anions - a range of Anions are determined by Ion Chromatography, in accordance with APHA 21st ED, 4110-B.
Metals.23	Determination of exchangeable cations and cation exchange capacity in soil.



QUALITY CONTROL	UNITS	PQL	METHOD	Blank	Duplicate Sm#	Duplicate results		Spike Sm#	Spike % Recovery		
Miscellaneous Inorg - soil	_					Base II Duplicate II %RPD	+				
Date prepared	-			14/05/2 010	40947-1	14/5/2010] 14/5/201	14/5/2010] 14/5/2010		14/05/2010		
Date analysed	-			19/05/2 010	40947-1	17/05/10 17/05/10	17/05/10] 17/05/10		17/05/10] 17/05/10		14/05/2010
pH 1:5 soil:water	pH Units		LAB.1	[NT]	40947-1	7.6 7.6 RPD: 0		LCS-1	99%		
Electrical Conductivity 1:5 soil:water	μS/cm	1	LAB.2	<1.0	40947-1	44 44 RPD: 0		LCS-1	100%		
Resistivity in soil*	ohm m	1	LAB.2	<1.0	40947-1	230 230 RPD: 0		LCS-1	100%		
Chloride, Cl 1:5 soil:water	mg/kg	2	LAB.81	<2.0	40947-1	17 17 RPD: 0		LCS-1	99%		
Sulphate, SO4 1:5 soil:water	mg/kg	2	LAB.81	<2.0	40947-1	3.8 3.9 RPD: 3		LCS-1	103%		
QUALITY CONTROL	UNITS	PQL	METHOD	Blank	Duplicate Sm#	Dunlinete regulte		Dalles Cout	0-1 0/		
QUALITY CONTINUE	014113	r vec	WILTHOU	Didilk	Duplicate Sitte	Duplicate results		Spike Sm#	Spike % Recovery		
ESP/CEC						Base II Duplicate II %RPD	Base II Duplicate II %RPD				
Exchangeable Ca*	meq/100 g	0.01	Metals.23	<0.01	40947-2	0.050 0.060 RPD:	0.050 0.060 RPD: 18		108%		
Exchangeable K*	meq/100	0.01	Metals.23	<0.01	40947-2	0.13 0.14 RPD: 7	0.13 0.14 RPD: 7		105%		
Exchangeable Mg*	meq/100	0.01	Metals.23	<0.01	40947-2	9.5 9.9 RPD: 4		LCS-1	104%		
Exchangeable Na*	meq/100	0.01	Metals.23	<0.01	40947-2	0.77 0.82 RPD: 6		LCS-1	108%		
Cation Exchange Capacity*	meq/100	1	Metals.23	<1.0	40947-2	11 11 RPD: 0		[NR]	[NR]		
ESP*	%	1	Metals.23	<1.0	40947-2	7.4 7.5 RPD; 1		[NR]	[NR]		
QUALITY CONTROL	UNITS	S	Dup. Sm#		Duplicate	Spike Sm#	Spi	ke % Recovery	1		
Miscellaneous Inorg - soil				Base +	Duplicate + %RPD						
Date prepared	-		40947-11	14/5/2	010 14/5/2010	LCS-2		14/05/2010	7		
Date analysed	-		40947-11	17/05	5/10 17/05/10	LCS-2		19/05/2010			
pH 1:5 soil:water	pH Uni	its	40947-11	5.4	5.7 RPD: 5	LCS-2		99%			
Electrical Conductivity 1:5 soil:water	μS/cn	n	40947-11	13	13 RPD; 0	LCS-2		100%			
Resistivity in soil*	ohm r	n	40947-11	770	770 RPD: 0	LCS-2		100%			
Chloride, Cl 1:5 soil:water	r mg/kg	9	[NT]		[NT]	LCS-2		98%			
Sulphate, SO4 1:5 soil:water	mg/k	g	[TM]		[NT]	LCS-2		98%			

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		Client Reference	e: 71706, Mulgoa		
QUALITY CONTROL	UNITS	Dup. Sm#	Duplicate	Spike Sm#	Spike % Recovery
Miscellaneous Inorg - soil			Base + Duplicate + %RPD		
Date prepared	-	40947-22	14/5/2010 14/5/2010	40947-2	14/05/2010
Date analysed	-	40947-22	17/05/10 17/05/10	40947-2	19/05/2010
pH 1:5 soil:water	pH Units	40947-22	8.4 8.4 RPD: 0	[NR]	[NR]
Resistivity in soil*	ohm m	40947-22	340 350 RPD: 3	[NR]	[NR]
Chloride, Cl 1:5 soil:water	mg/kg	40947-22	5.6 5.6 RPD: 0	40947-2	110%
Sulphate, SO4 1:5 soil:water	mg/kg	40947-22	<2.0 <2.0	40947-2	102%
QUALITY CONTROL	UNITS	Dup. Sm#	Duplicate	Spike Sm#	Spike % Recovery
Miscellaneous Inorg - soil			Base + Duplicate + %RPD		
Date prepared	-	40947-33	14/5/2010 14/5/2010	40947-56	14/5/2010
Date analysed	-	40947-33	17/05/10 17/05/10	40947-56	19/5/2010
pH 1:5 soil:water	pH Units	40947-33	5.4 5.6 RPD; 4	[NR]	[NR]
Electrical Conductivity 1:5 soil:water	μS/cm	40947-33	12 12 RPD; 0	[NR]	[NR]
Resistivity in soil*	ohm m	40947-33	830 830 RPD; 0	[NR]	[NR]
Chloride, Cl 1:5 soil:water	mg/kg	[NT]	[NT]	40947-56	96%
Sulphate, SO4 1:5 soil:water	mg/kg	[NT]	[NT]	40947-56	80%
QUALITY CONTROL	UNITS	Dup. Sm#	Duplicate		
Miscellaneous Inorg - soil			Base + Duplicate + %RPD		
Date prepared	-	40947-44	14/5/2010 14/5/2010		
Date analysed	-	40947-44	17/05/10 17/05/10		
pH 1:5 soil:water	pH Units	40947-44	6.2 6.3 RPD: 2		
Electrical Conductivity 1:5 soil:water	μS/cm	40947-44	21 [19] RPD: 10		
Resistivity in soil*	ohm m	40947-44	480 530 RPD: 10		
Chloride, Cl 1:5 soil:water	mg/kg	[NT]	[NT]		
Sulphate, SO4 1:5 soil:water	mg/kg	[NT]	[NT]		
QUALITY CONTROL	UNITS	Dup. Sm#	Duplicate		
Miscellaneous Inorg - soil			Base + Duplicate + %RPD		
Date prepared	-	40947-55	14/5/2010 14/5/2010	1	
Date analysed	-	40947-55	17/05/10 17/05/10		
pH 1:5 soil:water	pH Units	40947-55	5.4 5.5 RPD: 2		
Electrical Conductivity 1:5 soil:water	μ\$/cm	40947-55	61 59 RPD: 3		
Resistivity in soil*	ohm m	40947-55	160 160 RPD: 0		
Chloride, Cl 1:5 soil:water	mg/kg	40947-55	<20 <20		
Sulphate, SO4 1:5 soil:water	mg/kg	40947-55	<20 <20		

