# Traffic Impact Assessment

Werrington Subdivision

89914069

Prepared for Lendlease

2 October 2019





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### 1 Introduction

Cardno has been invited by Lendlease to prepare a Traffic Impact Assessment (TIA) to supplement a Development Application (DA) for the residential and industrial subdivision located at 16 Chapman Street, Werrington. The subject site is within the Penrith City Council and the development seeks to create 265 Residential Lots together with 14 light industrial lots. The light industrial lots have a combined Land Area of 31,000 m<sup>2</sup>.

The development also creates several super lots which create the potential for a further 91 Residential Lots and 33 apartments.

The staging of the development is per the table below:

Table 1-1 Development Staging

Stages	RESIDENTIAL LOTS (No)		APARTMENTS (No)		LIGHT INDUSTRIAL GROSS AREA (m²)	
	This DA	Future DA	This DA	Future DA	This DA	Future DA
Stage 1A	64	0	0	0	0	0
Stage 1B	60	16	0	0	0	0
Stage 1C and 1D	4	14	0	0	0	0
Stage 2A	44	16	0	0	0	0
Stage 2B	63	45	0	0	0	0
Stage 3	30	0	0	33	0	0
Stage 4A	0	0	0	0	10982.3 m <sup>2</sup>	0
Stage 4B	0	0	0	0	20017.7 m <sup>2</sup>	0
TOTAL	265	91	0	33	31000 m <sup>2</sup>	0

This assessment has been undertaken to demonstrate the compliance of the development with relevant standards and Council controls as well as identifying the relevant traffic impacts (if any) associated with the development.

#### 1.1 Scope of Works

The main objective of this report is to evaluate the traffic impacts that are generated by the subdivision together with its associated impact on the surrounding road network. Cardno's scope of works for this study includes the following tasks:

- > Review background information and collate supplied information from previous Cardno Tech Memo;
- > Review the current transport context, including pedestrian, cycling and public transport networks and the integration of these transport modes with the wider transport network;
- > Assess the traffic impact associated with the development using RMS guidelines; and
- > Review the access arrangements of the proposed Lots in accordance with the relevant standards and guidelines.

#### 1.2 Reference Documents

In preparing this report, reference has been made to a number of background documents, including:

- > Lendlease Werrington Subdivision Traffic Investigation (Cardno, 2019);
- > South Werrington Urban Village Traffic Impact Assessment (Traffix, 2014);
- > 25 Rance Road, Werrington Traffic Impact Assessment (Mott MacDonald, 2015);
- Guide to Traffic Generating Developments (Roads and Maritime, 2002); and
- > Technical Direction (Roads and Maritime, TDT 2013/04a).



## 2 Strategic Context

### 2.1 South Werrington Urban Village (SWUV) Precinct

The South Werrington Urban Village (SWUV) consists of approximately 48 hectares of land between the Great Western Highway and Werrington Railway Station. Penrith Council has a Development Control Plan (DCP) specific to the SWUV and is applicable to the land shown in **Figure 2-1**.



Figure 2-1 SWUV Area

The SWUV structure plan provided within the DCP shows the high level land-use and road hierarchy strategy and broadly informs the future development of land within SWUV. The structure plan is depicted in **Figure 2-2**, with the DCP citing the following access measures:

- > The structure plan envisages the construction of the proposed Werrington Arterial Road. A new major collector road is proposed to link the Werrington Arterial Road to the future employment development to the west. This new link road also provides a separation between the employment and residential land uses. A roundabout at the intersection of the new link road with the Werrington Arterial Road has been proposed as part of the development and can be constructed in stages if required. The location of the new link road is as shown on the Structure Plan, and is located on the residential zoned land.
- > A minor north south road is proposed linking Werrington Station with the Great Western Highway and forming an edge between the employment precinct and the land that forms part of the Wollemi School.
- > The arterial and collector road system are proposed to be designed to accommodate buses and articulated vehicles.
- Local streets are proposed to be generally inter-connective and to link with existing streets in South Werrington.
- A cycle system is proposed to provide movement through the area, linking with surrounding areas including the recreational areas to the east and St Marys. The system links with the proposed cycleway along the western side of the Werrington Arterial Road with the potential to extend northwards.



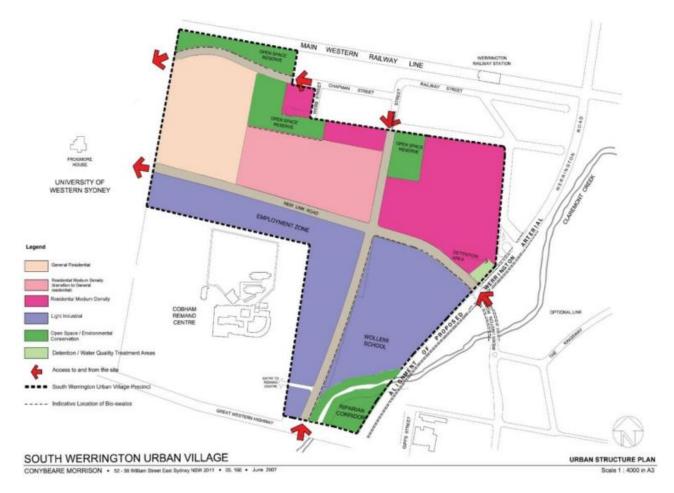


Figure 2-2 SWUV Structure Plan

The SWUV DCP also nominates the minimum level of residential development across the precinct. The total minimum development is shown as 414 dwellings. This is shown in **Figure 2-3**.



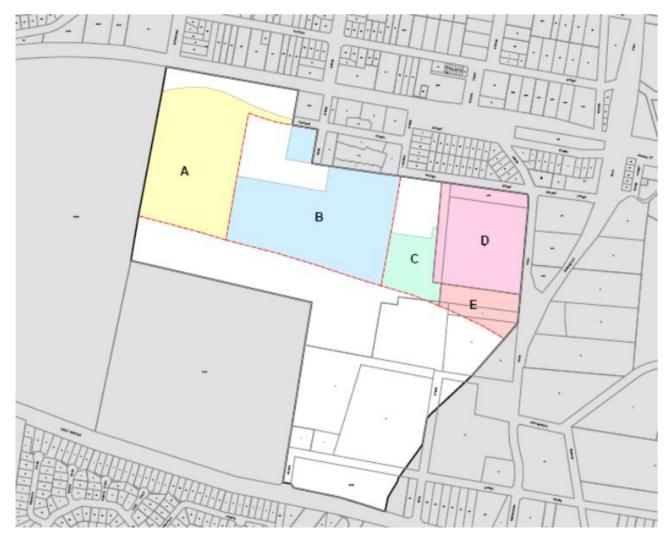


Figure 2-3 SUWV DCP Yield

Sub-Precinct	Minimum dwelling yield
Α	78
В	154
С	30
D	122
E	30
Totals	414

Traffix has previously undertaken a traffic impact assessment for the SWUVP development. The SWUVP Concept Plan envisaged a proposed development of some 2,000 units of high and medium density with a mix of employment and industrial land as well.

Cardno's review of the document identified that with no development, the Rance Road / Werrington Road intersection performs at LoS B & F during the AM and PM peak hour respectively. The Traffix report states that provision of the new East-West link between the subject site and the future signalised intersection with Werrington Road should be considered a priority for redevelopment of the South Werrington Urban Village as a result of the poor performance identified at Rance Road. The proposed signalised intersection along Werrington Road with the proposed East-West link (at the south of Rance Road) would provide additional capacity and ensure an acceptable level of service of intersection performance to cope with future developmental traffic.



The provision of the signalised intersection is likely to be warranted when some 950 residential units are developed to achieve 200veh/hr on the minor roads, as required under the warrant criteria at the time of the assessment.

Development Status within SWUV:

Cardno has undertaken a high level review of relevant documents and studies associated with the site (or nearby developments). They are summarised as follows:

- > South Werrington Urban Village TIA
  - Traffic report prepared by Traffix (6 February 2014)
  - Identified that with no development, Rance Road / Werrington Road performs at LoS B & F during the AM and PM peak hour respectively. The Traffix report states that provision of the new east-west connection between the subject site and the future signalised intersection with Werrington Road should be considered a priority for redevelopment of the South Werrington Urban Village as a result of the poor performance identified at Rance Road.
  - Provision of signalised intersection likely to be warranted when some 950 residential units are developed to achieve 200veh/hr on the minor road required under the warrant criteria at the time of the assessment.
- DA 14-0627 for 85 residential allotments and 2 residue lots
  - Consent details cross section requirements for the east-west road, being a 24m road reserve width with a 14m carriageway.
- > DA 15-0207 for 83 residential allotments (known as the Statewide site)
  - Traffic report prepared by Mott McDonald (February 2015). The report identifies a reduced trip generation rate based on surveys of the existing dwellings in Werrington. A trip rate of 0.44 to 0.54 was estimated based on the survey results, with a rate of 0.40 applied to the development application.
  - This February 2015 report identified that the intersection delay of Rance Road / Werrington Road was not adversely affected and proposed an upgrade to a Seagull intersection for safety reasons.
  - Peer review undertaken by Lyle Marshall & Associates (July 2015) identified capacity issues at the Rance Road / Werrington Road intersection and recommended a roundabout upgrade.
- > DA 16-1148 for 94 residential allotments and 1 superlot
- > DA 16-0789 for 95 apartments (L&EC Approved)
- DA 17-0982 for 751 apartments
  - Application withdrawn
  - Traffic report prepared by Traffix (August 2017)
  - Assessment did not identify an upgrade of Rance Road / Werrington Road intersection under the future traffic loading of the site (Stages 1 to 4 and the Statewide site). LoS B/C was reported for the priority controlled intersection
  - Recommendation to upgrade the roundabout intersection of Werrington Road / The Kingsway with a short northbound lane.
- DA 17/1204 for Place of Public Worship containing 89 place Child Care Centre (refused)
  - Site access via Water Street, directly onto Great Western Highway
  - Land provision for the proposed Lander Street Extension which would travel north towards the Lendlease site (north-south collector road within the DCP Structure Plan)
  - Peak traffic generation appears to be Friday evening (6:00-7:00pm) and Sundays (9:00am 11:30am).
  - Traffic generation in weekday morning in the order of 71 trips whilst the evening would see some 62 trips
  - Peak Friday evening traffic generation is in the order of 105 trips.
  - Peak Sunday morning traffic generation in the order of 676 coinciding with the changeover between two different services.
  - Application refusal appears to be, amongst other things, is due to concurrence with RMS not being achieved regarding access to / from Great Western Highway.



Based on the review, it is understood that there are 262 residential lots, inclusive of the Statewide site, and a further 94 residential apartments approved within the SWUV precinct.



## 3 Existing Conditions

## 3.1 Subject Site

The proposed development is located at the west of Werrington Road (approximately 400m south of Werrington Station) and is bounded to the east by Rance Road/ Werrington Road. It is located approximately 50 km west of Sydney CBD. The site is primarily vacant, with surrounding rural land and residential dwellings. The location of the proposed site is shown in **Figure 3-1.** 

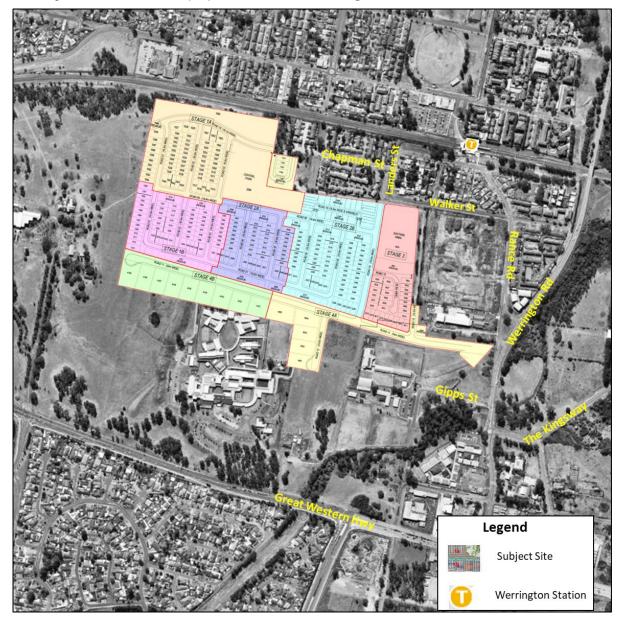


Figure 3-1 Subject Site

#### 3.2 Existing Road Network

#### 3.2.1 Schedule of Road Classification

Roads and Maritime Services (Roads and Maritime) in partnership with local government established an administrative framework of State, Regional and Local Road categories to help manage the extensive network of roads.

