



# SMEC Testing Services Pty Ltd

ACN 101 164 792 ABN 22 101 164 792

CONSULTING GEOTECHNICAL & ENVIRONMENTAL ENGINEERS

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NSW 2164

August 5, 2013  
Project No. 19161/0119  
Report No. 13/1308  
LWI/ms

## SUMMARY SHEET

Client: McDonald Jones Homes  
Address: Lot 2038 Killuna Way, Jordan Springs  
Reference: 601134/016/01



SITE CLASSIFICATION	M	AS2870-2011
WIND CLASSIFICATION	N1	AS4055-2012
EXPOSURE CLASSIFICATION	A2	AS2870-2011

This summary sheet must be read in conjunction with the full report.



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## **SITE INVESTIGATION REPORT**

Client: McDonald Jones Homes

Address: Lot 2038 Killuna Way, Jordan Springs

Proposed Development: Residential dwelling

### ***Site Description***

Approx. area (m<sup>2</sup>): 450

Approx. fall: 1.5 metres to the northwest, reasonable site drainage

Vegetation: Some grass, tree on footpath

Improvements: Vacant – new estate

### ***Geology, Fieldwork Details and Subsurface Conditions***

The Penrith geological series sheet at a scale of 1:100,000 show the site is underlain by Triassic Age Bringelly Shale of the Wianamatta Group. Rocks within this formation comprise shale, claystone and laminite.

Two boreholes were drilled and two Dynamic cone penetrometer (DCP) tests were carried out on July 27, 2013 at the locations shown on Drawing No. 13/1308. The subsurface conditions encountered are shown on the attached borehole logs. Explanation sheets and notes relating to geotechnical reports are also attached.

When making an assessment of the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. The actual condition at the site may differ from those inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies.

The subsurface conditions consist of fill overlying silty clays. The fill is 0.4 and 0.6 metres thick and appears to have been placed as engineered material. Stiff becoming very stiff natural silty clays are present to the depth of drilling, 2.5 metres.

No groundwater was observed in the boreholes during the fieldwork.

### ***Wind Classification***

The classification has been carried out in accordance with the guidelines set out in AS4055-2012 “Wind loads for housing”.

The site is classified N1.

### ***Laboratory Testing***

In order to assist with determining the site classification, a shrink/swell test was carried out on a representative sample retrieved from the site. The detailed test report is attached and summarised below:

Location	Depth (m)	Material Description	Shrink/Swell Index (% per $\Delta pF$ )
BH1	0.6-0.81	Light grey, orange brown silty clay	3.3

### ***Site Classification***

The classification has been prepared in accordance with the guidelines set out in the “Residential Slabs and Footings” Code, AS2870 - 2011.

Based on the subsurface conditions observed the site may be classified *moderately reactive (M)*, provided the recommendations given below are adopted.

Foundation design and construction consistent with this classification shall be adopted as specified in the above referenced standard and in accordance with the following design details.

### ***Foundation Design and Construction***

Pad and/or strip footings founded in controlled fill and underlying natural materials may be proportioned using an allowable bearing pressure of 100 kPa. The minimum depth of founding must comply with the requirements of AS2870. The growth of the tree on the footpath needs to be considered in the foundation design.

Piers founded in the very stiff natural materials may be proportioned using an allowable end bearing pressure of 450 kPa provided their depth to diameter ratio exceeds a value of 4. An adhesion value of 20 kPa may be adopted below a depth of 0.75 metres.



In order to ensure the bearing values given can be achieved, care should be taken to ensure the base of the excavations is free of all loose material prior to concreting. To this end, it is recommended that all excavations be concreted as soon as possible, preferably immediately after excavating, cleaning, inspecting and approval. Pier excavations should not be left open overnight. The possibility of groundwater inflow needs to be considered when drilling the piers and pouring concrete.

The site is considered suitable for slab on ground construction provided due regard is given to the ground surface slope and the fill has been placed in a controlled manner. Otherwise, piers will be required to suspend the slab.

During foundation construction, should the subsurface conditions vary to those inferred in this report, a suitably experienced geotechnical engineer should review the design and recommendations given above to determine if any alterations are required.

### *Soil Aggressiveness*

The exposure classification for the concrete has been determined for the onsite soils. The exposure classification is obtained from Tables 5.1 and 5.2 of AS2870-2011. In regards to the electrical conductivity, the laboratory test results have been multiplied by the appropriate factor to convert the results to  $EC_e$ .

Detailed test reports are attached and summarised below, together with the exposure classification.

Sample No.	Electrical Conductivity (dS/m)		pH	Sulfate (ppm)	Exposure Classification
	$EC_{1:5}$	$EC_e$			
S1/0119	0.601	5.4	8.0	<10	A2

The minimum concrete strength and reinforcement cover required for the various exposure classifications are given in Tables 5.3 and 5.4 of AS2870-2011 (see attached).

Reference to DLWC (2002) "Site Investigations for Urban Salinity" indicates that an  $EC_e$  value of 5.4 dS/m is consistent with moderately saline soils.

### *Additional Comments*

Attention is drawn to Appendix B of AS2870 - 2011 regarding the need to properly maintain the foundations. Surface drainage should be provided to avoid the possibility of water ponding near the building and the finished ground surface should fall at least 50 mm over a distance of one metre away from the building.