Envirolab Reference: 40947 Revision No:



Report Comments:

Sulphate\Chloride: PQL raised by a factor of X10 sor samples 13,39,40,41,43,51,55,56,60 due to sample matrix.

Asbestos was analysed by Approved Identifier: Not applicable for this job

Asbestos was authorised by Approved Signatory: Not applicable for this job

INS: Insufficient sample for this test NT: Not tested PQL: Practical Quantitation Limit <: Less than >: Greater than

RPD: Relative Percent Difference NA: Test not required LCS: Laborator

quired LCS: Laboratory Control Sample NR: Not requested

Quality Control Definitions

Blank: This is the component of the analytical signal which is not derived from the sample but from reagents, glassware etc, can be determined by processing solvents and reagents in exactly the same manner as for samples.

Duplicate: This is the complete duplicate analysis of a sample from the process batch. If possible, the sample selected should be one where the analyte concentration is easily measurable.

Matrix Spike: A portion of the sample is spiked with a known concentration of target analyte. The purpose of the matrix spike is to monitor the performance of the analytical method used and to determine whether matrix interferences exist.

LCS (Laboratory Control Sample): This comprises either a standard reference material or a control matrix (such as a blank sand or water) fortified with analytes representative of the analyte class. It is simply a check sample.

Surrogate Spike: Surrogates are known additions to each sample, blank, matrix spike and LCS in a batch, of compounds which are similar to the analyte of interest, however are not expected to be found in real samples.

Laboratory Acceptance Criteria:

Duplicate sample and matrix spike recoveries may not be reported on smaller jobs, however, were analysed at a frequency to meet or exceed NEPM requirements. All samples are tested in batches of 20. The duplicate sample RPD and matrix spike recoveries for the sample batch were within laboratory acceptance criteria.

Duplicates: <5xPQL - any RPD is acceptable; >5xPQL - 0-50% RPD is acceptable.

Matrix Spikes and LCS: Generally 70-130% for inorganics/metals; 60-140% for organics and 10-140% for

SVOC and speciated phenols is acceptable. Surrogates: 60-140% is acceptable for general organics and 10-140% for





Envirolab Services Pty Ltd

ABN 37 112 535 645

12 Ashley St Chatswood NSW 2067 ph 02 9958 5801 fax 02 9958 5803 mob 0400 88 5292 email: tnotaras@envirolabservices.com.au

Douglas Partners 96 Hermitage Rd West Ryde 2114

Att: Adam Podnar

Re: 71706, Mulgoa

Soil Texture was determined based on the Australian Governments Department of the Environment & Heritage, Australian Greenhouse Office Guide to Field Measurements.

ECe (Extract Electrical Conductivity) is determined by analysing a 1:5 soil water extract for conductivity then multiplying this result by the soil texture conversion factor based on 'Site Investigations for Urban Salinity', DIPNR 2002.