State roads are managed and financed by Roads and Maritime, and Regional / Local Roads are managed and financed by Councils. Notwithstanding, Regional Roads perform an intermediate function between the



main arterial network of State Roads and Council controlled Local Roads and therefore received financial assistance from Roads and Maritime.

The key road network surrounding the subject site consists of:

- > Werrington Road;
- > Rance Road;
- > Chapman St/Landers St/Walker St; and
- > Gipps Street.

#### 3.2.2 Werrington Road

Werrington Road is classified as a Regional Road (Road No 7485) under the care and maintenance of the local Council. The road is typically configured with a two-lane carriageway with one lane in each direction. The posted speed limit is 70km/hr.

#### 3.2.3 Rance Road

Rance Road is a local, unclassified road under the care and maintenance of the local Council, and runs in a north-south direction. Rance Road borders the development site to the east. The road is typically configured with a two lane carriageway with a posted speed limit of 50 km/hr.

#### 3.2.4 Chapman Street/Landers Street/Walker Street

Chapman St, Landers St and Walker St are local, unclassified road under the care and maintenance of the local Council with a two-lane undivided carriageway. There is no posted speed limit hence a default speed of 50km/hr applies.

#### 3.2.5 Gipp Street

Gipps St is a local, unclassified road under the care and maintenance of the local Council with a two-lane undivided carriageway. There is no posted speed limit hence a default speed of 50km/hr applies.

#### 3.3 Traffic Control

The description of the intersection in the close proximity of the subject site and their restrictions are given below:

#### Intersection Layout - Werrington Road / Rance Road



#### Description

Werrington Road / Rance Road intersection is a priority control intersection with a 'Give Way' sign at Rance Road to control the traffic. All vehicles approaching the intersection from Rance Road to turn left or right must give way to all vehicles approaching from Werrington Road.



## 3.4 Existing Traffic Volumes (2019)

An indication of the existing traffic volumes in the vicinity of the subject site is provided by peak hour traffic surveys, undertaken by Trans Traffic Survey (TTS), on Tuesday 27 August 2019 at the following locations:

- > Werrington Road/ The Kingsway;
- > Werrington Road/Rance Road; and
- > Rance Road/Walker Street.

In addition to the classified intersection counts, Cardno commissioned TTS to undertake a vehicle delay survey for the right turn onto Werrington Road from Rance Road. A summarised version of the surveys is provided in the **Figure 3-2** to **Figure 3-4**.

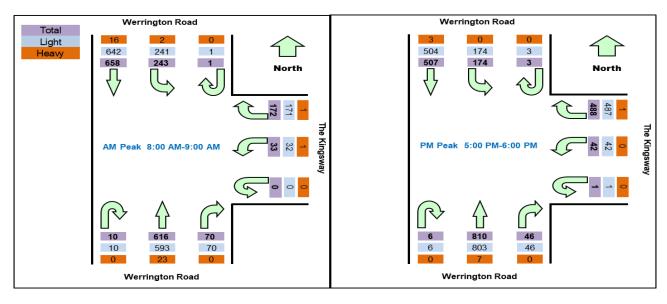


Figure 3-2 Werrington Road/ The Kingsway Intersection

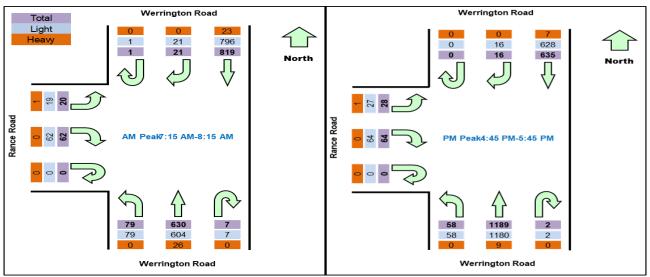




Figure 3-3 Werrington Road/ Rance Road Intersection

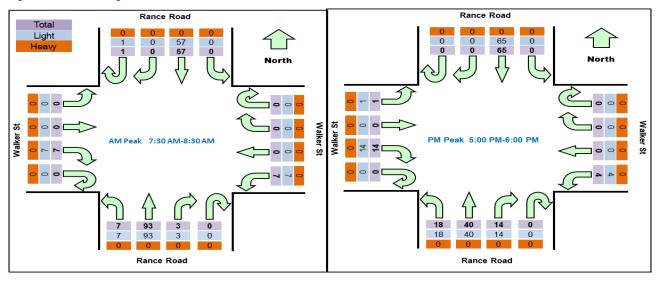


Figure 3-4 Rance Road/Walker Street Intersection



## 4 Existing Public Transport Services

#### 4.1 Train

The proposed location of the subject site is currently well served by public transport services as it is located within 500 metres walk from Werrington Station, which is served by Sydney Trains T1 Western line services.

**Table 4-1** provides a guide of the frequency of the existing train services.

Table 4-1 Train Service Frequency

Line	Weekday Peak Frequency	Weekend Peak Frequency
T1 (Towards Chatswood)	1 Service every 10 minutes	1 Service every 15 minutes
T1 (Towards Emu Plains)	1 Service every 15 minutes	1 Service every 15 minutes

#### 4.2 Buses

Busways operates two routes via Werrington station: Route 782 (Penrith station to St Mary's station) and Route 785 (to Penrith station). A public transport map showcasing the bus routes and train services serving the development and nearby suburbs is shown in **Figure 4-1**.

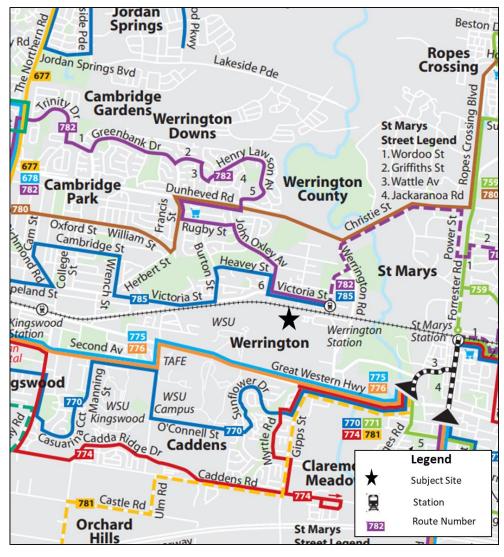


Figure 4-1 Public Transport Map

Source: Transport for NSW

**Table 4-2** provides a guide of the frequency of the existing bus services.



Table 4-2 Bus Service Frequency

Route	Weekdays Peak Frequency	Weekend Frequency
782 (To St Marys)	1 Service every 30 minutes	1 service Every Hour
782 (To Penrith)	1 Service every 30 minutes	1 service Every Hour
785 (To Penrith Station)	3 Services Daily	1 service Every Hour

The proposed development site is conveniently located to take advantage of the connectivity of existing public transport services and encourage the greater use of sustainable modes of transport, therefore reducing reliance on private vehicles.

## 4.3 Walking and Cycling

A desktop review using NearMaps showed that the site is surrounded by footpaths. A figure showing the surrounding footpath is given below in **Figure 4-2**.

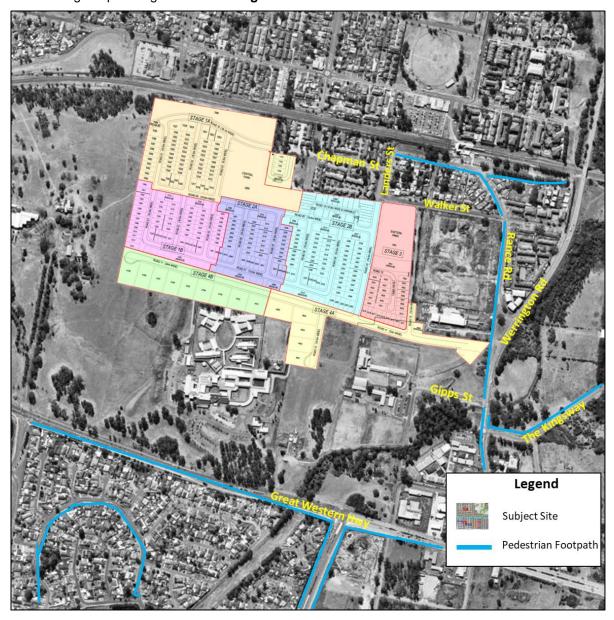


Figure 4-2 Pedestrian Footpath

On the above basis, it can be seen that on the areas surrounding the subject site footpaths are provided on the western side of Werrington Road, which links to Rance Road and ultimately connects to Werrington station. From the latest NearMap aerial image, the width of the footpath is calculated at approximately 1.2 m,



which is considered narrow as Sydney Streets Design Code (2013) recommends a minimum width of 2 m footpath to allow two pairs of people to comfortably pass each other.

In addition, RMS Cycle Way finder was accessed and reviewed and the cycleway network in the close vicinity of the subject site is shown in **Figure 4-3**.

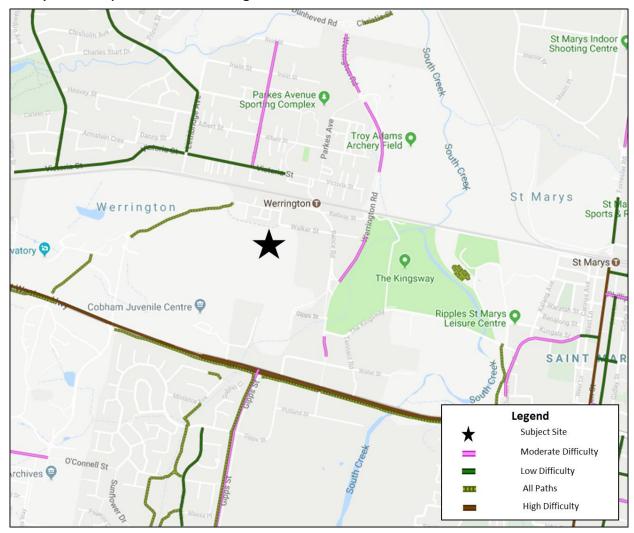


Figure 4-3 Existing Cycleway Network

Source: RMS Cycleway Finder (Accessed: 10/09/2019)



## 5 Proposed Development

## 5.1 Description

The proposed subdivision development is located at 16 Chapman Street, Werrington and the development seeks to create 265 Residential Lots together with 14 light industrial lots. The light industrial lots have a combined Land Area of 31,000 m<sup>2</sup>. The development also creates several super lots which create the potential for a further 91 Residential Lots and 33 apartments.

The proposed development has the following components shown in **Table 5-1**.

Stage 1a and 1b has been assessed for the year 2022 to test the impact of the development. The remaining stages (2-4) which is under the ultimate scenario are expected to be delivered by the year 2029.

This overall aim of this assessment is to ensure that the development master plan Lendlease is proposing has merit and can be achieved considering the traffic assessment impacts.

Table 5-1 Development Components

Stages	RESIDENTIAL LOTS (No)		APARTMENTS (No)		LIGHT INDUSTRIAL GROSS AREA (m²)	
	This DA	Future DA	This DA	Future DA	This DA	Future DA
Stage 1A	64	0	0	0	0	0
Stage 1B	60	16	0	0	0	0
Stage 1C and 1D	4	14	0	0	0	0
Stage 2A	44	16	0	0	0	0
Stage 2B	63	45	0	0	0	0
Stage 3	30	0	0	33	0	0
Stage 4A	0	0	0	0	10,982.3 m <sup>2</sup>	0
Stage 4B	0	0	0	0	20,017.7 m <sup>2</sup>	0
TOTAL	265	91	0	33	31,000 m <sup>2</sup>	0

### 5.2 Proposed Site Access

Vehicular access into and out of the proposed development will be through the existing connections to the development via Walker Street/Chapman Street and a new link road (East-West link) passing through the southern side of the development, which will connect to Werrington Road.