The above classification has been made assuming that the maximum depth to filling placed in any building platform will be 400 mm and that all footings will bear in either natural ground or in control filling. Prior to the placement of any filling the existing surface should be stripped of all vegetation and topsoil.

The above classification is based on the soil profiles observed at the time of testing. If site works are undertaken, the classification of the actual building platform may vary across the site depending upon the extent of the cut and/or fill and the degree of compaction of any fill. The designer of the footing system must take the above factors into account.

If excavations for rainwater or detention tanks are to be made within 6 metres of the building foundations, advice should be sought regarding their effect on the foundations.

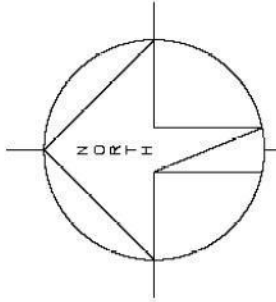
Placing absorption trenches on the high side of the property may create abnormal moisture conditions for the foundations (Refer to Section 1.3.3 of AS2870). This could have a negative effect on the foundation performance and more than likely alter the site classification provided above.

This report has been prepared assuming that no trees other than those noted will be present on the site. If future tree planting is planned, eg. there is a landscaping plan, their effect on the foundation performance must be considered.

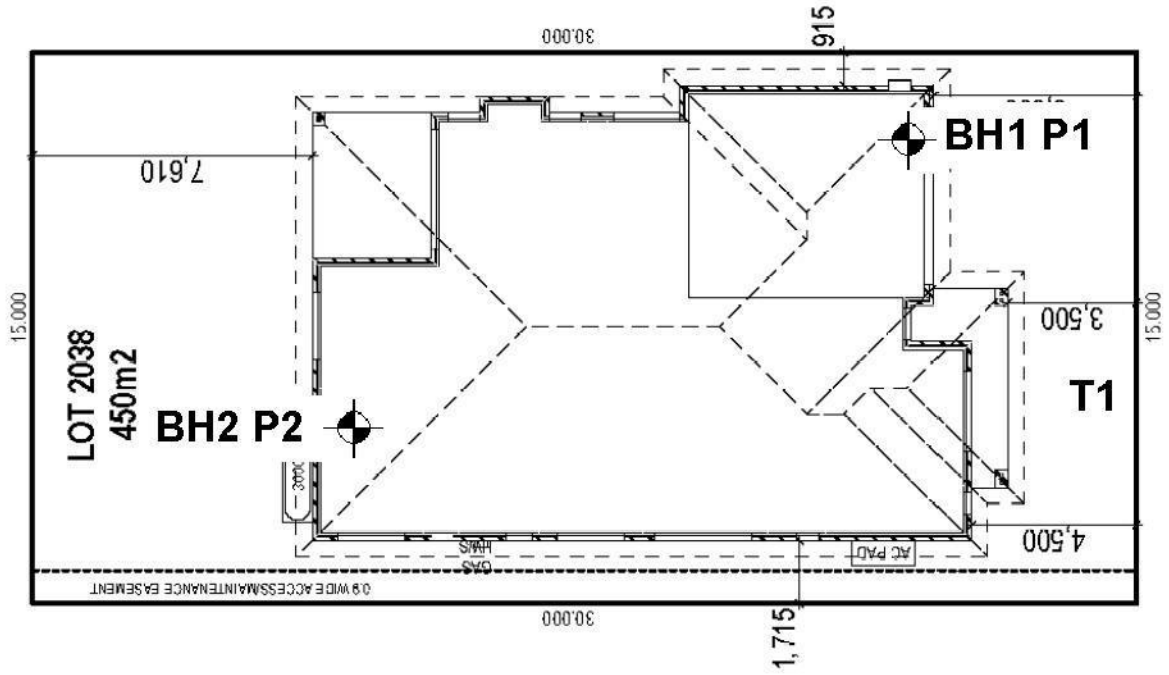
This report has been prepared assuming the site development will be limited to one or two storey residential buildings. The information and interpretation may not be relevant if the design proposal changes (e.g. to a five-storey building involving major cuts during the site preparation). If changes occur, we would be pleased to review the report and advise on the adequacy of the investigation.

A handwritten signature in black ink, appearing to read 'Laurie Ihnativ', written in a cursive style.

Laurie Ihnativ, BE, MEngSc, MBA, FIE Aust.  
Manager, SMEC Testing Services Pty Limited



M.G.A.  
NORTH



**KILLUNA WAY**

<b>SMEC TESTING SERVICES Pty. Ltd.</b>	Scale: Unknown	Date: July 2013
<b>Client: McDONALD JONES HOMES</b>		
<b>SITE INVESTIGATION</b>		Project No.
<b>LOT 2038 KILLUNA WAY, JORDAN SPRINGS</b>		19161/0119
<b>BOREHOLE AND PENETROMETER LOCATIONS</b>		Drawing No: 13/1308

## NOTES RELATING TO GEOTECHNICAL REPORTS

### Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

### Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by SMEC Testing Services Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, SMEC Testing Services Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

### Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, SMEC

Testing Services Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

### Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

### Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

**TABLE 5.3 FROM AS2870-2011****MINIMUM DESIGN CHARACTERISTIC STRENGTH ( $f_c$ ) AND CURING REQUIREMENTS FOR CONCRETE**

Exposure Classification	Minimum $f_c$ MPa	Minimum Initial Curing Requirement
A1	20	Cure continuously for at least 3 days
A2	25	
B1	32	Cure continuously for at least 7 days
B2	40	
C1	>50	
C2	>50	

**TABLE 5.4 – FROM AS2870-2011****MINIMUM REINFORCEMENT COVER FOR CONCRETE**

Exposure Classification	Minimum Cover in Saline Soils* (mm)	Minimum Cover in Sulfate Soils† (mm)
A1	See Clause 5.3.2	40
A2	45	50
B1	50	60
B2	55	65
C1	‡	70
C2	‡	85

\* Where a damp-proofing membrane is installed, the minimum reinforcement cover in saline soils may be reduced to 30 mm.