				ECe	
ID	Envirolab ID	EC dS/m	Texture	d\$/m	Class
TP3/0.25	40947-1	0.044	Light Clay	0.37	Non Saline
TP3/0.5	40947-2	0.037	Light Medium Clay	0.30	Non Saline
TP3/1	40947-3	0.039	Light Medium Clay	0.31	Non Saline
TP3/1.5	40947-4	0.058	Light Medium Clay	0.46	Non Saline
TP3/2	40947-5	0.038	Light Medium Clay	0.30	Non Saline
TP3/2.5	40947-6	0.041	Medium Clay	0.29	Non Saline
TP6/0.25	40947-7	0.012	Loam	0.12	Non Saline
TP6/0.5	40947-8	0.016	Light Medium Clay	0.13	Non Saline
TP9/0.25	40947-9	0.009	Light Clay	0.08	Non Saline
TP9/0.5	40947-10	0.014	Light Clay	0.12	Non Saline
TP9/1.0	40947-11	0.013	Medium Clay	0.09	Non Saline
TP12/0.25	40947-13	0.056	Clay Loam	0.50	Non Saline
TP12/0.5	40947-14	0.045	Light Medium Clay	0.36	Non Saline
TP14/0.25	40947-15	0.010	Clay Loam	0.09	Non Saline
TP14/0.5	40947-16	0.005	Light Medium Clay	0.04	Non Saline
TP14/1.0	40947-17	0.014	Medium Clay	0.10	Non Saline
TP14/1.5	40947-18	0.015	Medium Clay	0.11	Non Saline
TP16/0.25	40947-19	0.038	Light Medium Clay	0.30	Non Saline
TP16/0.5	40947-20	0.043	Light Medium Clay	0.34	Non Saline
TP18/0.25	40947-21	0.013	Light Medium Clay	0.10	Non Saline
TP18/0.5	40947-22	0.029	Light Medium Clay	0.23	Non Saline
TP20/0.25	40947-23	0.024	Light Medium Clay	0.19	Non Saline
TP20/0.5	40947-24	0.031	Medium Clay	0.22	Non Saline
TP22/0.25	40947-25	0.011	Medium Clay	0.08	Non Saline
TP22/0.5	40947-26	0.008	Medium Clay	0.06	Non Saline
TP26/0.25	40947-27	0.013	Light Medium Clay	0.10	Non Saline
TP26/0.5	40947-28	0.045	Light Medium Clay	.36	Non Saline

TP28/0.25	P28/0.25 40947-29 0.009 Clay Loam				Non Saline
TP28/0.5	40947-30	0.013	Medium Clay	0.08	Non Saline
ID	Envirolab ID	EC dS/m	Texture	ECe dS/m	Class
TP24/0.25	40947-31	0.009	Light Medium Clay	0.07	Non Saline
TP24/0.5	40947-32	0.013	Light Medium Clay	.10	Non Saline
TP24/1.0	40947-33	0.012	Medium Clay	0.08	Non Saline
TP106/0.25	40947-34	0.020	Heavy Clay	0.12	Non Saline
TP106/0.5	40947-35	0.110	Light Medium Clay	0.88	Non Saline
TP106/1.0	40947-36	.100	Medium Clay	0.80	Non Saline
TP106/1.5	40947-37	.088	Light Medium Clay	0.70	Non Saline
TP106/2.0	40947-38	.150	Medium Clay	1.05	Non Saline
TP111/0.25	40947-39	0.082	Medium Clay	0.57	Non Saline
TP111/0.5	40947-40	0.097	Medium Clay	0.68	Non Saline
TP111/1.0	40947-41	0.088	Heavy Clay	0.53	Non Saline
TP111/1.5	40947-42	0.100	Heavy Clay	0.60	Non Saline
TP111/1.8	40947-43	0.140	Heavy Clay	0.84	Non Saline
TP114/02.5	40947-44	0.021	Loam	0.21	Non Saline
TP114/0.5	40947-45	.100	Medium Clay	0.70	Non Saline
TP114/1.0	40947-46	0.07	Heavy Clay	0.42	Non Saline
TP114/1.5	40947-47	0.110	Light Medium Clay	0.88	Non Saline
TP114/2.0	40947-48	0.10	Medium Clay	.70	Non Saline
TP114/2.5	40947-49	0.20	Medium Clay	1.6	Non Saline
TP120/0.25	40947-50	0.05	Loam	0.50	Non Saline
TP120/0.5	40947-51	0.053	Clay Loam	0.48	Non Saline
TP120/1.0	40947-52	0.067	Medium Clay	0.47	Non Saline
TP120/1.5	40947-53	0.20	Heavy Clay	1.2	Non Saline
TP120/2.0	40947-54	0.23	Heavy Clay	1.4	Non Saline
TP127/0.25	40947-55	0.061	Loam	0.61	Non Saline
TP127/0.5	40947-56	0.045	Loam	0.45	Non Saline
TP127/1.0	40947-57	0.023	Light Medium Clay	0.18	Non Saline
TP127/1.5	40947-58	0.032	Light Medium Clay	0.26	Non Saline
TP127/2.0	40947-59	0.023	Medium Clay	0.16	Non Saline
TP131/0.25	40947-60	0.054	Loam	0.54	Non Saline
TP131/1.0	40947-61	0.021	Medium Clay	0.15	Non Saline
TP131/1.5	40947-62	0.024	Medium Clay	0.17	Non Saline
TP131/2.0	40947-63	0.012	Sandy Loams	0.17	Non Saline

DIPNR gives the following definitions:

^{&#}x27;Non-Saline' as 'Salinity effects mostly negligible'.

^{&#}x27;Slightly Saline' as 'yields of very sensitive crops may be affected'. 'Moderately Saline' as 'yields of many crops affected'.

^{&#}x27;Very Saline' as 'Only tolerant crops yield satisfactorily'.

^{&#}x27;Highly Saline' as 'Only a few very tolerant crops yield satisfactorily'.