The general arrangement of the proposed development is illustrated in **Figure 5-1**. A larger version of the layout is provided in **Appendix A**.

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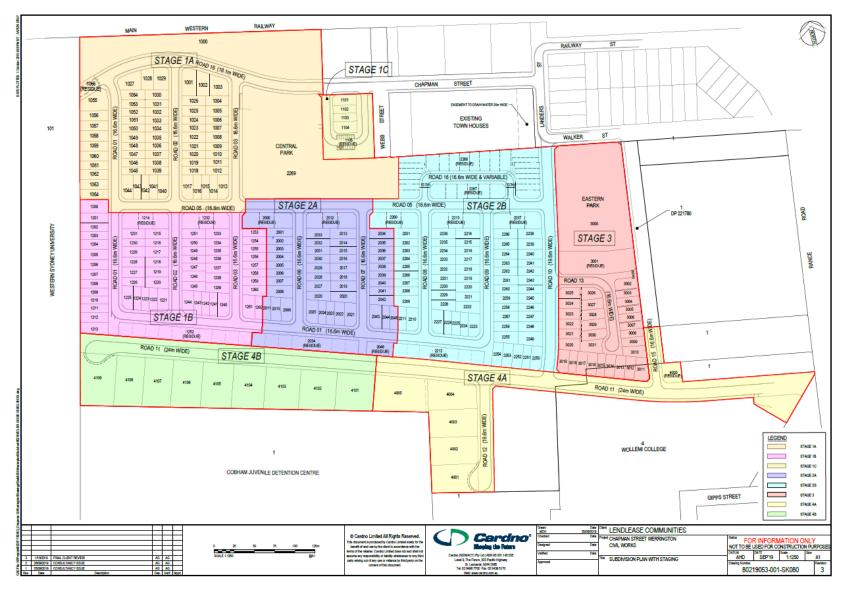


Figure 5-1 Development Layout

Source: Lendlease (dated 28 August 2019)



### 6 Crash Data

Crash data for the period 2014 to 2019 for the study area was accessed through TfNSW Centre for Road Safety. **Figure 6-1** shows the location of crashes within the study area.

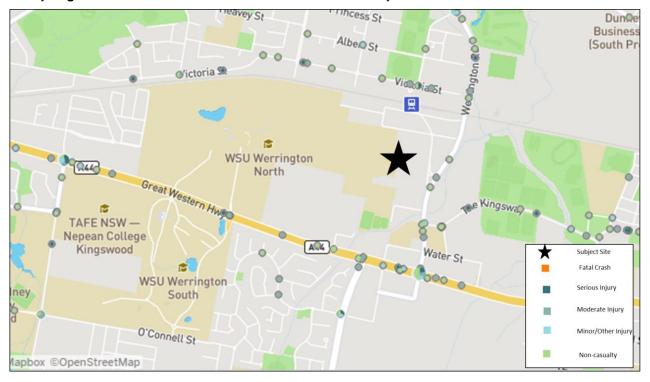


Figure 6-1 Crash locations by injury severity within study area between 2014-2018

The following key findings were identified from the analysis of the crash data:

- > No fatalities were recorded in the study area.
- > There was a total of two crashes at the Werrington Road / Rance Road Intersection (moderate injury and non-casual (tow-away)).
- > There was a total of 20 crashes along Werrington Road which extends from Christie Street to Great Western Highway. Most of the crashes along Werrington Road were non-casual. The details of the crash types are described below:
  - Total of 4 (four) Moderate injury crashes
  - Total of 12 non casual (tow-away) type of crashes
  - 2 (two) serious injury crashes
  - 2 (two) minor injury crashes



## 7 Traffic Assessment

#### 7.1 Assessment Years

As part of this study there is a need to assess intersection performance for key intersections at different design horizon years. This is done to identify capacity issues and propose mitigation measures (if any) that would ensure satisfactory performance in all design years. This assessment focuses on two design years and is in line with the proposed staging development as outlined in **Section 5.1** and **Table 5-1**.

- > Assessing interim requirements at opening year of Stage 1a and 1b only (2022); and
- > Assessing ultimate requirements under a ten-year horizon (2029).

## 7.2 Development Traffic Generation

An indication of the traffic generation potential of the proposed development is sourced from the "25 Rance Road Werrington Traffic Assessment Report" prepared by Mott Macdonald (February 2015). The report adopted a lower trip rate of 0.4 trips per dwelling considering the mixture of low and medium density residential dwellings and the potential for easy access to rail and bus services from the site.

RMS trip generation rates for medium density housing is between 0.4 and 0.65 trips per dwelling. Hence it was deemed reasonable to adopt a trip rate of 0.4 trips per dwelling for the mixture of low and medium density given the current public transport usage for residential commuter trips was around 30%.

Cardno have compared 2019 aerial photography to 2014 to confirm that the number of lots has remained unchanged between 2014 (Mott Macdonald Report traffic survey period) and the current year (2019). Classified intersection counts that were recently undertaken by Cardno (August 2019) were compared against previous assessments (Mott MacDonald, December 2014) relating to the Werrington Road / Rance Road Intersection and showed negligible turning volume differences (except for the northbound through approach) as seen in **Table 7-1**.

Table 7-1 Traffic Survey Comparison Werrington Rd/Rance Rd Intersection (Cardno 2019 vs Mott MacDonald 2014)

		Cardno ( <i>I</i>	August 2019)	Mott MacDonald (December 2014)		
Approach	Movement	AM (7:45-8:45)	PM (16:45-17:45)	AM (7:45-8:45)	PM (16:45-17:45)	
North	Т	819	635	860	645	
North	R	21	16	11	10	
South	L	79	58	56	66	
South	Т	630	1,189	517	1,174	
West	L	20	28	25	26	
West	R	62	64	51	63	
Total (v	/ehs/hr)	1,631	1,990	1,520	1,984	

Therefore, a trip generation rate of 0.4 for both AM and PM peaks for low and medium density residential dwellings has been applied for this assessment.

For High-Density Residential (apartments) the trip generation rates provided by the RMS Technical Direction is 0.19 per dwelling for the AM Peak and 0.15 per dwelling for the PM Peak. As the trip generation rates provided by RMS is extreme for this location this assessment has adopted a rate of 0.29 trips per dwelling considering that the apartment users are likely to have higher car ownership and less public transport availability. Regarding the trip generation for industrial land uses this assessment has adopted the rates provided by the RMS Technical Direction

The adopted traffic generation rates for this assessment is given below:

#### Low and Medium Density Residential

AM Peak: 0.4 trips per lot PM Peak: 0.4 trips per lot

#### High Density Residential (Apartments)

AM Peak: 0.29 trips per dwelling



PM Peak: 0.29 trips per dwelling

Industrial

AM Peak: 0.52 per 100 m<sup>2</sup> of GFA PM Peak: 0.56 per 100 m<sup>2</sup> of GFA

The resulting traffic generation is summarised in Table 7-1.

Table 7-1 Traffic Generation Estimate

Stage Lots/Dwellings/GFA		Traffic Generation (# of Trips)  AM Peak PM Peak		
Stage 1A	64 Lots	26	26	
Stage 1B	76 Lots	30	30	
Stage 1C	18 Lots	7	7	
Stage 2A	60 Lots	24	24	
Stage 2B	108 Lots	43	43	
Stage 3	30 Lots	12	12	
Stage 3	33 Apartments	10	10	
Stage 4A	10982.3 m <sup>2</sup> GFA	57	62	
Stage 4B	20017.7 m <sup>2</sup> GFA	104	112	
Total		313	326	

The proposed development is therefore expected to generate 313 vehicular trips during the 1-hour morning peak and 326 vehicular trips during the evening peak period.

In addition to the traffic generated by the new development, a background traffic growth rate of **2% per annum** was applied to the existing road network. Traffic background growth refers to the traffic volume increase experienced across corridors between different years, which are not impacted by the proposed developments within the study precinct. To calculate this traffic growth, historical traffic background data at the same locations are compared to most recent traffic surveyed data. A growth rate is then determined for the compared time period and a linear annual percentage growth is derived.

#### 7.3 Trip Distribution

- > The ratio of the inbound and outbound traffic movements for residential developments is assumed to be 20:80 in the AM peak hour and 80:20 in the PM peak hour respectively.
- > The ratio of the inbound and outbound traffic movements for industrial developments is assumed to be 50:50 in the AM and PM peak hour respectively.
- Cardno adopted the shortest distance calculation approach in order to derive the number of lots that are expected to use the two site accesses (new East-West link and Rance Road):
  - It is assumed that traffic from development stages 1a and 1c will divert to Chapman Street / Railway
     Street / Rance Road; and
  - The remaining development stages (1b, 2a, 2b, 3, 4a and 4b) are expected to use the new East-West link upon construction completion.
- > The 2019 surveyed turning count proportions were adopted to distribute the North/South trips going in/out of the development. The North/South trip assignment obtained from survey turning counts is shown in **Figure 7-1**.



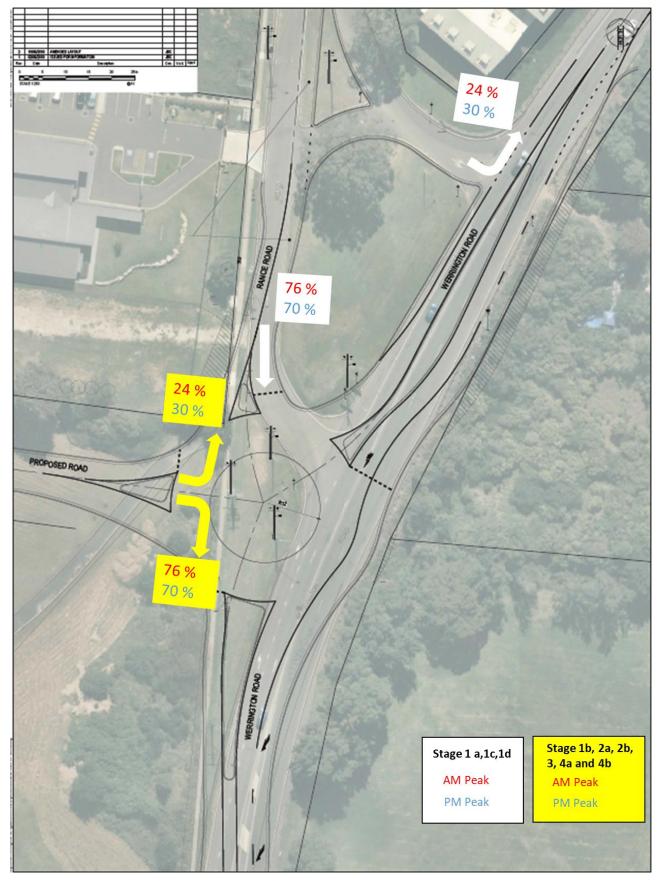


Figure 7-1 North/South Trip Assignment



## 7.4 Key Intersections Operations and Performance

The existing intersection operation performance was assessed using the SIDRA Intersection 8.0 software package. The key indicator of intersection performance is typically the Level of Service (LoS), where results are placed on a scale from 'A' to 'F', outlined in **Table 7-2** 

Table 7-2 Level of Service Criteria for Intersections

Level of Service	Average Delay per Vehicle (sec/veh)	Traffic Signals, Roundabout	Giveway & Stop Signs
Α	< 14	Good Operation	Good Operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near Capacity & accident study required
E	57 to 70	At Capacity, at signals incidents will cause excessive delays Roundabouts require other control mode	At capacity, requires other control mode
F	> 70	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires additional capacity.

Source: Guide to Traffic Generating Developments (RMS, 2002)

The Average Vehicle Delay (AVD) provides a measure of the operational performance of an intersection and determines the LoS when applying the RMS method. It should be noted that the AVD's should be taken as a guide only as longer delays could be tolerated in some locations (i.e. inner city conditions) and on some roads (i.e. minor side street intersecting with a major arterial route). For traffic signals, the weighted average delay over all movements should be utilised. For roundabouts and priority control intersections (sign control) the critical movement for assessing LoS should be the movement with the highest average delay.