† Where a damp-proofing membrane is installed, the minimum reinforcement cover in sulfate soils may be reduced by 10 mm.

‡ Saline soils have a maximum exposure classification of B2 as per Table 5.1.



Client: McDonald Jones Homes			Project No.: 19161/0119		BOREHOLE NO.: BH 1	
Project: Lot 2038 Kilmara Way, Jordan Springs			Date: July 27, 2013		Sheet 1 of 1	
Location: Referto Drawing No. 13/1308			Logged: DL			
W A T E R L E V E	S A M P L E S	DEPTH (m.)	DESCRIPTION OF DRILLED PRODUCT  (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY	M
					(cohesive soils) or RELATIVE DENSITY (sands and gravels)	0 1 S T U R E
	S1 @ 0.1m		SILTY CLAY: orange brown with light grey, low plasticity, trace of gravel	CL	VERY STIFF	M
		0.5	FILL			
	U50		SILTY CLAY: orange brown with light grey, low to medium plasticity	CL	STIFF	M
		1.0				
		1.5			VERY STIFF	
		2.0				
		2.5	SILTY CLAY: light grey with red brown, low to medium plasticity, trace of gravel	CL	VERY STIFF	M
			BOREHOLE DISCONTINUED AT 2.5 M			

NOTES: D - disturbed sample	U - undisturbed tube sample	B - bulk sample	Contractor: STS
WT - level of water table or free water	N - Standard Penetration Test (SPT)		Equipment: Christie
See explanation sheets for meaning of all descriptive terms and symbols			Hole Diameter (mm): 100
			Angle from Vertical (°): 0

Client: McDonald Jones Homes			Project No.: 19161/0119		<b>BOREHOLE NO.: BH 2</b>	
Project: Lot 2038 Kilmara Way, Jordan Springs			Date: July 27, 2013		Sheet 1 of 1	
Location: Refer to Drawing No. 13/1308			Logged: DL			
W A T T A M E B R L E	S A M P L E S	DEPTH (m.)	DESCRIPTION OF DRILLED PRODUCT  (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
		0.0	SILTY CLAY: orange brown with light grey, low to medium plasticity, trace of gravel	CL	STIFF	M
		0.5	FILL			
		0.8	SILTY CLAY: orange brown with red brown and light grey, low to medium plasticity, trace of gravel	CL	STIFF	M
		1.0	SILTY CLAY: orange brown with light grey, low to medium plasticity	CL	STIFF	M
		1.5			VERY STIFF	
		2.0				
		2.5	BOREHOLE DISCONTINUED AT 2.5 M			
NOTES: D - disturbed sample                      U - undisturbed tube sample                      B - bulk sample				Contractor: STS		
WT - level of water table or free water                      N - Standard Penetration Test (SPT)				Equipment: Christie		
See explanation sheets for meaning of all descriptive terms and symbols				Hole Diameter (mm): 100		
				Angle from Vertical (°) 0		

**SMEC Testing Services Pty Ltd**

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## Dynamic Cone Penetrometer Test Report

Project: LOT 2038 KILLUNA WAY, JORDAN SPRINGS

Project No.: 19161/0119

Client: **McDONALD JONES HOMES**

Report No.: 13/1308

Address: PO Box 7994, Baulkham Hills

Report Date: July 31, 2013

Test Method: AS 1289.6.3.2

Page: 1 of 1

Site No.	P1	P2				
Location	Refer to Drawing No. 13/1308	Refer to Drawing No. 13/1308				
Starting Level	Surface Level	Surface Level				
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	6	4				
0.15 - 0.30	8	6				
0.30 - 0.45	8	5				
0.45 - 0.60	6	6				
0.60 - 0.75	5	7				
0.75 - 0.90	8	7				
0.90 - 1.05	6	6				
1.05 - 1.20	7	6				
1.20 - 1.35	11	8				
1.35 - 1.50	10	7				
1.50 - 1.65	17	8				
1.65 - 1.80	21	9				
1.80 - 1.95	Discontinued	11				
1.95 - 2.10		16				
2.10 - 2.25		21				
2.25 - 2.40		Discontinued				
2.40 - 2.55						
2.55 - 2.70						
2.70 - 2.85						
2.85 - 3.00						
3.00 - 3.15						
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: \* Pre drilled prior to testing

Approved Signatory..