Appendix F Laboratory Test Results (Current)



Douglas Partners Ptv Ltd ABN 75 053 980 117 ABN 75 053 980 117 www.douglaspartners.com.au 96 Hermitage Road West Ryde NSW 2114 PO Box 472 West Ryde NSW 1685 Phone (02) 980 9666 Fax (02) 9809 4095

Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

2

Project:

FERNHILL ESTATE

Report No.: Report Date:

25-Jun-13

Location:

Date Sampled: Date of Test:

18-Jun-13

MULOGA

20-Jun-13

Road No:

Sample / Pit No: BH 1

Depth / Layer:

Page:

0.3-0.6m

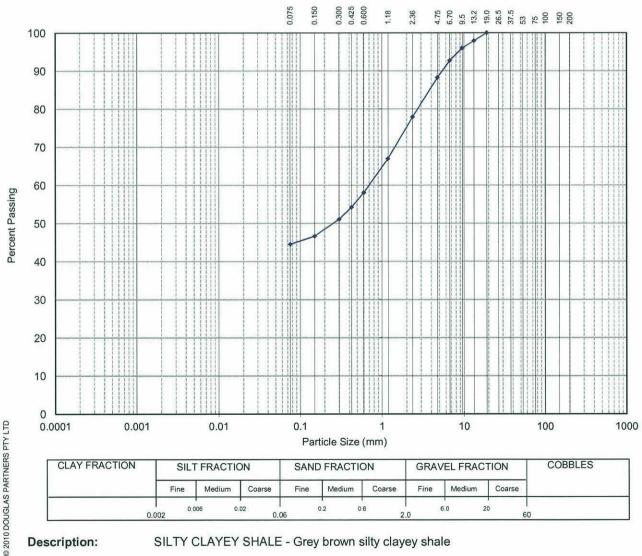
Chainage:

Section / Lot No: -

Test Request No:

1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	~
53.0	~
37.5	~
26.5	~
19.0	100%
13.2	98%
9.5	96%
6.7	93%
4.75	88%
2.36	78%
1.18	67%
0.600	58%
0.425	54%
0.300	51%
0.150	47%
0.075	45%

CLAY FRACTION	SIL	FRACTIO	NC	SAND	FRACTIO	NC	GRAV	/EL FRAC	TION	COBBLES
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse	
0.	002	006 0.	02 0.06	6	0.2 0.	6 2	.0	6.0 2	60	

Description:

SILTY CLAYEY SHALE - Grey brown silty clayey shale

Test Method(s):

AS 1289.3.6.1

Sampling Method(s): Sampled by Engineering Department

Remarks:



NATA Accredited Laboratory Number: 828 This Document is issued in accordance with NATA's accreditation requirements.

Accredited for compliance with ISO/IEC 17025

Tested: LW Checked: BH

Brett Hughes Laboratory Supervisor

FORM R004A REV 5 JULY 2010



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Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

Project:

FERNHILL ESTATE

Report No.:

3

Report Date: Date Sampled: 25-Jun-13 18-Jun-13

Location:

MULOGA

Date of Test:

Page:

20-Jun-13

Road No:

Sample / Pit No: BH 2

Depth / Layer:

0.3-0.5m

Chainage:

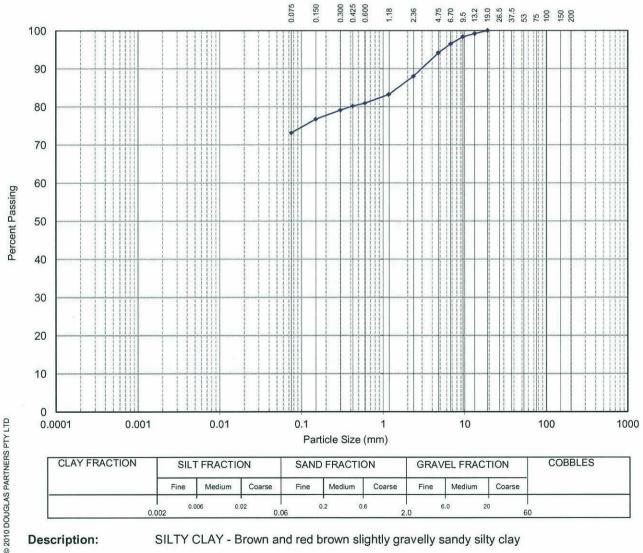
Section / Lot No: -

Test Request No:

1 of

1

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	~
53.0	~
37.5	~
26.5	~
19.0	100%
13.2	99%
9.5	98%
6.7	96%
4.75	94%
2.36	88%
1.18	83%
0.600	81%
0.425	80%
0.300	79%
0.150	77%
0.075	73%

CLAY FRACTION	SIL	FRACTIO	ON	SANE	FRACTIO	NC	GRAV	EL FRAC	TION	COBBLES
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse	
0.0	0.0	006 0.	02 0.0	6	0.2 0.	6 2.	.0	6.0 2	60	

Description:

SILTY CLAY - Brown and red brown slightly gravelly sandy silty clay

Test Method(s):

AS 1289.3.6.1

Sampling Method(s): Sampled by Engineering Department

Remarks:



Tested: LW Checked: BH

Brett Hughes Laboratory Supervisor

FORM R004A REV 5 JULY 2010



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Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

Project:

FERNHILL ESTATE

Report No.:

4 25-Jun-13

Report Date: Date Sampled:

18-Jun-13

Location:

MULOGA

Date of Test:

20-Jun-13

Road No:

Sample / Pit No: BH 3

Depth / Layer:

0.8-1.1m

Chainage:

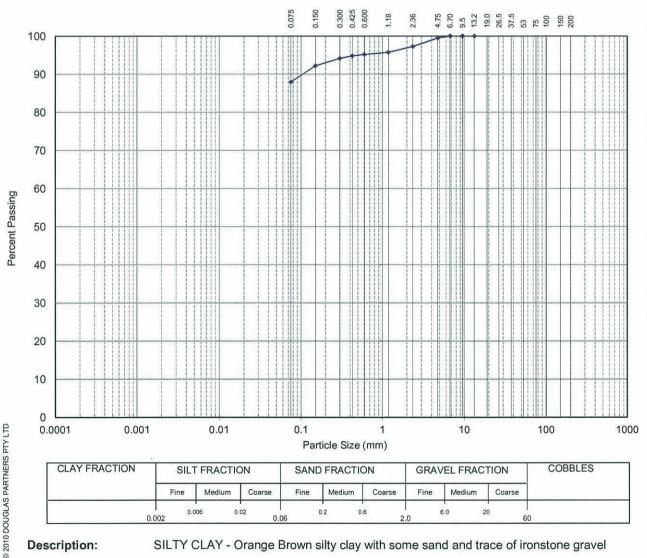
Section / Lot No: -

Test Request No:

Page:

of 1 1

AUSTRALIAN STANDARD SIEVE APERTURES



Fine

Medium

Sieve Size (mm)	% Passing
75.0	~
53.0	~
37.5	~
26.5	~
19.0	~
13.2	100%
9.5	100%
6.7	100%
4.75	99%
2.36	97%
1.18	96%
0.600	95%
0.425	95%
0.300	94%
0.150	92%
0.075	88%

Description:

SILTY CLAY - Orange Brown silty clay with some sand and trace of ironstone gravel

0.6

Fine

Medium

6.0

Coarse

20

Test Method(s):

AS 1289.3.6.1

0.006

Medium

0.02

Sampling Method(s): Sampled by Engineering Department

Remarks:



Tested: LW Checked: BH

Brett Hughes Laboratory Supervisor

FORM R004A REV 5 JULY 2010



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Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

Project :

FERNHILL ESTATE

Report No. : Report Date :

25-Jun-13

Location :

Date Sampled: Date of Test:

18-Jun-13

Location :

MULOGA

DILA

20-Jun-13

Road No:

-

Sample / Pit No: BH 4

Depth / Layer:

0.4-0.6m

Chainage:

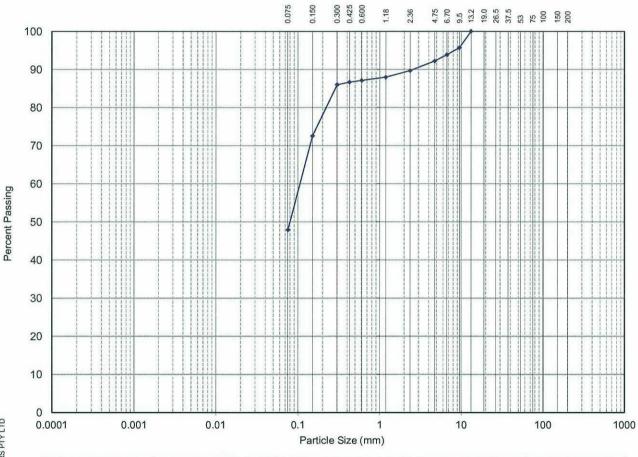
Section / Lot No: -

Test Request No:

Page:

1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing		
75.0	~		
53.0	~		
37.5	~		
26.5	~		
19.0	~		
13.2	100%		
9.5	96%		
6.7	94%		
4.75	92%		
2.36	90%		
1.18	88%		
0.600	87%		
0.425	87%		
0.300	86%		
0.150	73%		
0.075	48%		

CLAY FRACTION	SIL	FRACTIO	NC	SAND FRACTION (GRAVEL FRACTION			COBBLES
Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse		
0.	002	006 0.	02 0.0	6	0.2 0	.6 2	.0	6.0 2	60	

Description:

SAND AND SILT/CLAY - Brown and yellow brown sand and silt/clay with some gravel

Test Method(s):

AS 1289.3.6.1

Sampling Method(s): Sampled by Engineering Department

Remarks:

10000



Tested: LW Checked: BH Brett Hughes Laboratory Supervisor

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Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

Project:

FERNHILL ESTATE

Report No.:

25-Jun-13

Report Date: Date Sampled:

18-Jun-13

Location:

MULOGA

Date of Test:

20-Jun-13

Road No:

Sample / Pit No: BH 5

Depth / Layer:

0.6-0.8m

Chainage:

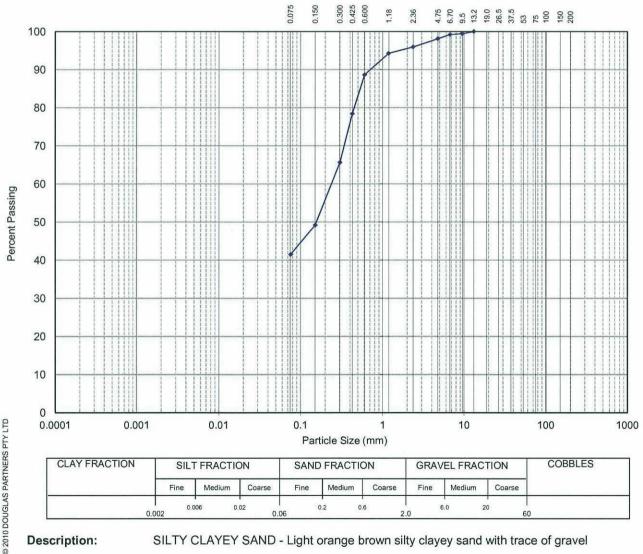
Section / Lot No: -

Test Request No:

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AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	~
53.0	~
37.5	~
26.5	~
19.0	~
13.2	100%
9.5	99%
6.7	99%
4.75	98%
2.36	96%
1.18	94%
0.600	89%
0.425	78%
0.300	66%
0.150	49%
0.075	42%

CLAY FRACTION	SIL	FRACTIO	FRACTION		SAND FRACTION			/EL FRAC	COBBLES	
	Fine	Medium	Coarse	Fine	Medium	Coarse	Fine	Medium	Coarse	
0.	002	006 0.	02	3	0.2 0.	6 2.		6.0 2	60	

Description:

SILTY CLAYEY SAND - Light orange brown silty clayey sand with trace of gravel

Test Method(s):