The Degree of Saturation (DoS) is another measure of the operational performance of individual intersections. For intersections controlled by traffic signals, both queue length and delay increase rapidly as DOS approaches 1.0. It is usual to attempt to keep DOS to less than 0.9. Degrees of Saturation in the order of 0.7 generally represent satisfactory intersection operation. When DOS exceed 0.9 queues can be anticipated.

#### 7.4.1 SIDRA Model Calibration

The Rance Road / Werrington Road intersection was assessed as an initial step using SIDRA and based on the updated 2019 traffic surveys. SIDRA outputs (under default parameters) showed LoS B in the AM peak with 26.8 seconds of average delay and LoS F in the PM peak with 152.7 seconds of average vehicle delay for the worst movement (right turn onto Werrington Road).

Delay surveys for the right turn from Rance Road onto Werrington Road were analysed and showed that average vehicle delay during the morning 1-hour peak was 17 seconds and for the afternoon 1-hour peak 40 seconds. This suggests a discrepancy between SIDRA outputs and surveyed vehicle delay.

The "critical gap" parameter was adjusted to calibrate the SIDRA model to the observed average vehicle delay for the right turn (for both AM and PM peaks). Critical gap for the Rance Road right turn was adjusted from the default value of 7.0 seconds to 6.35 seconds. The actual critical gap acceptance calculated by SIDRA through the Two Way Sign Control Calibration (TWSC) is actually 3.65 seconds in both the AM and PM peak which is aggressive behaviour. Updated LoS for the 2019 base AM peak remains at LoS B with average delay of 16.6 seconds and improves to LoS C with average delay of 41.7 seconds in the PM peak.

This parameter was carried forward for the future base modelling of 2022 and 2029.



#### 7.4.2 2019 Base Year

The Rance Road / Werrington Road geometry for 2019 base is illustrated in **Figure 7-2**. The intersection assessment has been undertaken using SIDRA v8 network input parameters. This is to enable the model to see the impact of queue propagation as modelled in SIDRA.

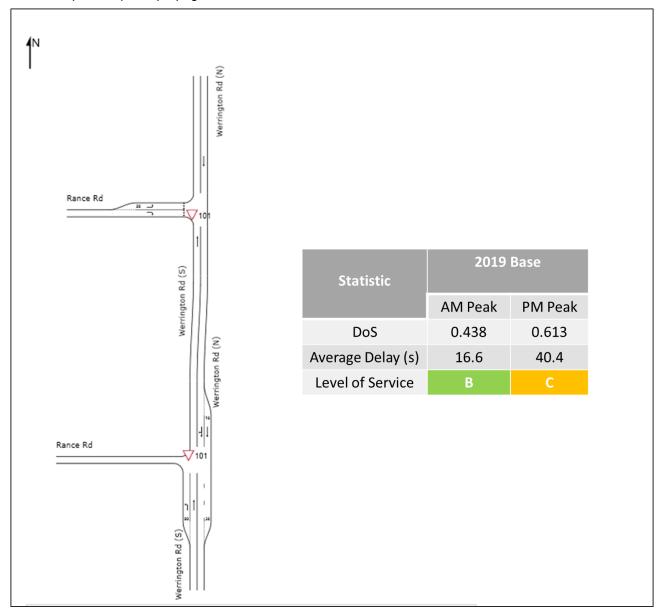


Figure 7-2 2019 Intersection Performance Summary

The intersection of Rance Road / Werrington Road is currently operating at satisfactory LoS B during the AM and LoS C during PM peak hour. The LoS is based on the worst movement which is the right turn movement from Rance Road.

### 7.4.3 2022 Design Horizon Year

The 2022 design horizon year assumes that stage 1a and 1b (including the development of the super lots through subsequent development application) will have been fully constructed without the East-West link in place.

As such, the proposed 4-leg roundabout would be constructed to operate as a 3- leg roundabout until Stage 4a of the development. **Figure 7-3** illustrates the base and upgraded SIDRA networks along with the operational performance summaries.



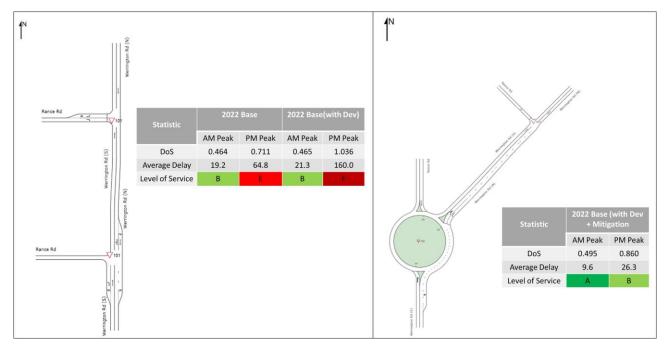


Figure 7-3 2022 Design Horizon Intersection Performance Summary

- Under the 2022 base scenario, the intersection of Rance Road / Werrington Road will operate at LoS B in the AM peak and LoS E in the PM peak. The intersection will deteriorate further to LoS F in the PM peak due to the additional developmental traffic. The worst movement was the right turn movement from Rance Road.
- With the application of the proposed 3-approach roundabout design, the intersection performance of Rance Road / Werrington Road will improve from LoS F to B in the PM peak. The roundabout design will also improve the intersection's performance in the AM peak from LoS B to A.

#### 7.4.4 2029 Design Horizon Year

The 2029 design horizon year assumes that all stages will have been constructed and the East-West link will be in place tying in with the already proposed 3-approach roundabout to convert it into a 4-approach as seen in **Figure 7-4**.

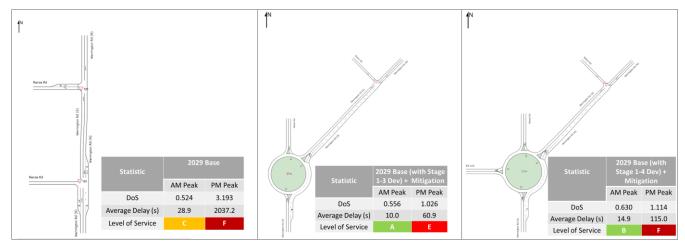


Figure 7-4 2029 Design Horizon Year Intersection Performance Summary

Under the 2029 base case (no developmental traffic), the intersection of Rance Road / Werrington Road will operate unsatisfactorily in the PM peak with LoS F. The LoS is based on the worst movement which is the right turn movement from Rance Road.



- The scenario for three-legged roundabout has only been tested for Stages 1-3, which purely consists of residential development. The four-legged roundabout was tested for all stages 1-4, which consists of both residential and industrial land uses.
- With the application of three-legged roundabout design, the intersection will operate at capacity in the PM peak with LoS E. The LoS is based on the worst movement which is the right turn movement from Rance Road. This LoS E for Rance Road is actually an improvement when compared to the LoS F under the base scenario (>2,000 seconds).
- > The introduction of the proposed East-West link as the fourth arm to the roundabout's design (west approach), will result in unsatisfactory performance during the PM peak with LoS F. The roundabout design however significantly improves the delay performance when compared to the 2029 base case. LoS in the AM peak improves from C to B when compared to the 2029 base. The afternoon peak hour is reported as LoS "F" however, again this is considerably better than the base scenario (>2,000 seconds).
- > Similarly, the SWUV structure plan relies upon the delivery of the Werrington Arterial Stage 2 project. The performance of the intersection of the East West links, Werrington Road and Rance Road will ultimately improve once the capacity of Werrington road increases per the planned Werrington Arterial Road Stage 2 upgrade.

Detailed intersection performance results extracted from SIDRA are provided in Appendix C.



## 8 Public and Active Travel

### 8.1 Proposed Internal Road Network

The road hierarchy proposed by the SWUV structure plan provided within the DCP of Penrith Development Control Plan 2014 is shown in **Figure 8-1**.

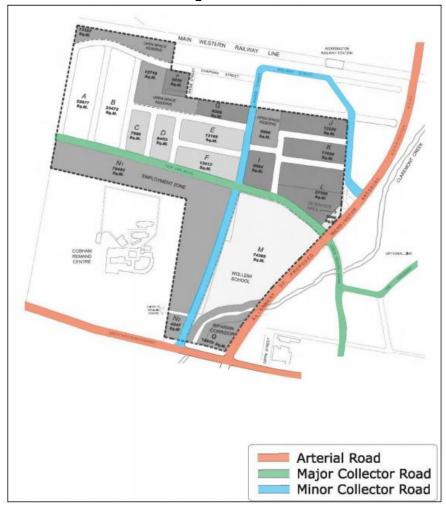


Figure 8-1 Proposed Road Hierarchy

As seen from **Figure 8-1**, the structure plan envisages a major and minor collector road (East-West New Link Road, North-South Road) to run through the subject site. The subdivision layout provided by Lendlease as seen in **Figure 5-1** shows an internal road in the east-west direction which runs in the same alignment envisioned by the structure plan. The North-South road has been segmented into two straights which are linked by the East-West New Link Road.

The indicative sections of East-West New Link Road, North-South Road and Local Road envisioned by the structure plan are illustrated in **Figure 8-2**, **Figure 8-3** and **Figure 8-4** respectively.

The structure plan envisions the width of the East-West link road for the carriageway (including pavement and kerbside lane only) to be 14 m whereas the proposed layout also shows an approximate width of 12 m for the carriageway. The overall road reserve width is unchanged and it is understood that some additional width has been provided to the footpath verges.

The North-South which runs north of the East-West Road is 19.6m wide and has a 12m wide carriageway (including pavement and kerbside lane only) in accordance with the structure plan.



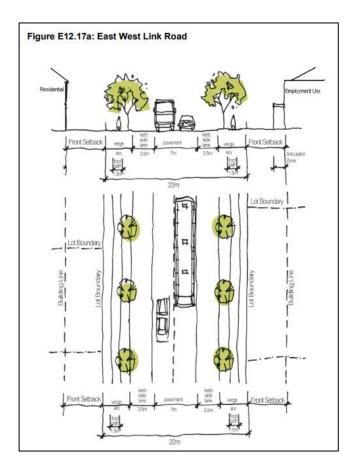


Figure 8-2 Indicative Section East-West Link Road

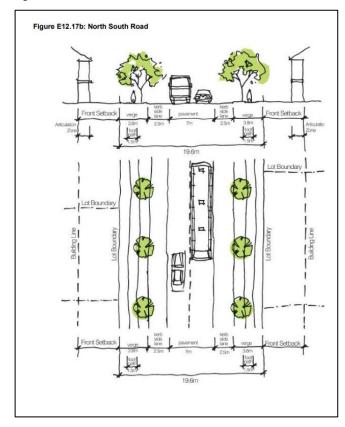


Figure 8-3 Indicative Section North-South Road



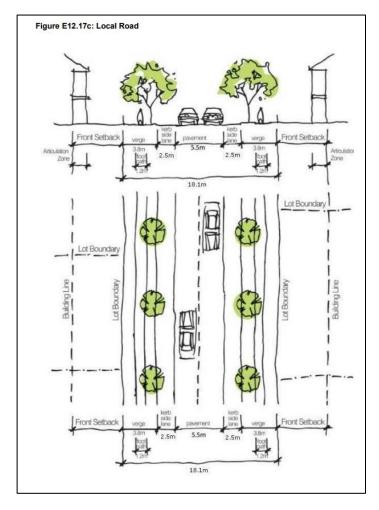


Figure 8-4 Indicative Section Local Road

Cardno recommends providing internal intersection controls where the internal roads intersect (for example New Link Road /Local Road intersections). Also, as the East West New Link Road is aligned as long straights, traffic calming measures like speed humps are recommended to control the speed. Also, pedestrian refuges are recommended to be provided to improve pedestrian permeability.

Stopping Sight Distance (SSD) for the individual lot access driveways is assessed based on the guidelines provided in relevant Australian Standard (AS 2890.1:2004). The SSD determines the minimum sight distance, which should be provided on a major road at an intersection and is dependent on several factors including the major road speed limit of frontage road and type of vehicle traveling on the road.

At the DA stage for each individual dwelling, the lot driveways need to be located and constructed so that there is adequate entering stopping sight distance (SSD), which is most likely adequately offset from corners and bends. It is understood that all corner Lots have been sized so to allow for a driveway to be placed a minimum of 6m from the tangent points of all kerb returns.