Laurie Ihnativ - Manager

Technician: DL



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**Shrink Swell Index Report**

Project: LOT 2038 KILLUNA WAY, JORDAN SPRINGS

Client: **McDonald Jones Homes Pty Ltd**

Address: PO Box 7994, Baulkham Hills NSW

Test Method: AS1289.7.1.1

Project No.: 19161 / 3264C

Report No.: 13/1316

Report Date: 1/08/2013

Page: 1 of 1

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.	0119-1				
Sample Location	Borehole 1 Refer to Drawing				
Material Description	SILTY CLAY: orange brown with light grey				
Depth (m)	0.6 - 0.8				
Sample Date	27/07/2013				
Shrink	Moisture Content (%)	21.3			
	Soil Crumbling	Nil			
	Extent of Cracking	Fine Cracks			
	Strain (%)	3.7			
Swell	Moisture Content Initial (%)	21.1			
	Moisture Content Final (%)	25.3			
	Strain (%)	4.5			
Inert Inclusions (%)	Nil				
<b>Shrink Swell Index (%)</b>	<b>3.3</b>				

Remarks:

Approved Signatory.....

Technician: LC

Orlando Mendoza - Lab Manager

## CERTIFICATE OF ANALYSIS

<p><b>Work Order</b> : ES1316905</p> <p><b>Client</b> : SMEC TESTING SERVICES PTY LTD</p> <p><b>Contact</b> : LAURIE IHNATIV</p> <p><b>Address</b> : P O BOX 6989 WETHERILL PARK NSW, AUSTRALIA 2164</p> <p><b>E-mail</b> : laurie@smectesting.com.au</p> <p><b>Telephone</b> : +61 02 9756 2166</p> <p><b>Facsimile</b> : +61 02 97561137</p> <p><b>Project</b> : 19161 3264C</p> <p><b>Order number</b> : 10308</p> <p><b>C-O-C number</b> : 158711</p> <p><b>Sampler</b> : JO</p> <p><b>Site</b> : ----</p> <p><b>Quote number</b> : EN/025/13</p>	<p><b>Page</b> : 1 of 4</p> <p><b>Laboratory</b> : Environmental Division Sydney</p> <p><b>Contact</b> : Client Services</p> <p><b>Address</b> : 277-289 Woodpark Road Smithfield NSW Australia 2164</p> <p><b>E-mail</b> : sydney@alsglobal.com</p> <p><b>Telephone</b> : +61-2-8784 8555</p> <p><b>Facsimile</b> : +61-2-8784 8500</p> <p><b>QC Level</b> : NEPM 2013 Schedule B(3) and ALS QCS3 requirement</p> <p><b>Date Samples Received</b> : 29-JUL-2013</p> <p><b>Issue Date</b> : 02-AUG-2013</p> <p><b>No. of samples received</b> : 10</p> <p><b>No. of samples analysed</b> : 10</p>
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This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. All pages of this report have been checked and approved for release.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results



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Accredited for compliance with  
ISO/IEC 17025.

### Signatories

This document has been electronically signed by the authorized signatories indicated below. Electronic signing has been carried out in compliance with procedures specified in 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Inorganic Chemist	Sydney Inorganics
Celine Conceicao	Senior Spectroscopist	Sydney Inorganics
Hoa Nguyen	Senior Inorganic Chemist	Sydney Inorganics
Nanthini Coilparampil	Laboratory Manager - Inorganics	Sydney Inorganics
Pabi Subba	Senior Organic Chemist	Sydney Inorganics



### **General Comments**

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

LOR = Limit of reporting

^ = This result is computed from individual analyte detections at or above the level of reporting



## Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)

				Client sample ID	0105-S1	0107-S1	0119S1	0126-S1	0133-S1
				Client sampling date / time	[29-JUL-2013]	27-JUL-2013 15:00	27-JUL-2013 15:00	24-JUL-2013 15:00	24-JUL-2013 15:00
Compound	CAS Number	LOR	Unit	ES1316905-001	ES1316905-002	ES1316905-003	ES1316905-004	ES1316905-005	
<b>EA002 : pH (Soils)</b>									
pH Value	----	0.1	pH Unit	4.5	4.3	8.0	7.0	7.7	
<b>EA010: Conductivity</b>									
Electrical Conductivity @ 25°C	----	1	µS/cm	259	24	601	132	137	
<b>EA055: Moisture Content</b>									
Moisture Content (dried @ 103°C)	----	1.0	%	16.2	38.2	14.5	19.3	13.6	
<b>ED040S : Soluble Sulfate by ICPAES</b>									
Sulfate as SO4 2-	14808-79-8	10	mg/kg	100	40	560	140	70	





## Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)

				Client sample ID	0134-S1	0124-S1	0132S1	0151-S1	0147-S1
				Client sampling date / time	26-JUL-2013 15:00	27-JUL-2013 15:00	27-JUL-2013 15:00	27-JUL-2013 15:00	27-JUL-2013 15:00
Compound	CAS Number	LOR	Unit		ES1316905-006	ES1316905-007	ES1316905-008	ES1316905-009	ES1316905-010
<b>EA002 : pH (Soils)</b>									
pH Value	----	0.1	pH Unit		6.5	5.9	6.6	6.2	7.5
<b>EA010: Conductivity</b>									
Electrical Conductivity @ 25°C	----	1	µS/cm		176	39	36	31	1020
<b>EA055: Moisture Content</b>									
Moisture Content (dried @ 103°C)	----	1.0	%		24.7	26.3	13.2	7.1	10.8
<b>ED040S : Soluble Sulfate by ICPAES</b>									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		410	40	40	<10	1540