AS 1289.3.6.1

Sampling Method(s): Sampled by Engineering Department

Remarks:



Tested: LW Checked: BH

Brett Hughes Laboratory Supervisor

FORM R004A REV 5 JULY 2010

NATA Accredited Laboratory Number: 828
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Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

Project:

FERNHILL ESTATE

Report No.:

7

Report Date: Date Sampled: 25-Jun-13 18-Jun-13

Location:

MULOGA

Date of Test:

20-Jun-13

Road No:

Sample / Pit No: BH 6

Depth / Layer:

0.3-0.5m

Chainage:

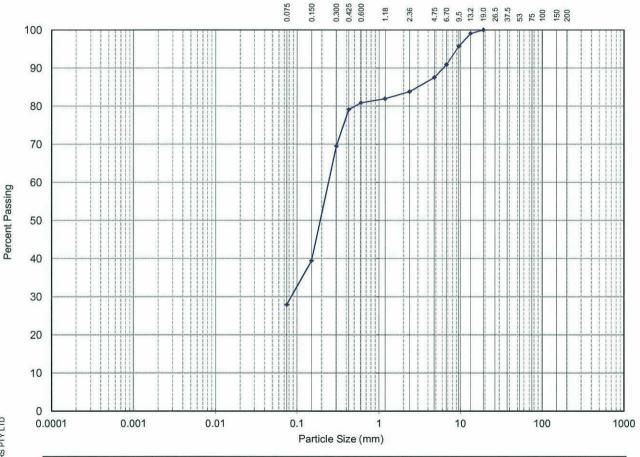
Section / Lot No: -

Test Request No:

Page:

1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	~
53.0	~
37.5	~
26.5	~
19.0	100%
13.2	99%
9.5	96%
6.7	91%
4.75	87%
2.36	84%
1.18	82%
0.600	81%
0.425	79%
0.300	69%
0.150	39%
0.075	28%

CLAY FRACTION COBBLES SILT FRACTION SAND FRACTION **GRAVEL FRACTION** Fine Medium Coarse Fine Medium Coarse Fine Medium Coarse 0.006 0.02 0.2 0.6 6.0 20

Description:

SILTY/CLAYEY SAND - Brown and yellow brown slightly gravelly silty/clayey sand

Test Method(s):

AS 1289.3.6.1

Sampling Method(s): Sampled by Engineering Department

Remarks:



Tested: IW Checked: BH

Brett Hughes Laboratory Supervisor

FORM R004A REV 5 JULY 2010

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Results of Particle Size Distribution

Client:

CUBELIC HOLDINGS PTY LTD

Project No.:

71706.01

Project:

FERNHILL ESTATE

Report No.:

Report Date: Date Sampled: 25-Jun-13 18-Jun-13

Location:

MULOGA

Date of Test:

20-Jun-13

Road No:

Sample / Pit No: BH 7

Depth / Layer:

0.7-1.0m

Chainage:

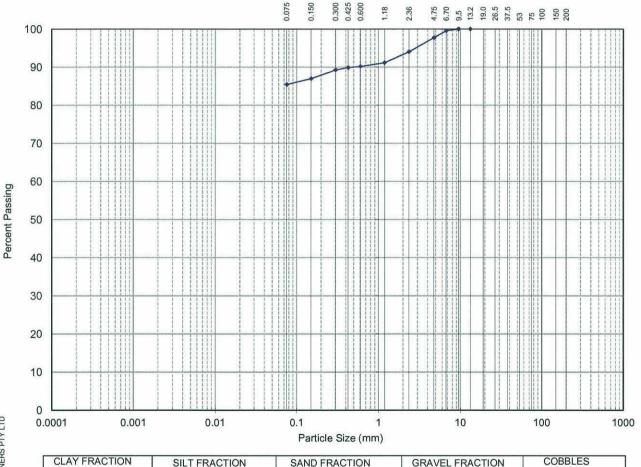
Section / Lot No: -

Test Request No:

Page:

1 of 1

AUSTRALIAN STANDARD SIEVE APERTURES



Sieve Size (mm)	% Passing
75.0	~
53.0	~
37.5	~
26.5	~
19.0	~
13.2	100%
9.5	100%
6.7	100%
4.75	98%
2.36	94%
1.18	91%
0.600	90%
0.425	90%
0.300	89%
0.150	87%
0.075	85%

SILTY CLAY - Light brown silty clay with some sand and gravel

0.2

Fine

SAND FRACTION

Medium

Coarse

0.6

Test Method(s):

Description:

AS 1289.3.6.1

0.006

Fine

SILT FRACTION

Medium

Coarse

0.02

Sampling Method(s): Sampled by Engineering Department

Remarks:



Tested: IW Checked: BH

GRAVEL FRACTION

Medium

6.0

Coarse

20

Fine

Brett Hughes Laboratory Supervisor

FORM R004A REV 5 JULY 2010

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Determination of Emerson Class Number of Soil

Client:

CUBELIC HOLDINGS PTY LTD

Project No: Report No: 71706.01

Project:

FERNHILL ESTATE

Report Date:

25/06/2013

Location:

MULOGA

Date of Test:

24-25/06/2013

Page:

1 of 1

Sample No.	Depth (m)	Description	Water Type	Water Temp	Class No.
BH 1	0.3-0.6	SILTY CLAYEY SHALE - Grey brown silty clayey shale	Distilled	22	2
BH 2	0.3-0.5	SILTY CLAY - Brown and red brown slightly gravelly sandy silty clay	Distilled	22	6
BH 3	0.8-1.1	SILTY CLAY - Orange Brown silty clay with some sand and trace of	Distilled	20	6
BH 4	0.4-0.6	gravel SANDY SILT/CLAY - Brown and yellow brown sandy silt/clay with	Distilled	22	6
BH 5	0.6-0.8	some gravel SILTY CLAYEY SAND - Light orange brown silty clayey sand with	Distilled	22	6
BH 6	0.3-0.5	trace of gravel SILTY/CLAYEY SAND - Brown and yellow brown slightly gravelly	Distilled	22	6
BH 7	0.7-1.0	silty/clayey sand SILTY CLAY - Light brown silty clay with some sand and gravel	Distilled	22	6

Test Methods:

AS 1289 3.8.1

Sampling Methods:

Sampled by Engineering Department

Remarks:



NATA Accredited Laboratory Number: 828

This Document is issued in accordance with NATA's accreditation requirements. Accredited for compliance with ISO/IEC 17025

Tested: LW Checked: BH





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INSITU DENSITY

CLIENT:

CUBELIC HOLDINGS PTY LTD

PROJECT NO:

71706.01

PROJECT:

FERNHILL ESTATE

DATE REPORT: DATE OF TESTING: 20-06-13

25-06-13

LOCATION:

MULOGA

PAGE:

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*	BH 4
,	0.4-0.6m
	*
Description	SAND AND SILT/CLAY - Brown and yellow brown sand and silt/clay with some gravel
Density (t/m ³)	1.97

Method: Density = Mass/ Volume

Report No. 9

Tested: MBG Checked: BH Signed: Brett Hughes Laboratory Supervisor



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CERTIFICATE OF ANALYSIS 92564

Client:

Douglas Partners 96 Hermitage Rd West Ryde NSW 2114

Attention: Ray Blinman

Sample log in details:

Your Reference: 71706, Mulgoa

No. of samples: 7 soils

Date samples received / completed instructions received 19/06/13 / 19/06/13

PHOSPHORUS analysed by EAST WEST report number EW130499

Analysis Details:

Please refer to the following pages for results, methodology summary and quality control data.

Samples were analysed as received from the client. Results relate specifically to the samples as received.

Results are reported on a dry weight basis for solids and on an as received basis for other matrices.

Please refer to the last page of this report for any comments relating to the results.

Report Details:

Date results requested by: / Issue Date: 26/06/13 / 27/06/13

Date of Preliminary Report: 27/06/2013

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Results Approved By:

Jacinta/Hurst Laboratory Manager



Miscellaneous Inorg - soil						
Our Reference:	UNITS	92564-1	92564-2	92564-3	92564-4	92564-5
Your Reference		BH1	BH2	внз	BH4	BH5
Depth		0.3-0.6	0.3-0.5	0.8-1.1	0.4-0.6	0.6-0.8
DateSampled		18/06/2013	18/06/2013	18/06/2013	18/06/2013	18/06/2013
Type of sample		Soil	Soil	Soil	Soil	Soil
Date prepared	-	20/06/2013	20/06/2013	20/06/2013	20/06/2013	20/06/2013
Date analysed	-	20/06/2013	20/06/2013	20/06/2013	20/06/2013	20/06/2013
pH 1:5 soil: water	pH Units	5.5	5.8	5.8	6.1	6.5
Electrical Conductivity 1:5 soil:water	μS/cm	16	20	34	13	13

Miscellaneous Inorg - soil			
Our Reference:	UNITS	92564-6	92564-7
Your Reference		BH6	BH7
Depth		0.3-0.5	0.7-1.0
DateSampled		18/06/2013	18/06/2013
Type of sample		Soil	Soil
Date prepared	-	20/06/2013	20/06/2013
Date analysed	-	20/06/2013	20/06/2013
pH 1:5 soil: water	pH Units	6.1	5.6
Electrical Conductivity 1:5 soil:water	μS/cm	11	20

ESP/CEC						
Our Reference:	UNITS	92564-1	92564-2	92564-3	92564-4	92564-5
Your Reference		BH1	BH2	внз	BH4	BH5
Depth		0.3-0.6	0.3-0.5	0.8-1.1	0.4-0.6	0.6-0.8
DateSampled		18/06/2013	18/06/2013	18/06/2013	18/06/2013	18/06/2013
Type of sample		Soil	Soil	Soil	Soil	Soil
Exchangeable Ca	meq/100g	0.2	0.5	0.8	1.2	<0.1
Exchangeable K	meq/100g	0.3	0.2	0.2	0.2	<0.1
Exchangeable M g	meq/100g	1.8	2.6	6.7	2.2	2.8
Exchangeable Na	meq/100g	0.12	0.13	0.27	<0.1	<0.1
Cation Exchange Capacity	meq/100g	2.5	3.3	8.1	3.7	3.1
ESP	%	4.7	4.0	3.3	1.7	2.9

ESP/CEC			
Our Reference:	UNITS	92564-6	92564-7
Your Reference		BH6	BH7
Depth		0.3-0.5	0.7-1.0
DateSampled		18/06/2013	18/06/2013
Type of sample		Soil	Soil
Exchangeable Ca	meq/100g	<0.1	0.3
Exchangeable K	meq/100g	<0.1	0.2
Exchangeable M g	meq/100g	1.4	2.7
E xchangeable Na	meq/100g	<0.1	0.16
Cation Exchange Capacity	meq/100g	1.6	3.3
ESP	%	2.5	4.7