#### 8.2 Public transport Accessibility

The public transport proposed by the SWUV structure plan provided within the DCP of Penrith Development Control Plan 2014 is shown in **Figure 8-5**.



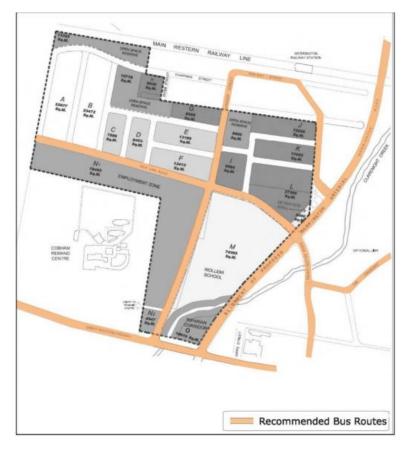


Figure 8-5 Proposed Bus Routes

As seen from **Figure 8-5**, the structure plan envisions bus routes to run along the arterial roads (Great Western Highway and Werrington Road) and to enter the site through the proposed East-West link road and North-South link Roads.

The proposed subdivision layout as seen in **Figure 5-1** shows an internal road in the east-west direction that has the potential to run the bus in the same route as envisioned by the structure plan. The location of the proposed North-South link road is slightly different to the structure plan however affords the the same connectivity through the site.

### 8.3 Pedestrian / Cycle Connectivity

The pedestrian/cycle connectivity proposed by the SWUV structure plan provided within the DCP of Penrith Development Control Plan 2014 is shown in **Figure 8-6**.



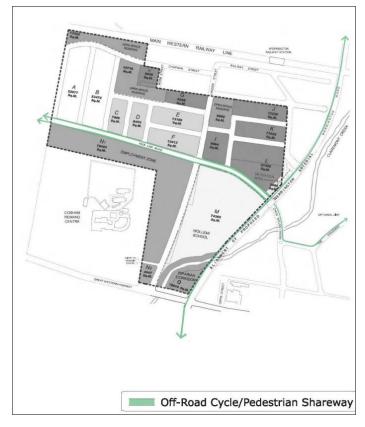


Figure 8-6 Proposed Pedestrian/Cycling Links

As seen from **Figure 8-6**, the structure plans envisions an off-road cycle/pedestrian share way to run along the East-West Road.

It should be noted that DCP requires pedestrian and cycle pathway to have a minimum width of 1.5 m which should be provided along both sides of the road. The subdivision layout at **Figure 5-1** proposes a share way width of 3.6m. This greatly improves the safety of the users of this share way.



## 9 Conclusion

The traffic impact assessment undertaken for the Werrington subdivision results in the following conclusions:

- > Previous traffic reports have shown that:
  - Traffix has previously undertaken a traffic impact assessment for the SWUVP development. The SWUVP Concept Plan envisaged a proposed development of some 2,000 units of high and medium density with a mix of employment and industrial land as well;
  - Cardno's review of the document identified that with no development, Rance Road / Werrington Road performs at LoS B & F during the AM and PM peak hour respectively. The Traffix report states that provision of the new East-West link between the subject site and the future signalised intersection with Werrington Road should be considered a priority for redevelopment of the South Werrington Urban Village as a result of the poor performance identified at Rance Road. The proposed signalised intersection along Werrington Road with the new East-West link (at the south of Rance Road) would provide additional capacity and ensure acceptable level of service intersection performance to cope with future developmental traffic; and
  - Provision of signalised intersection likely to be warranted when some 950 residential units are developed to achieve 200veh/hr on the minor road required under the warrant criteria at the time of the assessment.
- > The Lot yield plan is proposing a development which will ultimately provide for a total of 389 dwellings. This is split up into 265 Lots, 91 future Lots and 33 future apartments. The development also includes the creation of 14 industrial lots comprising 31,005 m<sup>2</sup> gross land area. The development scale is considerably lower when compared to the yield highlighted in Traffix's report;
- > The complete development is expected to generate 317 and 330 vehicular trips per hour in the AM and PM peaks respectively;
- > A 2% per annum background traffic growth has been added across Werrington Road for the purpose of this assessment to capture significant growth from the SWUV and other growth precincts nearby;
- > Trip rate of 0.4 trips per dwellings was adopted for the reasons specified in this assessment as well as Mott Macdonald's report. This considers the sites locality to the Werrington railway station;
- > Stage 1a and Stage 1b are planned to be completed by the end of 2022 with the remaining stages being completed by the end of year 2029;
- Vehicular access into and out of the proposed development will be through the existing connections to the development via Walker Street/Chapman Street and a new link road (East-West link), which will connect to Werrington Road;
- > Cardno adopted the shortest distance calculation approach in order to derive the number of lots that are expected to use the two site accesses (new East-West link and Rance Road):
  - It is assumed that traffic from development stages 1a and 1c will divert to Chapman Street / Railway
     Street / Rance Road; and
  - The remaining development stages (1b, 2a, 2b, 3, 4a and 4b) are expected to use the new East-West link upon construction completion.
- The TIA assessed a roundabout design using SIDRA in both 2022 and 2029 with the following specifications:
  - 3-approach roundabout conversion at Rance Road / Werrington Road by 2022 (Stage 1a and 1b);
  - 3-approach roundabout conversion at Rance Road / Werrington Road by 2029 (Stage 1-3); and
  - 4-approach roundabout conversion tying in the East-West link to the west by 2029 (Stage 1-4).
- > SIDRA results show the intersection of Rance Road / Werrington Road is currently (2019) operating at LoS B and C in the AM and PM peaks respectively;
- > The 2022 and 2029 base case SIDRA models showed unsatisfactory intersection performance with LoS E and F in the PM peak at the respective years;



- > The implementation of the three-legged roundabout design in 2022 will improve performance to LoS B in the PM peak when compared to the base case.
- In 2029 the three-legged and four-legged roundabout will operate at LoS E and F respectively in the PM peak. However, both scenarios show significant delay improvements when compared to the 2029 base case;
- It is important to state that the development of the Werrington site is part of the wider SWUV precinct development. Under the SWUV, it was intended that the East-West link intersection with Werrington Road would be signalised and as such providing additional capacity, making the roundabout design and corresponding LoS performance redundant.
- Similarly, the SWUV structure plan relies upon the delivery of the Werrington Arterial Stage 2 project. The performance of the intersection of the East West links, Werrington Road and Rance Road will ultimately improve once the capacity of Werrington road increases per the planned Werrington Arterial Road Stage 2 upgrade.