## QUALITY CONTROL REPORT

<p><b>Work Order</b> : <b>ES1316905</b></p> <p><b>Client</b> : <b>SMEC TESTING SERVICES PTY LTD</b></p> <p><b>Contact</b> : <b>LAURIE IHNATIV</b></p> <p><b>Address</b> : <b>P O BOX 6989</b> <b>WETHERILL PARK NSW, AUSTRALIA 2164</b></p> <p><b>E-mail</b> : <b>laurie@smectesting.com.au</b></p> <p><b>Telephone</b> : <b>+61 02 9756 2166</b></p> <p><b>Facsimile</b> : <b>+61 02 97561137</b></p> <p><b>Project</b> : <b>19161 3264C</b></p> <p><b>Site</b> : <b>----</b></p> <p><b>C-O-C number</b> : <b>158711</b></p> <p><b>Sampler</b> : <b>JO</b></p> <p><b>Order number</b> : <b>10308</b></p> <p><b>Quote number</b> : <b>EN/025/13</b></p>	<p><b>Page</b> : <b>1 of 4</b></p> <p><b>Laboratory</b> : <b>Environmental Division Sydney</b></p> <p><b>Contact</b> : <b>Client Services</b></p> <p><b>Address</b> : <b>277-289 Woodpark Road Smithfield NSW Australia 2164</b></p> <p><b>E-mail</b> : <b>sydney@alsglobal.com</b></p> <p><b>Telephone</b> : <b>+61-2-8784 8555</b></p> <p><b>Facsimile</b> : <b>+61-2-8784 8500</b></p> <p><b>QC Level</b> : <b>NEPM 2013 Schedule B(3) and ALS QCS3 requirement</b></p> <p><b>Date Samples Received</b> : <b>29-JUL-2013</b></p> <p><b>Issue Date</b> : <b>02-AUG-2013</b></p> <p><b>No. of samples received</b> : <b>10</b></p> <p><b>No. of samples analysed</b> : <b>10</b></p>
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This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. All pages of this report have been checked and approved for release.

This Quality Control Report contains the following information:

- Laboratory Duplicate (DUP) Report; Relative Percentage Difference (RPD) and Acceptance Limits
- Method Blank (MB) and Laboratory Control Spike (LCS) Report; Recovery and Acceptance Limits
- Matrix Spike (MS) Report; Recovery and Acceptance Limits



NATA Accredited Laboratory 825

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### Signatories

This document has been electronically signed by the authorized signatories indicated below. Electronic signing has been carried out in compliance with procedures specified in 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Inorganic Chemist	Sydney Inorganics
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### **General Comments**

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

Key :  
Anonymous = Refers to samples which are not specifically part of this work order but formed part of the QC process lot  
CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.  
LOR = Limit of reporting  
RPD = Relative Percentage Difference  
# = Indicates failed QC



## Laboratory Duplicate (DUP) Report

The quality control term Laboratory Duplicate refers to a randomly selected intralaboratory split. Laboratory duplicates provide information regarding method precision and sample heterogeneity. The permitted ranges for the Relative Percent Deviation (RPD) of Laboratory Duplicates are specified in ALS Method QWI-EN/38 and are dependent on the magnitude of results in comparison to the level of reporting: Result < 10 times LOR:- No Limit; Result between 10 and 20 times LOR:- 0% - 50%; Result > 20 times LOR:- 0% - 20%.

Sub-Matrix: **SOIL**

				Laboratory Duplicate (DUP) Report					
Laboratory sample ID	Client sample ID	Method: Compound	CAS Number	LOR	Unit	Original Result	Duplicate Result	RPD (%)	Recovery Limits (%)
<b>EA002 : pH (Soils) (QC Lot: 2989613)</b>									
ES1316905-001	0105-S1	EA002: pH Value	----	0.1	pH Unit	4.5	4.5	0.0	0% - 20%
ES1316905-004	0126-S1	EA002: pH Value	----	0.1	pH Unit	7.0	6.9	0.0	0% - 20%
<b>EA010: Conductivity (QC Lot: 2989615)</b>									
ES1316905-001	0105-S1	EA010: Electrical Conductivity @ 25°C	----	1	µS/cm	259	261	0.7	0% - 20%
ES1316940-002	Anonymous	EA010: Electrical Conductivity @ 25°C	----	1	µS/cm	47	48	2.5	0% - 20%
<b>EA055: Moisture Content (QC Lot: 2990276)</b>									
ES1316904-002	Anonymous	EA055-103: Moisture Content (dried @ 103°C)	----	1.0	%	9.2	8.8	3.5	No Limit
ES1316905-009	0151-S1	EA055-103: Moisture Content (dried @ 103°C)	----	1.0	%	7.1	8.0	11.8	No Limit
<b>ED040S: Soluble Major Anions (QC Lot: 2989614)</b>									
ES1316905-004	0126-S1	ED040S: Sulfate as SO4 2-	14808-79-8	10	mg/kg	140	140	0.0	0% - 20%
ES1316940-004	Anonymous	ED040S: Sulfate as SO4 2-	14808-79-8	10	mg/kg	<10	<10	0.0	0% - 20%



### Method Blank (MB) and Laboratory Control Spike (LCS) Report

The quality control term Method / Laboratory Blank refers to an analyte free matrix to which all reagents are added in the same volumes or proportions as used in standard sample preparation. The purpose of this QC parameter is to monitor potential laboratory contamination. The quality control term Laboratory Control Sample (LCS) refers to a certified reference material, or a known interference free matrix spiked with target analytes. The purpose of this QC parameter is to monitor method precision and accuracy independent of sample matrix. Dynamic Recovery Limits are based on statistical evaluation of processed LCS.