PhosphorusSorption						
Our Reference:	UNITS	92564-1	92564-2	92564-3	92564-4	92564-5
Your Reference		BH1	BH2	внз	BH4	BH5
Depth		0.3-0.6	0.3-0.5	0.8-1.1	0.4-0.6	0.6-0.8
DateSampled		18/06/2013	18/06/2013	18/06/2013	18/06/2013	18/06/2013
Type of sample		Soil	Soil	Soil	Soil	Soil
Phosphorus Capacity	mg/kg	950	990	930	1,000	870
Phosphorus Buffer Index	mg/kg	430	800	390	1,000	300
PhosphorusSorption	kg/ha	9,500	9,900	9,300	10,000	8,700

PhosphorusSorption			
Our Reference:	UNITS	92564-6	92564-7
Your Reference		BH6	BH7
Depth		0.3-0.5	0.7-1.0
DateSampled		18/06/2013	18/06/2013
Type of sample		Soil	Soil
Phosphorus Capacity	mg/kg	740	990
Phosphorus Buffer Index	mg/kg	190	840
PhosphorusSorption	kg/ha	7,400	9,900

MethodID	MethodologySummary
Inorg-001	pH - Measured using pH meter and electrode in accordance with APHA 22nd ED , 4500-H+.
Inorg-002	Conductivity and Salinity - measured using a conductivity cell and dedicated meter, in accordance with APHA 22nd ED 2510 and Rayment & Lyons.
Metals-009	Determination of exchangeable cations and cation exchange capacity in soil based on Rayment and Lyons 2011.
Ext-062	Analysed by East West Enviroag

		CITE	nt Keterenc	e. / i	706, Muigoz			
QUALITYCONTROL	UNITS	PQL	METHOD	Blank	Duplicate Sm#	Duplicate results	SpikeSm#	Spike % Recovery
Miscellaneous Inorg - soil						Base II Duplicate II % RPD		
Date prepared	-			20/06/2 013	92564-1	20/06/2013 20/06/2013	LCS-1	20/06/2013
D ate analysed	-			20/06/2 013	92564-1	20/06/2013 20/06/2013	LCS-1	20/06/2013
pH 1:5 soil:water	pH Units		Inorg-001	[NT]	92564-1	5.5 5.5 RPD:0	LCS-1	101%
Electrical Conductivity 1:5 soil:water	µЅлат	1	Inorg-002	<1	92564-1	16 15 RPD:6	LCS-1	100%
QUALITYCONTROL	UNITS	PQL	METHOD	Blank	Duplicate Sm#	Duplicate results	SpikeSm#	Spike % Recovery
ESP)CEC						Base II Duplicate II% RPD		
Exchangeable Ca	meq/100	0.1	Metals-009	<0.1	92564-6	<0.1 <0.1	LCS-1	98%
Exchangeable K	meq/100 g	0.1	Metals-009	<0.1	92564-6	<0.1 < 0.1	LCS-1	104%
Exchangeable Mg	meq/100 g	0.1	Metals-009	<0.1	92564-6	1.4 1.4 RPD:0	LCS-1	97%
E xchangeable Na	meq/100 g	0.1	Metals-009	<0.1	92564-6	<0.1 < 0.1	LCS-1	90%
Cation Exchange Capacity	meq/100 g	1	Metals-009	<1.0	92564-6	1.6 1.6 RPD:0	[NR]	[NR]
ESP	%	1	Metals-009	<1.0	92564-6	2.5 2.1 RPD:17	[NR]	[NR]
QUALITYCONTROL Phosphorus Sorption	UNITS	PQL	METHOD	Blank				
PhosphorusCapacity	mg/kg	2	Ext-062	<2.0	1			
Phosphorus Buffer Index	mg/kg	2	Ext-062	<2.0				
PhosphorusSorption	kg/ha	2	Ext-062	<2.0				

Report Comments:

P-sorp's in soil: analysed by EASTWEST, report no EW130499.

Asbestos ID was analysed by Approved Identifier: Not applicable for this job Asbestos ID was authorised by Approved Signatory: Not applicable for this job

INS: Insufficient sample for this test PQL: Practical Quantitation Limit NT: Not tested NA: Test not required RPD: Relative Percent Difference NA: Test not required

Quality Control Definitions

Blank: This is the component of the analytical signal which is not derived from the sample but from reagents, glassware etc, can be determined by processing solvents and reagents in exactly the same manner as for samples.

Duplicate: This is the complete duplicate analysis of a sample from the process batch. If possible, the sample selected should be one where the analyte concentration is easily measurable.

Matrix Spike: A portion of the sample is spiked with a known concentration of target analyte. The purpose of the matrix spike is to monitor the performance of the analytical method used and to determine whether matrix interferences exist.

LCS (Laboratory Control Sample): This comprises either a standard reference material or a control matrix (such as a blank sand or water) fortified with analytes representative of the analyte class, it is simply a check sample.

Surrogate Spike: Surrogates are known additions to each sample, blank, matrix spike and LCS in a batch, of compounds which are similar to the analyte of interest, however are not expected to be found in real samples.

Laboratory Acceptance Criteria

Duplicate sample and matrix spike recoveries may not be reported on smaller jobs, however, were analysed at a frequency to meet or exceed NEPM requirements. All samples are tested in batches of 20. The duplicate sample RPD and matrix spike recoveries for the batch were within the laboratory acceptance criteria.

Filters, swabs, wipes, tubes and badges will not have duplicate data as the whole sample is generally extracted during sample extraction.

Spikes for Physical and Aggregate Tests are not applicable.

For VOCs in water samples, three vials are required for duplicate or spike analysis.

Duplicates: <5xPQL - any RPD is acceptable; >5xPQL - 0-50% RPD is acceptable.

Matrix Spikes, LCS and Surrogate recoveries: Generally 70-130% for inorganics/metals; 60-140% for organics and 10-140% for SVOC and speciated phenols is acceptable.

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