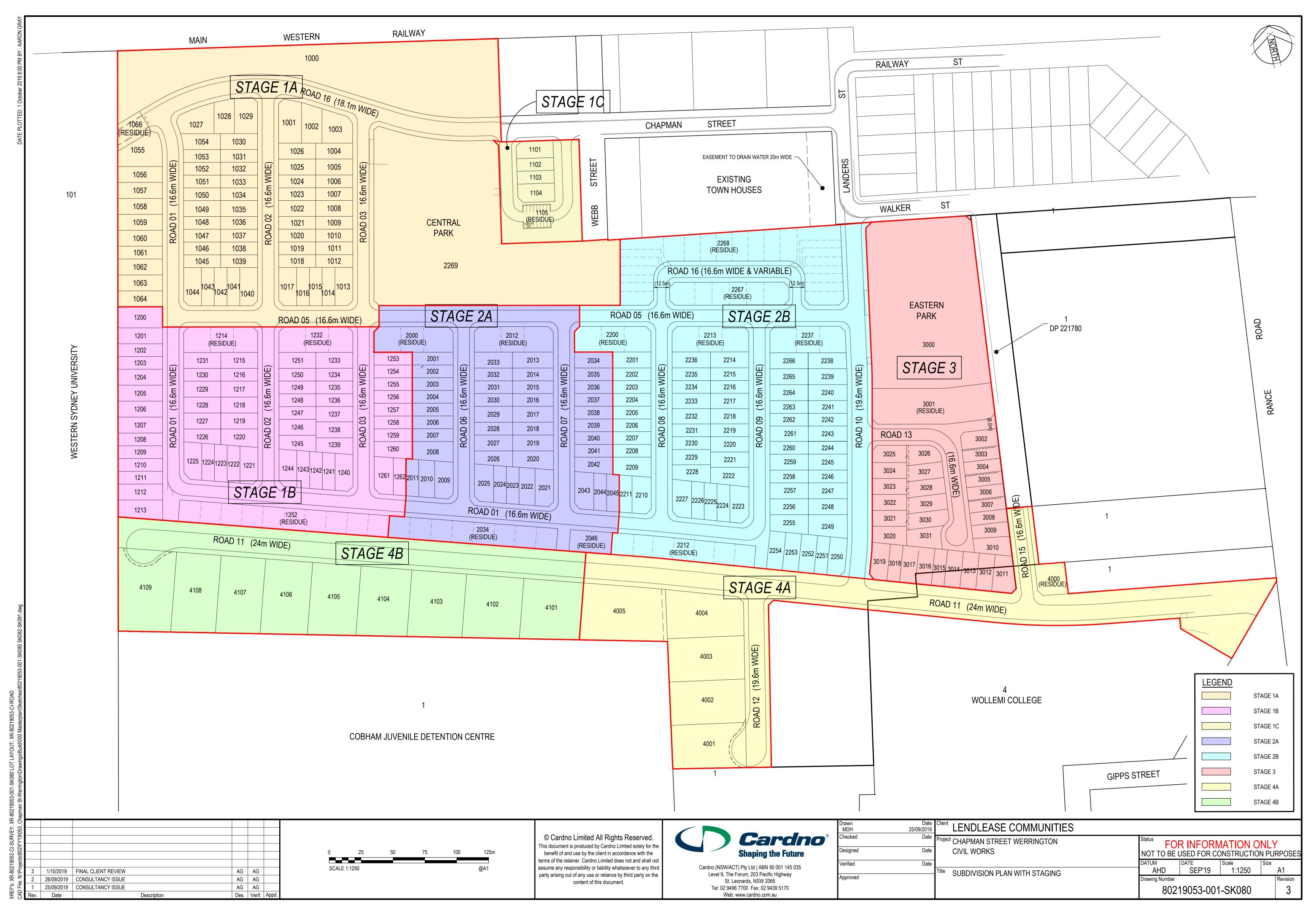
**APPENDIX** 



**DEVELOPMENT LAYOUT** 



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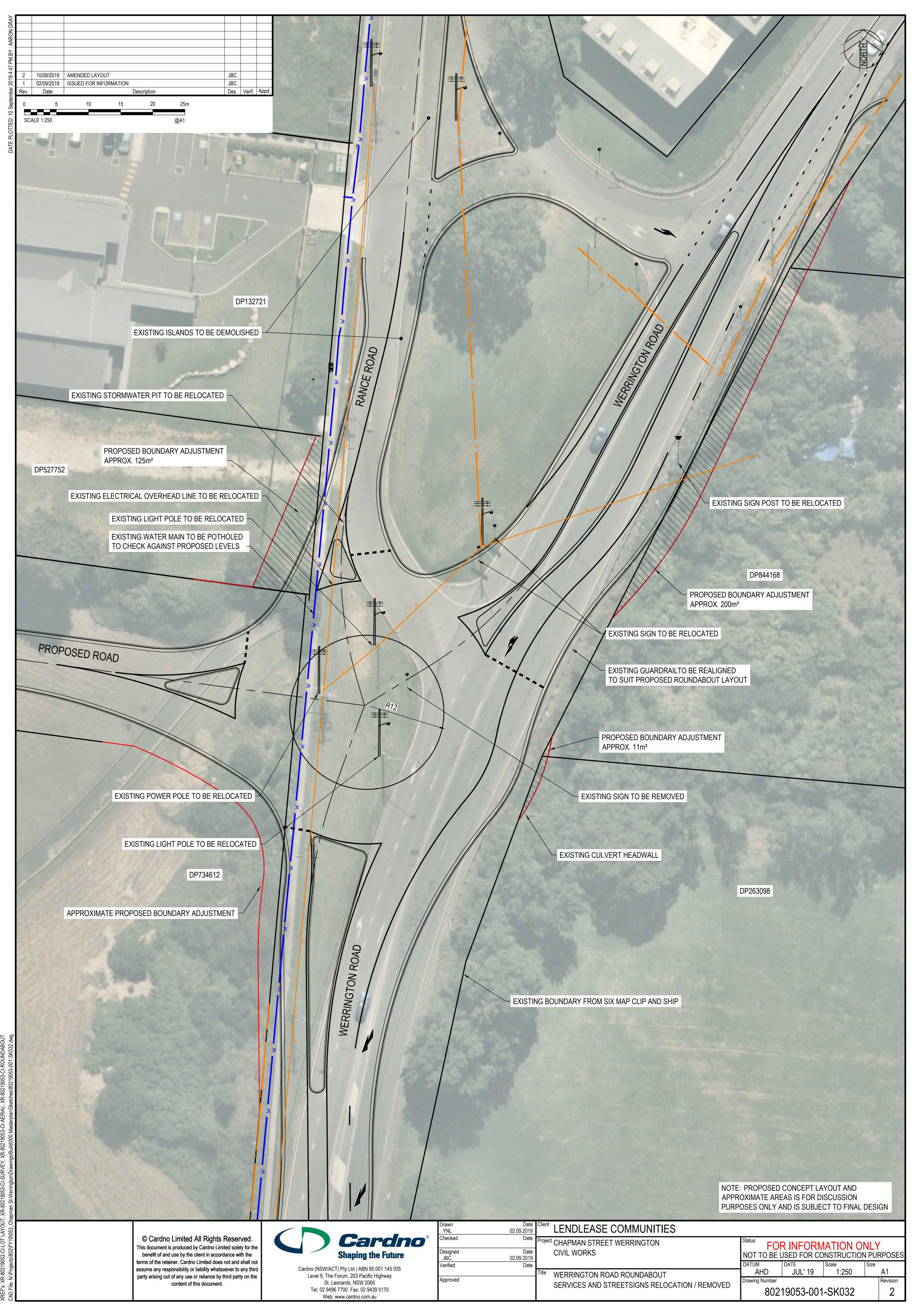


**APPENDIX** 

B

RANCE ROAD/WERRINGTON ROAD INTERSECTION DESIGN





APPENDIX

C

SIDRA SUMMARY





Site: 101 [Site1 - Rance Rd 2019 AM (Base)-inbound]

ф Network: N101 [Base 2019 -AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ince -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	79	0.0	79	0.0	0.043	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	630	4.1	630	4.1	0.332	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	709	3.7	709	3.7	0.332	0.6	NA	0.0	0.0	0.00	0.06	0.00	57.1
North	: Werrir	ngton Rd (N	N)											
8	T1	881	2.6	881	2.6	0.403	0.5	LOSA	0.3	2.0	0.07	0.02	0.10	57.3
9	R2	21	0.0	21	0.0	0.403	11.6	LOSA	0.3	2.0	0.09	0.02	0.12	47.8
Appro	oach	902	2.5	902	2.5	0.403	0.7	NA	0.3	2.0	0.07	0.02	0.10	56.8
All Ve	hicles	1611	3.0	1611	3.0	0.403	0.7	NA	0.3	2.0	0.04	0.04	0.05	56.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site1 - Rance Rd 2019 AM (Base)-outbound]

ф Network: N101 [Base 2019 -AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles		Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	630	4.1	630	4.1	0.332	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	630	4.1	630	4.1	0.332	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
North	: Werrir	ngton Rd (I	N)											
8	T1	840	2.7	840	2.7	0.438	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	840	2.7	840	2.7	0.438	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.9
West:	Rance	Rd												
10	L2	20	5.0	20	5.0	0.025	7.5	LOSA	0.0	0.3	0.54	0.68	0.54	47.7
12	R2	62	0.0	62	0.0	0.228	16.6	LOS B	0.3	1.8	0.83	0.95	0.91	34.4
Appro	ach	82	1.2	82	1.2	0.228	14.4	LOSA	0.3	1.8	0.76	0.89	0.82	38.6
All Ve	hicles	1552	3.2	1552	3.2	0.438	0.8	NA	0.3	1.8	0.04	0.05	0.04	57.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site1 - Rance Rd 2019 PM (Base)-inbound]

ф Network: N101 [Base 2019 -PM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ınce -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	58	0.0	58	0.0	0.031	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	1189	8.0	1189	8.0	0.613	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Appro	oach	1247	0.7	1247	0.7	0.613	0.3	NA	0.0	0.0	0.00	0.03	0.00	58.4
North	: Werrir	ngton Rd (N	۷)											
8	T1	699	1.0	699	1.0	0.360	3.3	LOSA	0.9	6.4	0.16	0.02	0.22	48.1
9	R2	16	0.0	16	0.0	0.360	31.1	LOS C	0.9	6.4	0.21	0.02	0.28	43.6
Appro	oach	715	1.0	715	1.0	0.360	3.9	NA	0.9	6.4	0.16	0.02	0.22	47.8
All Ve	hicles	1962	0.8	1962	8.0	0.613	1.6	NA	0.9	6.4	0.06	0.02	0.08	53.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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igvee Site: 101 [Site1 - Rance Rd 2019 PM (Base)-outbound]

ф Network: N101 [Base 2019 -PM Peak1

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ınce -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles		Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	1189	8.0	1189	0.8	0.613	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach	1189	8.0	1189	8.0	0.613	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.7
North	: Werrir	ngton Rd (N	۷)											
8	T1	651	1.1	651	1.1	0.336	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	651	1.1	651	1.1	0.336	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
West:	Rance	Rd												
10	L2	28	3.6	28	3.6	0.107	17.5	LOS B	0.1	0.9	0.85	0.93	0.85	42.3
12	R2	64	0.0	64	0.0	0.532	40.4	LOS C	0.6	4.2	0.95	1.05	1.24	23.8
Appro	ach	92	1.1	92	1.1	0.532	33.4	LOS C	0.6	4.2	0.92	1.01	1.12	29.9
All Ve	hicles	1932	0.9	1932	0.9	0.613	1.6	NA	0.6	4.2	0.04	0.05	0.05	56.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Site1 - Rance Rd 2022 AM (Base)-inbound]

ф Network: N101 [Base 2022 -AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	79	0.0	79	0.0	0.043	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	668	4.2	668	4.2	0.352	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	747	3.7	747	3.7	0.352	0.6	NA	0.0	0.0	0.00	0.06	0.00	57.2
North	ı: Werrir	ngton Rd (I	N)											
8	T1	868	2.8	868	2.8	0.399	0.5	LOS A	0.3	2.2	0.08	0.02	0.10	57.0
9	R2	21	0.0	21	0.0	0.399	12.3	LOSA	0.3	2.2	0.09	0.02	0.13	47.7
Appro	oach	889	2.7	889	2.7	0.399	8.0	NA	0.3	2.2	0.08	0.02	0.10	56.5
All Ve	ehicles	1636	3.2	1636	3.2	0.399	0.7	NA	0.3	2.2	0.04	0.04	0.06	56.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site1 - Rance Rd 2022 AM (Base)-outbound]

ф Network: N101 [Base 2022 -AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles		Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	668	4.2	668	4.2	0.352	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	668	4.2	668	4.2	0.352	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
North	: Werrir	ngton Rd (I	N)											
8	T1	889	2.7	889	2.7	0.464	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	889	2.7	889	2.7	0.464	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.9
West:	Rance	Rd												
10	L2	20	5.0	20	5.0	0.027	7.9	LOSA	0.0	0.3	0.55	0.70	0.55	47.5
12	R2	62	0.0	62	0.0	0.265	19.2	LOS B	0.3	2.1	0.86	0.97	0.97	32.8
Appro	ach	82	1.2	82	1.2	0.265	16.4	LOS B	0.3	2.1	0.78	0.91	0.87	37.3
All Ve	hicles	1639	3.2	1639	3.2	0.464	0.9	NA	0.3	2.1	0.04	0.05	0.04	57.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Site1 - Rance Rd 2022 PM (Base)-inbound]

ф Network: N101 [Base 2022 -PM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	58	0.0	58	0.0	0.031	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	1261	0.8	1261	8.0	0.650	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Appro	oach	1319	8.0	1319	0.8	0.650	0.3	NA	0.0	0.0	0.00	0.03	0.00	58.4
North	: Werrir	ngton Rd (I	N)											
8	T1	673	1.0	673	1.0	0.364	4.9	LOSA	1.2	8.6	0.19	0.02	0.26	44.2
9	R2	16	0.0	16	0.0	0.364	37.3	LOS C	1.2	8.6	0.25	0.02	0.34	41.5
Appro	oach	689	1.0	689	1.0	0.364	5.6	NA	1.2	8.6	0.19	0.02	0.26	44.1
All Ve	ehicles	2008	0.8	2008	0.8	0.650	2.1	NA	1.2	8.6	0.07	0.02	0.09	52.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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igvee Site: 101 [Site1 - Rance Rd 2022 PM (Base)-outbound]

ф Network: N101 [Base 2022 -PM Peak1

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles		Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	1261	8.0	1261	0.8	0.650	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach	1261	8.0	1261	8.0	0.650	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.7
North	: Werrir	ngton Rd (I	N)											
8	T1	689	1.0	689	1.0	0.356	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	689	1.0	689	1.0	0.356	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
West:	Rance	Rd												
10	L2	28	3.6	28	3.6	0.133	20.9	LOS B	0.2	1.1	0.88	0.95	0.88	40.7
12	R2	64	0.0	64	0.0	0.711	64.8	LOS E	8.0	5.9	0.98	1.11	1.46	18.1
Appro	ach	92	1.1	92	1.1	0.711	51.5	LOS D	0.8	5.9	0.95	1.06	1.28	24.4
All Ve	hicles	2042	0.9	2042	0.9	0.711	2.3	NA	0.8	5.9	0.04	0.05	0.06	55.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Site1 - Rance Rd 2022 AM (Base + Dev)-inbound]

 Physical Physi 2022 - AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	88	0.0	88	0.0	0.047	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	668	4.2	668	4.2	0.352	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	756	3.7	756	3.7	0.352	0.7	NA	0.0	0.0	0.00	0.07	0.00	57.0
North	: Werrir	ngton Rd (I	N)											
8	T1	868	2.8	868	2.8	0.404	0.6	LOSA	0.4	2.6	0.09	0.02	0.12	56.6
9	R2	24	0.0	24	0.0	0.404	12.5	LOSA	0.4	2.6	0.11	0.02	0.15	47.5
Appro	oach	892	2.7	892	2.7	0.404	0.9	NA	0.4	2.6	0.09	0.02	0.12	56.0
All Ve	hicles	1648	3.2	1648	3.2	0.404	0.8	NA	0.4	2.6	0.05	0.04	0.07	56.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site1 - Rance Rd 2022 AM (Base + Dev)-outbound]

♦♦ Network: N101 [Base + Dev 2022 - AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ince -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	668	4.2	668	4.2	0.352	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	668	4.2	668	4.2	0.352	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
North	: Werrir	ngton Rd (N	N)											
8	T1	891	2.7	891	2.7	0.465	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	891	2.7	891	2.7	0.465	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.9
West:	Rance	Rd												
10	L2	31	3.2	31	3.2	0.041	7.8	LOSA	0.1	0.4	0.55	0.72	0.55	47.6
12	R2	97	0.0	97	0.0	0.417	21.3	LOS B	0.5	3.5	0.89	1.02	1.12	31.7
Appro	ach	128	8.0	128	8.0	0.417	18.0	LOS B	0.5	3.5	0.81	0.94	0.99	36.4
All Ve	hicles	1687	3.1	1687	3.1	0.465	1.4	NA	0.5	3.5	0.06	0.07	0.07	56.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Site1 - Rance Rd 2022 PM (Base + Dev)-inbound]

 Physical Physi 2022 - PM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ince -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	9
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	94	0.0	94	0.0	0.051	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	1261	8.0	1261	8.0	0.650	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.7
Appro	oach	1355	0.7	1355	0.7	0.650	0.4	NA	0.0	0.0	0.00	0.04	0.00	57.8
North	: Werrir	ngton Rd (N	N)											
8	T1	673	1.0	673	1.0	0.422	8.1	LOS A	4.6	32.2	0.72	0.04	0.85	37.8
9	R2	26	0.0	26	0.0	0.422	39.5	LOS C	4.6	32.2	1.00	0.05	1.17	37.4
Appro	oach	699	1.0	699	1.0	0.422	9.3	NA	4.6	32.2	0.73	0.04	0.86	37.8
All Ve	ehicles	2054	0.8	2054	8.0	0.650	3.5	NA	4.6	32.2	0.25	0.04	0.29	48.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site1 - Rance Rd 2022 PM (Base + Dev)-outbound]