Sub-Matrix: SOIL

Method: Compound	CAS Number	LOR	Unit	Method Blank (MB) Report	Laboratory Control Spike (LCS) Report			
				Result	Spike Concentration	Spike Recovery (%)		
						LCS	Low	High
<b>EA010: Conductivity (QCLot: 2989615)</b>								
EA010: Electrical Conductivity @ 25°C	----	1	µS/cm	<1	1412 µS/cm	101	70	130
<b>ED040S: Soluble Major Anions (QCLot: 2989614)</b>								
ED040S: Sulfate as SO4 2-	14808-79-8	10	mg/kg	<10	750 mg/kg	104	84	112

### Matrix Spike (MS) Report

The quality control term Matrix Spike (MS) refers to an intralaboratory split sample spiked with a representative set of target analytes. The purpose of this QC parameter is to monitor potential matrix effects on analyte recoveries. Static Recovery Limits as per laboratory Data Quality Objectives (DQOs). Ideal recovery ranges stated may be waived in the event of sample matrix interference.

- No Matrix Spike (MS) Results are required to be reported.

### Matrix Spike (MS) and Matrix Spike Duplicate (MSD) Report

The quality control term Matrix Spike (MS) and Matrix Spike Duplicate (MSD) refers to intralaboratory split samples spiked with a representative set of target analytes. The purpose of these QC parameters are to monitor potential matrix effects on analyte recoveries. Static Recovery Limits as per laboratory Data Quality Objectives (DQOs). Ideal recovery ranges stated may be waived in the event of sample matrix interference.

- No Matrix Spike (MS) or Matrix Spike Duplicate (MSD) Results are required to be reported.

## INTERPRETIVE QUALITY CONTROL REPORT

Work Order	: ES1316905	Page	: 1 of 6
Client	: SMEC TESTING SERVICES PTY LTD	Laboratory	: Environmental Division Sydney
Contact	: LAURIE IHNATIV	Contact	: Client Services
Address	: P O BOX 6989 WETHERILL PARK NSW, AUSTRALIA 2164	Address	: 277-289 Woodpark Road Smithfield NSW Australia 2164
E-mail	: laurie@smectesting.com.au	E-mail	: sydney@alsglobal.com
Telephone	: +61 02 9756 2166	Telephone	: +61 2-8784 8555
Faxsimile	: +61 02 97561137	Faxsimile	: +61 2-8784 8500
Project	: 19161 3264C	QC Level	: NEPM 2013 Schedule B(3) and ALS QCS3 requirement
Site	: ---	Date Samples Received	: 29-JUL-2013
C-O-C number	: 158711	Issue Date	: 02-AUG-2013
Sampler	: JO	No. of samples received	: 10
Order number	: 10308	No. of samples analysed	: 10
Quote number	: EN/025/13		

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. All pages of this report have been checked and approved for release.

This Interpretive Quality Control Report contains the following information:

- Analysis Holding Time Compliance
- Quality Control Parameter Frequency Compliance
- Brief Method Summaries
- Summary of Outliers



## Analysis Holding Time Compliance

This report summarizes extraction / preparation and analysis times and compares each with recommended holding times (USEPA SW 846, APHA, AS and NEPM) based on the sample container provided. Dates reported represent first date of extraction or analysis and preclude subsequent dilutions and reruns. A listing of breaches (if any) is provided herein.

Holding time for leachate methods (e.g. TCLP) vary according to the analytes reported. Assessment compares the leach date with the shortest analyte holding time for the equivalent soil method. These are: organics 14 days, mercury 28 days & other metals 180 days. A recorded breach does not guarantee a breach for all non-volatile parameters.

Holding times for VOC in soils vary according to analytes of interest. Vinyl Chloride and Styrene holding time is 7 days; others 14 days. A recorded breach does not guarantee a breach for all VOC analytes and should be verified in case the reported breach is a false positive or Vinyl Chloride and Styrene are not key analytes of interest/concern.

Matrix: SOIL

Evaluation: \* = Holding time breach ; ✓ = Within holding time.