♦♦ Network: N101 [Base + Dev

2022 - PM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	1261	8.0	1261	0.8	0.650	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach	1261	8.0	1261	8.0	0.650	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.7
North	: Werrir	ngton Rd (I	N)											
8	T1	699	1.0	699	1.0	0.458	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.8
Appro	ach	699	1.0	699	1.0	0.458	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.8
West	Rance	Rd												
10	L2	32	3.1	32	3.1	0.151	20.9	LOS B	0.2	1.3	0.89	0.95	0.89	40.7
12	R2	72	0.0	72	0.0	1.036	160.0	LOS F	2.2	15.1	1.00	1.62	3.54	9.3
Appro	ach	104	1.0	104	1.0	1.036	117.2	LOS F	2.2	15.1	0.96	1.41	2.73	14.5
All Ve	hicles	2064	0.9	2064	0.9	1.036	5.9	NA	2.2	15.1	0.05	0.07	0.14	50.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**▽** Site: 101 [Site 3 - Rance Rd 2022 AM]

♦♦ Network: N101 [Base + Dev 2022 - AM Peak (Mitigation)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	East: V	Verrington	Rd (N)											
8	T1	938	2.7	938	2.7	0.489	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.8
Appro	oach	938	2.7	938	2.7	0.489	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.8
North	West: F	Rance Rd												
10	L2	33	3.2	33	3.2	0.045	8.1	LOSA	0.1	0.5	0.57	0.74	0.57	47.4
Appro	oach	33	3.2	33	3.2	0.045	8.1	LOSA	0.1	0.5	0.57	0.74	0.57	47.4
South	اWest: ۱	Werrington	Rd (S	)										
2	T1	703	4.2	703	4.2	0.370	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	703	4.2	703	4.2	0.370	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
All Ve	hicles	1674	3.3	1674	3.3	0.489	0.2	NA	0.1	0.5	0.01	0.01	0.01	59.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: \\cardno.corp\global\AU\NSW\DirectoryStructure\Projects\899\FY14\069\_ENGINEERING FEASIBILITY LEND LEASE COMMUNITIES

\015\_WERRINGTON\Des-An\Traffic\Werrington - Rance Access Only v02-SS.sip8



₩ Site: 102 [Site 2 - EW Connection 2022 AM]

 Physical Physi 2022 - AM Peak (Mitigation)]

Site 2 - EW Connection 2022 AM Site Category: (None) Roundabout

Move	ement	Performa	ance -	Vehic	les									
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rate		km/h
South	n: Werri	ngton Rd (			,,	.,.								1111/
2	T1	93	0.0	93	0.0	0.495	4.3	LOS A	1.8	12.7	0.18	0.56	0.18	50.3
3a	R1	703	4.2	703	4.2	0.495	7.9	LOS A	1.8	12.7	0.18	0.56	0.18	48.0
Appro	oach	796	3.7	796	3.7	0.495	7.5	LOSA	1.8	12.7	0.18	0.56	0.18	48.5
North	East: V	Verrington	Rd (N)											
24a	L1	914	2.8	914	2.8	0.455	3.6	LOS A	1.6	11.4	0.37	0.44	0.37	53.3
26b	R3	25	0.0	25	0.0	0.455	9.6	LOS A	1.6	11.4	0.38	0.44	0.38	49.3
Appro	oach	939	2.7	939	2.7	0.455	3.8	LOSA	1.6	11.4	0.37	0.44	0.37	53.2
North	: Rance	e Rd												
8	T1	102	0.0	102	0.0	0.123	6.9	LOSA	0.3	2.0	0.69	0.66	0.69	50.1
Appro	oach	102	0.0	102	0.0	0.123	6.9	LOSA	0.3	2.0	0.69	0.66	0.69	50.1
All Ve	hicles	1837	3.0	1837	3.0	0.495	5.6	LOSA	1.8	12.7	0.30	0.50	0.30	50.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**▽** Site: 101 [Site 3 - Rance Rd 2022 PM]

♦♦ Network: N101 [Base + Dev 2022 - PM Peak (Mitigation)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	East: V	Verrington	Rd (N)											
8	T1	736	1.0	736	1.0	0.380	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	736	1.0	736	1.0	0.380	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.9
North	West: F	Rance Rd												
10	L2	34	3.1	34	3.1	0.200	26.3	LOS B	0.2	1.7	0.91	0.97	0.96	38.4
Appro	ach	34	3.1	34	3.1	0.200	26.3	LOS B	0.2	1.7	0.91	0.97	0.96	38.4
South	West: \	Werrington	Rd (S	)										
2	T1	1327	8.0	1327	0.8	0.684	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.7
Appro	ach	1327	8.0	1327	8.0	0.684	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.7
All Ve	hicles	2097	0.9	2097	0.9	0.684	0.5	NA	0.2	1.7	0.01	0.02	0.02	58.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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\015\_WERRINGTON\Des-An\Traffic\Werrington - Rance Access Only v02-SS.sip8



₩ Site: 102 [Site 2 - EW Connection 2022 PM]

 Physical Physi 2022 - PM Peak (Mitigation)]

Site 2 - EW Connection 2022 AM Site Category: (None) Roundabout

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total				Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ı: Werri	ngton Rd (	(S)											
2	T1	99	0.0	99	0.0	0.860	4.7	LOS A	8.8	61.8	0.44	0.49	0.44	49.4
3a	R1	1327	0.8	1327	8.0	0.860	8.3	LOS A	8.8	61.8	0.44	0.49	0.44	46.5
Appro	oach	1426	0.7	1426	0.7	0.860	8.0	LOSA	8.8	61.8	0.44	0.49	0.44	46.9
North	East: V	Verrington	Rd (N)											
24a	L1	708	1.0	708	1.0	0.349	3.4	LOSA	1.2	8.4	0.31	0.42	0.31	53.7
26b	R3	27	0.0	27	0.0	0.349	9.4	LOSA	1.2	8.4	0.32	0.42	0.32	49.6
Appro	ach	736	1.0	736	1.0	0.349	3.6	LOSA	1.2	8.4	0.31	0.42	0.31	53.5
North	: Rance	e Rd												
8	T1	76	0.0	76	0.0	0.222	20.2	LOS B	0.7	4.8	1.00	0.95	1.00	42.5
Appro	ach	76	0.0	76	0.0	0.222	20.2	LOS B	0.7	4.8	1.00	0.95	1.00	42.5
All Ve	hicles	2238	0.8	2238	8.0	0.860	7.0	LOS A	8.8	61.8	0.42	0.48	0.42	48.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Site1 - Rance Rd 2029 AM (Base)-inbound]

ф Network: N101 [Base 2029 -AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ince -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	79	0.0	79	0.0	0.043	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	756	4.1	756	4.1	0.398	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	835	3.7	835	3.7	0.398	0.5	NA	0.0	0.0	0.00	0.05	0.00	57.4
North	: Werrir	ngton Rd (N	N)											
8	T1	983	2.8	983	2.8	0.453	0.7	LOSA	0.4	2.9	0.09	0.01	0.12	56.3
9	R2	21	0.0	21	0.0	0.453	15.4	LOS B	0.4	2.9	0.11	0.02	0.15	47.4
Appro	oach	1004	2.8	1004	2.8	0.453	1.0	NA	0.4	2.9	0.09	0.01	0.12	55.9
All Ve	hicles	1839	3.2	1839	3.2	0.453	8.0	NA	0.4	2.9	0.05	0.03	0.07	56.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site1 - Rance Rd 2029 AM (Base)-outbound]

ф Network: N101 [Base 2029 -AM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ı: Werri	ngton Rd (	S)											
2	T1	756	4.1	756	4.1	0.398	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	756	4.1	756	4.1	0.398	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
North	: Werrir	ngton Rd (I	N)											
8	T1	1004	2.8	1004	2.8	0.524	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.8
Appro	ach	1004	2.8	1004	2.8	0.524	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.8
West	Rance	Rd												
10	L2	20	5.0	20	5.0	0.031	8.7	LOSA	0.0	0.3	0.60	0.75	0.60	47.0
12	R2	62	0.0	62	0.0	0.394	28.9	LOS C	0.4	3.1	0.92	1.02	1.11	28.0
Appro	ach	82	1.2	82	1.2	0.394	23.9	LOS B	0.4	3.1	0.84	0.95	0.99	33.3
All Ve	hicles	1842	3.3	1842	3.3	0.524	1.1	NA	0.4	3.1	0.04	0.04	0.04	57.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 101 [Site1 - Rance Rd 2029 PM (Base)-inbound]

ф Network: N101 [Base 2029 -PM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total		Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective / Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	58	0.0	58	0.0	0.031	5.5	LOSA	0.0	0.0	0.00	0.58	0.00	51.3
2	T1	1427	8.0	1427	8.0	0.735	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.6
Appro	oach	1485	0.7	1485	0.7	0.735	0.3	NA	0.0	0.0	0.00	0.02	0.00	58.4
North	: Werrir	ngton Rd (	N)											
8	T1	762	1.0	725	1.1	0.445	12.3	LOSA	7.1	50.0	0.73	0.02	0.85	32.4
9	R2	16	0.0	15	0.0	0.445	65.7	LOS E	7.1	50.0	1.00	0.03	1.16	33.9
Appro	oach	778	1.0	<mark>740</mark> <sup>N</sup>	1.1	0.445	13.4	NA	7.1	50.0	0.74	0.02	0.85	32.4
All Ve	hicles	2263	0.8	2225 <sup>N</sup>	0.9	0.735	4.7	NA	7.1	50.0	0.24	0.02	0.28	45.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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V Site: 101 [Site1 - Rance Rd 2029 PM (Base)-outbound]

ф Network: N101 [Base 2029 -PM Peak]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles		Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Werri	ngton Rd (	S)											
2	T1	1427	8.0	1427	0.8	0.735	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.6
Appro	ach	1427	8.0	1427	8.0	0.735	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.6
North	: Werrir	ngton Rd (I	N)											
8	T1	778	1.0	778	1.0	0.402	0.1	LOSA	0.3	1.9	0.00	0.00	0.00	59.9
Appro	ach	778	1.0	778	1.0	0.402	0.1	NA	0.3	1.9	0.00	0.00	0.00	59.9
West:	Rance	Rd												
10	L2	28	3.6	28	3.6	0.249	38.6	LOS C	0.3	2.1	0.95	0.99	1.02	34.0
12	R2	64	0.0	64	0.0	3.193	2037.2	LOS F	12.5	87.5	1.00	3.11	10.02	0.9
Appro	ach	92	1.1	92	1.1	3.193	1428.9	LOS F	12.5	87.5	0.98	2.47	7.28	1.6
All Ve	hicles	2297	0.9	2297	0.9	3.193	57.3	NA	12.5	87.5	0.04	0.10	0.29	21.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 102 [Site 2 - EW Connection 2029 AM (Stage 1-3)]

♦♦ Network: N101 [Base + Dev (Stage 1-3) 2029 - AM Peak (Mitigation) ]

Site 2 - EW Connection 2022 AM Site Category: (None) Roundabout

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arriva Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	(S)											
2	T1	108	0.0	108	0.0	0.566	4.4	LOS A	2.4	17.1	0.23	0.55	0.23	50.2
3a	R1	796	4.1	796	4.1	0.566	8.0	LOS A	2.4	17.1	0.23	0.55	0.23	47.8
Appro	oach	904	3.6	904	3.6	0.566	7.6	LOSA	2.4	17.1	0.23	0.55	0.23	48.3
North	East: V	/errington	Rd (N)											
24a	L1	1035	2.8	1035	2.8	0.553	4.1	LOS A	2.1	15.4	0.52	0.51	0.52	52.4
26b	R3	29	0.0	29	0.0	0.553	10.0	LOS A	2.1	15.4	0.54	0.51	0.54	48.5
Appro	oach	1064	2.8	1064	2.8	0.553	4.2	LOSA	2.1	15.4	0.52	0.51	0.52	52.3
North	: Rance	e Rd												
8	T1	164	0.0	164	0.0	0.219	8.1	LOSA	0.6	3.9	0.77	0.76	0.77	49.3
Appro	oach	164	0.0	164	0.0	0.219	8.1	LOSA	0.6	3.9	0.77	0.76	0.77	49.3
All Ve	hicles	2133	2.9	2133	2.9	0.566	5.9	LOSA	2.4	17.1	0.42	0.54	0.42	50.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site 3 - Rance Rd 2029 AM (Stage 1-3)]