Method Container / Client Sample ID(s)	Sample Date	Extraction / Preparation			Analysis			
		Date extracted	Due for extraction	Evaluation	Date analysed	Due for analysis	Evaluation	
<b>EA002 : pH (Soils)</b>								
Soil Glass Jar - Unpreserved (EA002) 0126-S1, 0133-S1	24-JUL-2013	31-JUL-2013	31-JUL-2013	✓	31-JUL-2013	31-JUL-2013	✓	
Soil Glass Jar - Unpreserved (EA002) 0134-S1	26-JUL-2013	31-JUL-2013	02-AUG-2013	✓	31-JUL-2013	31-JUL-2013	✓	
Soil Glass Jar - Unpreserved (EA002) 0107-S1, 0124-S1, 0151-S1, 0119S1, 0132S1, 0147-S1	27-JUL-2013	31-JUL-2013	03-AUG-2013	✓	31-JUL-2013	31-JUL-2013	✓	
Soil Glass Jar - Unpreserved (EA002) 0105-S1	29-JUL-2013	31-JUL-2013	05-AUG-2013	✓	31-JUL-2013	31-JUL-2013	✓	
<b>EA010: Conductivity</b>								
Soil Glass Jar - Unpreserved (EA010) 0126-S1, 0133-S1	24-JUL-2013	31-JUL-2013	31-JUL-2013	✓	31-JUL-2013	28-AUG-2013	✓	
Soil Glass Jar - Unpreserved (EA010) 0134-S1	26-JUL-2013	31-JUL-2013	02-AUG-2013	✓	31-JUL-2013	28-AUG-2013	✓	
Soil Glass Jar - Unpreserved (EA010) 0107-S1, 0124-S1, 0151-S1, 0119S1, 0132S1, 0147-S1	27-JUL-2013	31-JUL-2013	03-AUG-2013	✓	31-JUL-2013	28-AUG-2013	✓	
Soil Glass Jar - Unpreserved (EA010) 0105-S1	29-JUL-2013	31-JUL-2013	05-AUG-2013	✓	31-JUL-2013	28-AUG-2013	✓	
<b>EA055: Moisture Content</b>								
Soil Glass Jar - Unpreserved (EA055-103) 0126-S1, 0133-S1	24-JUL-2013	----	----	----	30-JUL-2013	07-AUG-2013	✓	
Soil Glass Jar - Unpreserved (EA055-103) 0134-S1	26-JUL-2013	----	----	----	30-JUL-2013	09-AUG-2013	✓	
Soil Glass Jar - Unpreserved (EA055-103) 0107-S1, 0124-S1, 0151-S1, 0119S1, 0132S1, 0147-S1	27-JUL-2013	----	----	----	30-JUL-2013	10-AUG-2013	✓	
Soil Glass Jar - Unpreserved (EA055-103) 0105-S1	29-JUL-2013	----	----	----	30-JUL-2013	12-AUG-2013	✓	



Matrix: SOIL

Evaluation: \* = Holding time breach ; ✓ = Within holding time.

Method Container / Client Sample ID(s)	Sample Date	Extraction / Preparation			Analysis			
		Date extracted	Due for extraction	Evaluation	Date analysed	Due for analysis	Evaluation	
<b>ED040S : Soluble Sulfate by ICPAES</b>								
Soil Glass Jar - Unpreserved (ED040S) 0126-S1,	0133-S1	24-JUL-2013	31-JUL-2013	31-JUL-2013	✓	31-JUL-2013	28-AUG-2013	✓
Soil Glass Jar - Unpreserved (ED040S) 0134-S1		26-JUL-2013	31-JUL-2013	02-AUG-2013	✓	31-JUL-2013	28-AUG-2013	✓
Soil Glass Jar - Unpreserved (ED040S) 0107-S1, 0124-S1, 0151-S1,	0119S1, 0132S1, 0147-S1	27-JUL-2013	31-JUL-2013	03-AUG-2013	✓	31-JUL-2013	28-AUG-2013	✓
Soil Glass Jar - Unpreserved (ED040S) 0105-S1		29-JUL-2013	31-JUL-2013	05-AUG-2013	✓	31-JUL-2013	28-AUG-2013	✓





## Quality Control Parameter Frequency Compliance

The following report summarises the frequency of laboratory QC samples analysed within the analytical lot(s) in which the submitted sample(s) was(when) processed. Actual rate should be greater than or equal to the expected rate. A listing of breaches is provided in the Summary of Outliers.

Matrix: **SOIL**

Evaluation: \* = Quality Control frequency not within specification ; ✓ = Quality Control frequency within specification.

Quality Control Sample Type	Method	Count		Rate (%)			Quality Control Specification
		QC	Reaular	Actual	Expected	Evaluation	
<b>Analytical Methods</b>							
<b>Laboratory Duplicates (DUP)</b>							
Electrical Conductivity (1:5)	EA010	2	20	10.0	10.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
Major Anions - Soluble	ED040S	2	16	12.5	10.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
Moisture Content	EA055-103	2	15	13.3	10.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
pH (1:5)	EA002	2	20	10.0	10.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
<b>Laboratory Control Samples (LCS)</b>							
Electrical Conductivity (1:5)	EA010	1	20	5.0	5.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
Major Anions - Soluble	ED040S	1	16	6.3	5.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
<b>Method Blanks (MB)</b>							
Electrical Conductivity (1:5)	EA010	1	20	5.0	5.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement
Major Anions - Soluble	ED040S	1	16	6.3	5.0	✓	NEPM 2013 Schedule B(3) and ALS QCS3 requirement



## Brief Method Summaries

The analytical procedures used by the Environmental Division have been developed from established internationally recognized procedures such as those published by the US EPA, APHA, AS and NEPM. In house developed procedures are employed in the absence of documented standards or by client request. The following report provides brief descriptions of the analytical procedures employed for results reported in the Certificate of Analysis. Sources from which ALS methods have been developed are provided within the Method Descriptions.

<i>Analytical Methods</i>	<i>Method</i>	<i>Matrix</i>	<i>Method Descriptions</i>
pH (1:5)	EA002	SOIL	(APHA 21st ed., 4500H+) pH is determined on soil samples after a 1:5 soil/water leach. This method is compliant with NEPM (2013) Schedule B(3) (Method 103)
Electrical Conductivity (1:5)	EA010	SOIL	(APHA 21st ed., 2510) Conductivity is determined on soil samples using a 1:5 soil/water leach. This method is compliant with NEPM (2013) Schedule B(3) (Method 104)
Moisture Content	EA055-103	SOIL	A gravimetric procedure based on weight loss over a 12 hour drying period at 103-105 degrees C. This method is compliant with NEPM (2013) Schedule B(3) Section 7.1 and Table 1 (14 day holding time).
Major Anions - Soluble	ED040S	SOIL	In-house. Soluble Anions are determined off a 1:5 soil / water extract by ICPAES.
<i>Preparation Methods</i>	<i>Method</i>	<i>Matrix</i>	<i>Method Descriptions</i>
1:5 solid / water leach for soluble analytes	EN34	SOIL	10 g of soil is mixed with 50 mL of distilled water and tumbled end over end for 1 hour. Water soluble salts are leached from the soil by the continuous suspension. Samples are settled and the water filtered off for analysis.



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## Summary of Outliers

### **Outliers : Quality Control Samples**

The following report highlights outliers flagged in the Quality Control (QC) Report. Surrogate recovery limits are static and based on USEPA SW846 or ALS-QWI/EN/38 (in the absence of specific USEPA limits). This report displays QC Outliers (breaches) only.

#### **Duplicates, Method Blanks, Laboratory Control Samples and Matrix Spikes**

- For all matrices, no Method Blank value outliers occur.
- For all matrices, no Duplicate outliers occur.
- For all matrices, no Laboratory Control outliers occur.
- For all matrices, no Matrix Spike outliers occur.

#### **Regular Sample Surrogates**

- For all regular sample matrices, no surrogate recovery outliers occur.

### **Outliers : Analysis Holding Time Compliance**

This report displays Holding Time breaches only. Only the respective Extraction / Preparation and/or Analysis component is/are displayed.

- No Analysis Holding Time Outliers exist.

### **Outliers : Frequency of Quality Control Samples**

The following report highlights breaches in the Frequency of Quality Control Samples.

- No Quality Control Sample Frequency Outliers exist.
-

## E1 CLASSIFICATION OF SOILS

### E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by SMLC in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-1981, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

#### Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour)

#### Soil condition

- moisture condition
- consistency or density index

#### Soil structure

- structure (zoning, defects, cementing)

#### Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

### E1.2 Soil Composition

#### (a) Soil Name and Classification Symbol

The USC system is summarized in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils - more than 50% of the material less than 60 mm is larger than 0.06 mm (60 µm).
- Fine grained soils - more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 µm).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE.

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 µm
Silt (2)		2 µm to 60 µm
Sand	Fine	60 µm to 200 µm
	Medium	200 µm to 600 µm
	Coarse	600 µm to 2 mm
Gravel (3)	Fine	2 mm to 6 mm
	Medium	6 mm to 20 mm
	Coarse	20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12% in increasing order of significance, eg sandy silty clay).

Minor components of a soil are included in the description by means of the terms "some" and "trace" as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3.

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	M
Clay	C
Organic	O
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4.

TABLE E.1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	P
Silty	M
Clayey	C
Liquid Limit <math>50 u</math> - low to medium plasticity	L
Liquid Limit >math>50 u</math> - low to medium plasticity	H

(b) Grading

"Well graded"	Good representation of all particle sizes from the largest to the smallest.
"Poorly graded"	One or more intermediate sizes poorly represented
"Gap graded"	One or more intermediate sizes absent
"Uniformly graded"	Essentially single size material.

(c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

**Angularity** may be expressed as "rounded", "sub-rounded", "sub-angular" or "angular".

**Particle form** can be "equidimensional", "flat" or "elongate".

**Surface texture** can be "glassy", "smooth", "rough", "pitted" or "striated".

(d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

Black	White	Grey	Red
Brown	Orange	Yellow	Green
Blue			

These may be modified as necessary by "light" or "dark". Borderline colours may be described as a combination of two colours, eg. red-brown.

For soils that contain more than one colour terms such as:

- Speckled - Very small (<math>1-10</math> mm dia) patches
- Mottled - Irregular
- Blotched - Large irregular (>75 mm dia)
- Streaked - Randomly oriented streaks

(e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

E.1.3 Soil Condition

(a) Moisture

Soil moisture condition is described as "dry", "moist" or "wet".

The moisture categories are defined as:

Dry (D) - Little or no moisture evident. Soils are running.  
Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit.

(b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E.1.3.1.

TABLE E.1.3.1 - CONSISTENCY OF FINE-GRAINED SOILS

TERM	UNCONFINED STRENGTH (kPa)	FIELD IDENTIFICATION
Very Soft	<math>< 25</math>	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.
Firm	50 - 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength ( $q_u = 2 c_u$ ).

(c) Density Index

The in situ density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SP1 N VALUE	STATIC CONE VALUE, $q_c$ (MPa)	DENSITY INDEX $I_{p(0)}$
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8 - 25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

#### E1.4 Soil Structure

##### (a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample

Lens - discontinuous with lenticular shape

Pocket - irregular inclusion

Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

##### (b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (filled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded.

#### E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

"Residual Soil" - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

"Colluvium" - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

"Landslide Debris" - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure.

"Alluvium" - Material which has been transported essentially by water. Usually associated with former stream activity.

"Fill" - Material which has been transported and placed by man. This can range from natural soils which have been placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

#### E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clays.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy - an increase in volume due to shearing - is indicated by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- feels gritty to the teeth

#### E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes "O" or "H" depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an "organic material" by classification.

Coal and lignite should be described as such and not simply as organic matter.