 Physical Physi (Stage 1-3) 2029 - AM Peak (Mitigation) ]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	0
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	East: V	Verrington	Rd (N)											
8	T1	1064	2.8	1064	2.8	0.556	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.8
Appro	ach	1064	2.8	1064	2.8	0.556	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.8
North'	West: F	Rance Rd												
10	L2	53	2.0	53	2.0	0.084	9.1	LOSA	0.1	8.0	0.64	0.83	0.64	46.8
Appro	ach	53	2.0	53	2.0	0.084	9.1	LOSA	0.1	8.0	0.64	0.83	0.64	46.8
South	West: \	Werrington	Rd (S	)										
2	T1	796	4.1	796	4.1	0.419	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	796	4.1	796	4.1	0.419	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
All Ve	hicles	1913	3.3	1913	3.3	0.556	0.3	NA	0.1	0.8	0.02	0.02	0.02	59.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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\015\_WERRINGTON\Des-An\Traffic\Werrington - Rance Access Only v02-SS.sip8



₩ Site: 102 [Site 2 - EW Connection 2029 PM (Stage 1-3)]

♦♦ Network: N101 [Base + Dev (Stage 1-3) 2029 - PM Peak (Mitigation)]

Site 2 - EW Connection 2022 AM Site Category: (None)

Roundabout

Move	omont	Dorforma	nnoo	Vohio	loo									
		Performa												
Mov	Turn	Demand				Deg.	Average	Level of	Aver. Back			Effective A		
ID		Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	peed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
2	T1	157	0.0	157	0.0	1.026	33.9	LOS C	55.8	393.1	1.00	0.53	1.29	36.2
3a	R1	1502	8.0	1502	8.0	1.026	37.5	LOS C	55.8	393.1	1.00	0.53	1.29	28.0
Appro	oach	1659	0.7	1659	0.7	1.026	37.1	LOS C	55.8	393.1	1.00	0.53	1.29	29.1
North	East: V	Verrington I	Rd (N)											
24a	L1	802	1.0	802	1.0	0.407	3.5	LOS A	1.5	10.4	0.35	0.44	0.35	53.3
26b	R3	43	0.0	43	0.0	0.407	9.5	LOSA	1.5	10.4	0.36	0.44	0.36	49.2
Appro	oach	845	1.0	845	1.0	0.407	3.8	LOSA	1.5	10.4	0.35	0.44	0.35	53.1
North	: Rance	e Rd												
8	T1	88	0.0	88	0.0	0.576	60.9	LOS E	2.1	14.7	1.00	1.17	1.41	29.0
Appro	oach	88	0.0	88	0.0	0.576	60.9	LOS E	2.1	14.7	1.00	1.17	1.41	29.0
All Ve	ehicles	2593	8.0	2593	8.0	1.026	27.1	LOS B	55.8	393.1	0.79	0.52	0.99	34.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [Site 3 - Rance Rd 2029 PM (Stage 1-3)]

♦♦ Network: N101 [Base + Dev (Stage 1-3) 2029 - PM Peak (Mitigation)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	Vehicl	es									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	East: V	Verrington	Rd (N)											
8	T1	845	1.0	845	1.0	0.436	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	845	1.0	845	1.0	0.436	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.9
North'	West: F	Rance Rd												
10	L2	39	2.7	39	2.7	0.404	50.4	LOS D	0.5	3.4	0.96	1.02	1.12	30.6
Appro	ach	39	2.7	39	2.7	0.404	50.4	LOS D	0.5	3.4	0.96	1.02	1.12	30.6
South	اWest: ۱	Werrington	Rd (S	)										
2	T1	1502	8.0	1465	8.0	0.755	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.5
Appro	ach	1502	8.0	1465 <sup>N</sup>	0.8	0.755	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.5
All Ve	hicles	2386	0.9	2349 <sup>N</sup>	0.9	0.755	0.9	NA	0.5	3.4	0.02	0.02	0.02	58.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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**∀** Site: 101 [Site 2 - EW Connection 2029 AM]

 Physical Physi 2029 - AM Peak (Mitigation)]

Site 2 - EW Connection 2022 AM Site Category: (None) Roundabout

Mov	ement	Performa	ınce -	Vehic	les									
Mov	Turn	Demand	Flows	Arrival	Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	ver. No.A	verage
ID		Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	Speed
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
South	n: Werri	ngton Rd (	S)											
1	L2	88	0.0	88	0.0	0.630	4.3	LOS A	3.0	21.3	0.34	0.53	0.34	48.8
2	T1	89	0.0	89	0.0	0.630	4.6	LOS A	3.0	21.3	0.34	0.53	0.34	50.0
3a	R1	796	4.1	796	4.1	0.630	8.2	LOS A	3.0	21.3	0.34	0.53	0.34	47.5
Appro	oach	974	3.4	974	3.4	0.630	7.5	LOSA	3.0	21.3	0.34	0.53	0.34	48.0
North	East: V	Verrington I	Rd (N)											
24a	L1	1035	2.8	1035	2.8	0.589	4.5	LOS A	2.2	15.7	0.59	0.56	0.59	52.0
26a	R1	23	0.0	23	0.0	0.589	8.3	LOSA	2.2	15.7	0.62	0.57	0.62	46.4
26b	R3	23	0.0	23	0.0	0.589	10.4	LOS A	2.2	15.7	0.62	0.57	0.62	48.1
Appro	oach	1081	2.7	1081	2.7	0.589	4.7	LOSA	2.2	15.7	0.59	0.56	0.59	51.7
North	: Rance	e Rd												
8	T1	86	0.0	86	0.0	0.144	10.4	LOSA	0.4	2.7	0.84	0.79	0.84	47.8
9	R2	1	0.0	1	0.0	0.144	14.9	LOS B	0.4	2.7	0.84	0.79	0.84	45.2
Appro	oach	87	0.0	87	0.0	0.144	10.4	LOSA	0.4	2.7	0.84	0.79	0.84	47.8
West	: EW Li	nk												
10	L2	1	0.0	1	0.0	0.292	9.8	LOS A	0.8	5.7	0.86	0.88	0.86	42.7
10a	L1	46	0.0	46	0.0	0.292	9.5	LOS A	0.8	5.7	0.86	0.88	0.86	37.5
12	R2	142	0.0	142	0.0	0.292	14.3	LOS A	0.8	5.7	0.86	0.88	0.86	46.3
Appro	oach	189	0.0	189	0.0	0.292	13.1	LOSA	0.8	5.7	0.86	0.88	0.86	44.7
All Ve	ehicles	2332	2.7	2332	2.7	0.630	6.8	LOSA	3.0	21.3	0.52	0.59	0.52	49.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**▽** Site: 101 [Site 3 - Rance Rd 2029 AM]

♦♦ Network: N101 [Base + Dev 2029 - AM Peak (Mitigation)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	East: V	Verrington	Rd (N)											
8	T1	1081	2.7	1081	2.7	0.564	0.1	LOSA	0.0	0.0	0.00	0.00	0.00	59.8
Appro	ach	1081	2.7	1081	2.7	0.564	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.8
North	West: F	Rance Rd												
10	L2	28	3.7	28	3.7	0.050	9.7	LOSA	0.1	0.5	0.66	0.83	0.66	46.5
Appro	ach	28	3.7	28	3.7	0.050	9.7	LOSA	0.1	0.5	0.66	0.83	0.66	46.5
South	اWest: ۱	Werrington	Rd (S	)										
2	T1	842	3.9	842	3.9	0.443	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	ach	842	3.9	842	3.9	0.443	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
All Ve	hicles	1952	3.2	1952	3.2	0.564	0.2	NA	0.1	0.5	0.01	0.01	0.01	59.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [Site 2 - EW Connection 2029 PM]

 Physical Physi 2029 - PM Peak (Mitigation)]

Site 2 - EW Connection 2022 AM Site Category: (None) Roundabout

Movement Performance - Vehicles														
Mov	Turn	Demand	Flows	Arriva	l Flows	Deg.	Average	Level of	Aver. Back	of Queue	Prop.	Effective A	Aver. No.A	verage
ID		Total	HV	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles S	Speed
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
South		ngton Rd (	` '											
1	L2	153	0.0	153	0.0	1.114	111.1	LOS F	76.1	535.4	1.00	1.19	2.52	20.5
2	T1	83	0.0	83	0.0	1.114	111.4	LOS F	76.1	535.4	1.00	1.19	2.52	20.7
3a	R1	1502	0.8	1502	0.8	1.114	115.0	LOS F	76.1	535.4	1.00	1.19	2.52	13.2
Appro	oach	1738	0.7	1738	0.7	1.114	114.5	LOS F	76.1	535.4	1.00	1.19	2.52	14.4
North	nEast: V	Verrington	Rd (N)											
24a	L1	802	1.0	802	1.0	0.439	3.9	LOSA	1.4	9.9	0.43	0.49	0.43	52.8
26a	R1	42	0.0	42	0.0	0.439	7.7	LOSA	1.4	9.9	0.44	0.49	0.44	47.1
26b	R3	23	0.0	23	0.0	0.439	9.8	LOSA	1.4	9.9	0.44	0.49	0.44	48.8
Appro	oach	867	1.0	867	1.0	0.439	4.2	LOSA	1.4	9.9	0.43	0.49	0.43	52.4
North	n: Ranc	e Rd												
8	T1	73	0.0	73	0.0	0.453	41.8	LOS C	1.5	10.5	1.00	1.09	1.22	34.1
9	R2	1	0.0	1	0.0	0.453	46.3	LOS D	1.5	10.5	1.00	1.09	1.22	32.7
Appro	oach	74	0.0	74	0.0	0.453	41.9	LOS C	1.5	10.5	1.00	1.09	1.22	34.1
West	: EW Li	nk												
10	L2	1	0.0	1	0.0	0.725	88.0	LOS F	3.2	22.4	1.00	1.31	1.75	22.7
10a	L1	36	0.0	36	0.0	0.725	87.7	LOS F	3.2	22.4	1.00	1.31	1.75	15.1
12	R2	82	0.0	82	0.0	0.725	92.5	LOS F	3.2	22.4	1.00	1.31	1.75	23.6
Appro	oach	119	0.0	119	0.0	0.725	91.0	LOS F	3.2	22.4	1.00	1.31	1.75	21.4
All Ve	ehicles	2798	0.7	2798	0.7	1.114	77.4	LOS F	76.1	535.4	0.82	0.97	1.80	19.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**▽** Site: 101 [Site 3 - Rance Rd 2029 PM]

♦♦ Network: N101 [Base + Dev 2029 - PM Peak (Mitigation)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
North	East: V	Verrington	Rd (N)											
8	T1	867	1.0	867	1.0	0.448	0.1	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	867	1.0	867	1.0	0.448	0.1	NA	0.0	0.0	0.00	0.00	0.00	59.9
North	West: F	Rance Rd												
10	L2	32	3.3	32	3.3	0.233	32.5	LOS C	0.3	2.0	0.93	0.99	1.00	36.0
Appro	oach	32	3.3	32	3.3	0.233	32.5	LOS C	0.3	2.0	0.93	0.99	1.00	36.0
South	nWest: \	Werrington	Rd (S	)										
2	T1	1538	8.0	1384	0.8	0.713	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.6
Appro	oach	1538	8.0	1384 <sup>N</sup>	0.8	0.713	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.6
All Ve	hicles	2437	0.9	2283 <sup>N</sup>	0.9	0.713	0.5	NA	0.3	2.0	0.01	0.01	0.01	58.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

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